

Site Address
Resource Consent Number
Flux Reviewed

Rangitopu Development Riverhead, Auckland
BUN00449727
Stormwater Management Plan Rev A, Main Date: 30 April 25
Appendix A1.3 Stormwater Downpipe, Main
Flux Assessment Report: Main, Rev A, Date: 5 May 2025

Model Review Comments

Item Number	Comments	Description of issues or concerns	Supporting figures	Reason for request	Mayor Response 202020	Mayor Response 202024
	HEC-RAS Model					
	2D Model inflow boundaries	No hydrological model files have been provided to support the inflow hydrograph used in the model.		Justifications of the peak flow adopted.	The hydrological model files have been provided in the revised OneDrive link, which includes the hydrograph used in the model.	The hydrological model files have been provided in the revised OneDrive link, which includes the hydrograph used in the model.
3	Representation of the post development building platforms and results in future development scenarios	The model results indicate significant ponding with depths vary from 0.6m to 1.5m over large areas, possibly due to grading design issues. These pondings are unrealistic and can distort the peak discharge from these areas.	Figure 1	Excessive pondings can attenuate peak flow	As discussed in the meeting, the ponding on the platforms and/or within the Retirement Village is representative of what occurs to largely flat or low grade areas during periods of sustained rainfall in a 500-year return event. Flooding and sheet flow of surface water does occur in such areas during rainfall events. The surface within the HEC-RAS model is the actual surface which Main has designed, including all platforms in the CSL, and the Retirement Village, which includes the roads which act as the 2D flow paths within the village, and all of the areas have been graded so that external run-off does not occur towards the built areas. Main does not agree that the ponding shown in the HEC-RAS model is unrealistic, nor do we believe that this is skewing the results. Instead, we would argue that the suggestion of including a design rain-fall event before the 200-year event would skew the results, and that such a scenario is neither reasonable or realistic to model.	Additional Modelling was completed and sent through on the 23/10/25 investigating the effects of the ponding on the building platforms on the peak flow. Two sensitivity runs were completed for the 1000YCS including: Scenario 1: Terrain updated to remove discussed ponding Scenario 2: Terrain updated to remove discussed ponding and 10% Manning (increased to 0.05) Updates increased post development flows by approximately 2%, however flows were still found to be less than predevelopment.
5	Manning's values used	The model adopted a value of 0.1 for all streams and general pervious area, 0.3 for building/impervious and 0.03 for roads. Suggest more realistic values to be used pervious area and stream based on land cover. Manning's values of 0.1 should be used for building platforms.		0.1 for all pervious area can slow down the runoff and therefore reduce peak discharges modelled.	We believe that the use of 0.1 is acceptable throughout the site. The reasons behind this assumption is that rugged terrain are not reflective of pasture or grass land, and surface water flowing over the land has to pass through shrubs, logs and other debris. The use of 0.1 in the Retirement Village is also considered acceptable, as the retirement village features, fencing, retaining, landscaping and buildings of which is consistent with urban areas which the Code of practice requires the use of 0.1 for.	We believe that the use of 0.1 is acceptable throughout the site. The reasons behind this assumption is that rugged terrain are not reflective of pasture or grass land, and surface water flowing over the land has to pass through shrubs, logs and other debris. The use of 0.1 in the Retirement Village is also considered acceptable, as the retirement village features, fencing, retaining, landscaping and buildings of which is consistent with urban areas which the Code of practice requires the use of 0.1 for.
6	Downstream model boundary condition	A constant water level of 2.55m has been adopted at the junction with the Rangitopu River. Regional Rate Road Flood Model indicates an elevated tail water during extreme events. Suggest the TS from the RRM to be used as a dynamic boundary condition.	Figure 2 and Tab DS VLS TS	Downstream boundary can impact flood levels at Riverhead	Confirming Main will use a 500-year model with the dynamic heights of the downstream boundary condition. The results of this will be shared with Healthy Waters as soon as these are available. We remain of the view, that this scenario is not critical, as absolute values downstream should not be a key concern, if our model continues to confirm the reduction in peak run-off, which will deliver downstream benefits irrespective of the boundary condition modelled.	Additional Modelling was completed and sent through on the 23/10/25 investigating the effects of the using the provided dynamic height downstream boundary condition. Model confirmed that dynamic boundary did not have any effect on the peak flows and assessment.
7	Pre development catchment runoff CN number	A pre development CN number of 74 has been adopted for the catchment area while a CN number of 79 is assumed for future pervious areas converted for native bush. Please note the existing ground has a top soil depth of 200-400mm. The catchment area was mostly covered by forest in the past on long term average basis. A conservative CN of 79 is considered to be more representative of the long term average condition of the area. Minimum top soil depth of 300mm should be provided for all pervious area to	Figures 19,20 and 21 showing historic land cover	Assessment of effects from development requires appropriate representation of the existing condition.	As was noted in the meeting we do not agree with the sentiment that the pre-development condition should assume a CN value of 70 based on historic land cover. We are instead of the view that the pre-development assessment should be based on the current status of the site, which for all of Lot 1, is currently logged. The existing catchment within Segues 6 and 7 which feature forest cover are also being logged in December 2025 - March 2026. Lot 2 features recently planted pine (4-5 years old) which also features minimal cover. These trees will be cut and removed without any consideration of downstream effects due to existing nature of the forest. A more robust planning and legislative review to the effects based assessment will be provided to the panel in the forthcoming response on 24/10/25. If we assume a CN value of 70 for a plantation forest, this fails to consider the increased run-off effects that occur during after and for a time beyond the forest is harvested. On this basis, if a rainfall event was to occur after logging activities were done, the effects on Riverhead would not be increased. A value of CN 74 has been used in all of Council's modelling and PCS20, and this is further reasoning why we have used this as a basis for our pre-development assessment.	As was noted in the meeting we do not agree with the sentiment that the pre-development condition should assume a CN value of 70 based on historic land cover. We are instead of the view that the pre-development assessment should be based on the current status of the site, which for all of Lot 1 is currently logged. The existing catchment within Segues 6 and 7 which feature forest cover are also being logged in December 2025 - March 2026. Lot 2 features recently planted pine (4-5 years old) which also features minimal cover. These trees will be cut and removed without any consideration of downstream effects due to existing nature of the forest. A more robust planning and legislative review to the effects based assessment will be provided to the panel in the forthcoming response on 24/10/25. If we assume a CN value of 70 for a plantation forest, this fails to consider the increased run-off effects that occur during after and for a time beyond the forest is harvested. On this basis, if a rainfall event was to occur after logging activities were done, the effects on Riverhead would not be increased. A value of CN 74 has been used in all of Council's modelling and PCS20, and this is further reasoning why we have used this as a basis for our pre-development assessment.
8	Post development CN Number	There is no calculation and GIS file supporting the derivation of the composite CN number for each infiltration type polygon. The converted bush area will be in the range of 15% and will take a very long time to establish. Therefore there will be a long time during and after the development, where the represented bush area will not be established. The increased flood risk during this period should also be assessed. The timing of the artwork, housing development and native bush establishment should be provided to enable a clear understanding of this intermediate risk.	Figure 3	Assessment of effects from development requires appropriate representation of the post development condition	Mayor's position remains that the pre-development assessment is based on a conservative CN of 74, which we are of the view that the current CN should be 77. As such, we feel that this allows for the transition period where the bush is establishing. Given that modelling is based on long term probabilities, we still feel that the approach is the best for the catchment. If this is an issue, we are open to introducing some interim mitigation which would use on-site attenuation of roof and/or driveway areas being proposed. However, we do not believe that this is required for property and would suggest that such controls would only be required for the first few years, or until canopy cover is achieved in the planted areas. We are happy to discuss how this could be considered, if this is an approach that we need to take. We are of the view, that this would only need to be considered for the eastern catchment.	Mayor's position remains that the pre-development assessment is based on a conservative CN of 74, which we are of the view that the current CN should be 77. As such, we feel that this allows for the transition period where the bush is establishing. Given that modelling is based on long term probabilities, we still feel that the approach is the best for the catchment. If this is an issue, we are open to introducing some interim mitigation which would use on-site attenuation of roof and/or driveway areas being proposed. However, we do not believe that this is required for property and would suggest that such controls would only be required for the first few years, or until canopy cover is achieved in the planted areas. We are happy to discuss how this could be considered, if this is an approach that we need to take. We are of the view, that this would only need to be considered for the eastern catchment.
9	Post development pervious area CN number	Geotechnical investigation indicates existing top soil depth from 0.2m to 1.0m at the catchment area. For the post development catchment area, if similar top soil depth can not be maintained then higher CN number should be used.		Assessment of effects from development requires appropriate representation of the post development condition	For the CSL development, we are only going to be earth working the platform areas and roads. There is no wider earthworks proposed outside of these areas. Within the earthworked platform areas, the existing topsoil (which includes stumps / roots and vegetation) will be matched in situ down to a depth of 300mm (average) and then this material will be excavated and placed on hard around the carpark areas. Bulk out and 100 tonnes will be placed on the site plans, before the topsoil is placed over the platform area. We are aiming for a depth of 200-300mm topsoil which may be supplemented by additional mulch (pine chippings from on-site) in the planted areas directly around the platform. On this basis, the CN values for future landscape area is considered acceptable.	For the CSL development, we are only going to be earth working the platform areas and roads. There is no wider earthworks proposed outside of these areas. Within the earthworked platform areas, the existing topsoil (which includes stumps / roots and vegetation) will be matched in situ down to a depth of 300mm (average) and then this material will be excavated and placed on hard around the carpark areas. Bulk out and 100 tonnes will be placed on the site plans, before the topsoil is placed over the platform area. We are aiming for a depth of 200-300mm topsoil which may be supplemented by additional mulch (pine chippings from on-site) in the planted areas directly around the platform. On this basis, the CN values for future landscape area is considered acceptable.
11	Representation of Culverts proposed in the model	There are at least five very large culverts proposed which are not found in the HEC-RAS Model, either in Figure 4 - Please explain.	Figure 4	Correct representation of all culverts proposed	These culverts have been added to the model. (Downstream catchment model still containing minor updates and will be made available tomorrow)	Additional Modelling was completed and sent through on the 23/10/25 and 23/10/25 with additional culverts added. Two of the mentioned culverts were located in the Western catchment and three in the eastern catchment. Updated model included Culverts 1-5, 1-6, 4-6 and updated culvert 2 to 1.5m x 1.5m box shape culvert addition of culvert 6-6, 1-6, 13-6 results in peak flow of cross section 6 (figure below) increase of 0.3m from 145.48m to 145.78m, the peak flow is initially 495.84 m/s to 496.84 m/s below development (496.84 m/s).
12	Culverts representation inconsistencies	The 2.6m culvert under Deacon Road near the junction with Link Road is included in the pre-development model but not in the post development model. Please explain.		Correct representation of existing culvert	The 2.6m culvert is proposed to be upgraded to a bridge. Proposed development terrain modification has been used to model the reclaimed stream beneath the bridge. Refer to excel sheet "Response sheet 2". We note that this is consistent in all post-development scenarios modelled.	The 2.6m culvert under Deacon Road is proposed to be upgraded to a bridge. Proposed development terrain modification has been used to model the reclaimed stream beneath the bridge. Refer to excel sheet "Response sheet 2". We note that this is consistent in all post-development scenarios modelled.
13	Culverts representation inconsistencies	Culverts 1 to 7 in the post development model. Are there any existing culverts at the locations of Culverts 1-7 under the pre-development condition? Please explain.		Correct representation of existing culvert	The pre-development model does not include any of the existing culverts under Forestry Road, due to their condition and size. All existing culverts are listed under Appendix A of the flood modelling report. The largest culvert under Forestry Road (all of which have been surveyed) is 450mm. As such, given the existing state (poor condition and some retention barriers), and also ensure that the model is correctly represented with them being blocked. This also ensures that the pre-development state includes backwater effects to the west of Forestry Road.	The pre-development model does not include any of the existing culverts under Forestry Road, due to their condition and size. All existing culverts are listed under Appendix A of the flood modelling report. The largest culvert under Forestry Road (all of which have been surveyed) is 450mm. As such, given the existing state (poor condition and some retention barriers), and also ensure that the model is correctly represented with them being blocked. This also ensures that the pre-development state includes backwater effects to the west of Forestry Road.
14	Culvert size inconsistencies	Culvert 4 is 6m x 3m in the report, 6m x 3m (path 0.5m embankment). Modelled culvert 4 is 6m x 3m (path 0.5m embankment). Culvert 3 is 4m x 2m (path 0.5m embankment). Modelled culvert 3 is 4m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.	Figures 5,6 and 14	Culvert size inconsistency	Culverts 4 size is 6m x 3m (path 0.5m embankment). Modelled culvert 4 is 6m x 3m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.	Culverts 4 size is 6m x 3m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.
15	Culvert size inconsistencies	Culvert 3 is 4m x 2m in the report, 4m x 2m in the report and plans. Invert levels don't match.	Figure 6	Culvert size inconsistency	Culverts 3 size is 4m x 2m (path 0.5m embankment). Modelled culvert 3 is 4m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.	Culverts 3 size is 4m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.
16	Culvert size inconsistencies	Culvert 2 is 1.5m Dia culvert, but a 1.5m Dia culvert on plan. Invert levels don't match.	Figure 7	Culvert size inconsistency	Culverts 2 size is 1.5m x 2m (path 0.5m embankment). Modelled culvert 2 is 1.5m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.	Culverts 2 size is 1.5m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error, so this is why there is a difference in size between HEC Model and the design plans. But this accounts for the embankment depth and details in the standard cross-section drawings.
17	Culvert size inconsistencies	Culvert 6 is 2m x 1m in the report, 1.5m x 1.5m in the model and on the design plans. Invert levels don't match between design and model. Please explain.	Figure 9 and Figure 4	Culvert size inconsistency	Culverts 6 size is 1.5m x 2m (path 0.5m embankment). We have updated this to be a box culvert not 1500mm round. Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error.	Culverts 6 size is 1.5m x 2m (path 0.5m embankment). We have updated this to be a box culvert not 1500mm round. Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error.
18	Culvert size inconsistencies	Culvert 1 is 5m x 2m in the report, 5m x 2m in the model and 4m x 2m on the design plans. Invert levels don't match between design and model. Please explain.	Figure 8	Culvert size inconsistency	Culverts 1 size is 5m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error.	Culverts 1 size is 5m x 2m (path 0.5m embankment). Modelled culvert in HEC RAS model does not include the embankment depth to avoid software error.

38		Culvert 7 included a blocked 1.5m box culvert. Design plan show a bridge not included in the model.	Figure 1		Culvert 7 is legacy culvert and shall be removed.	Culvert 7 is legacy culvert and shall be removed.
17	Culvert position in model	Many culverts in the HEC-RAS model has centre line station of 0, this leads to that half of the culvert opening is outside the embankment length. Please correct this to ensure appropriate representation of the model.		Correct modeling of culvert positions	It is noted that HEC RAS only uses the digitized X,Y coordinates of culvert barrels when connected to 2D flow areas, therefore this shall have no impact on the computations	It is noted that HEC RAS only uses the digitized X,Y coordinates of culvert barrels when connected to 2D flow areas, therefore this shall have no impact on the computations
18	Culvert sizes	It has been noticed that soft base for fish pass is proposed as a typical design for all box culvert. Has the lost cross section area and roughness of the soft base been considered in culvert hydraulic modelling?	Figure 15		Base of culverts have been modelled as the top of the embedded depth with a Manning value of 0.035 to account for bedding roughness.	Base of culverts have been modelled as the top of the embedded depth with a Manning value of 0.035 to account for bedding roughness.
19	On Detention Pond modelling	No outlet included for the dry detention pond. Please clarify how is the detention represented. Any initial water pool?	Figure 12	Correct modeling of dry detention pond	Outlet not included in 100yr model as assumed to be fully blocked. No initial water level with pond base at nRL 32.0.	Outlet not included in 100yr model as assumed to be fully blocked. No initial water level with pond base at nRL 32.0.
20	Terrain and Structure representation issues	Duke Road Bridge not included, and DTM errors in stream corridor require verifications	Figure 16	Model representation of terrain and structures	Confirming Phase will run a 100-yr model with the features included in the pre and post model. We endeavour to do this before Wednesday's meeting. We remain of the view, that this scenario is not critical, as absolute levels downstream should not be a key concern. If our model continues to outline the reduction in peak run-off, which will deliver downstream benefits.	Additional Modelling was completed and sent through on the 22/10/25 with downstream structures listed in Item 20, 21 and 22. Provided sensitivity check model confirmed that the downstream structures did not have any effect on the peak flows. Confirming mentioned downstream structures have no effect on relative water levels.
21	Structure representation issues	Private bridge to 17 Wairua Drive not included.	Figure 17	Model representation of structures	Confirming Phase will run a 100-yr model with the features included in the pre and post model. We endeavour to do this before Wednesday's meeting. We remain of the view, that this scenario is not critical, as absolute levels downstream should not be a key concern. If our model continues to outline the reduction in peak run-off, which will deliver downstream benefits.	Additional Modelling was completed and sent through on the 22/10/25 with downstream structures listed in Item 20, 21 and 22. Provided sensitivity check model confirmed that the downstream structures did not have any effect on the peak flows. Confirming mentioned downstream structures have no effect on relative water levels.
22	Structure representation issues	Private bridge to 22 School Road not included	Figure 18	Model representation of structures	Confirming Phase will run a 100-yr model with the features included in the pre and post model. We endeavour to do this before Wednesday's meeting. We remain of the view, that this scenario is not critical, as absolute levels downstream should not be a key concern. If our model continues to outline the reduction in peak run-off, which will deliver downstream benefits.	Additional Modelling was completed and sent through on the 22/10/25 with downstream structures listed in Item 20, 21 and 22. Provided sensitivity check model confirmed that the downstream structures did not have any effect on the peak flows. Confirming mentioned downstream structures have no effect on relative water levels.
23	HEC-RAS Model set up	There are too many models provided. All scenarios can be combined into one model for easy checking and comparisons.		Too many models can cause inconsistencies and hard to compare results	As was noted in the meeting, due to the size and nature of the catchment, we have had to split the models so that we can run the model in a reasonable timeframe. Whilst we appreciate that this is not the usual approach, we are not able to combine all the models, as this becomes too labour and time intensive.	As was noted in the meeting, due to the size and nature of the catchment, we have had to split the models so that we can run the model in a reasonable timeframe. Whilst we appreciate that this is not the usual approach, we are not able to combine all the models, as this becomes too labour and time intensive.
24	Culverts for countryside lying into Kaipara Harbour	These culverts and detention pond proposed have not been included in the model.		Need to verify the performance of the proposed detention at Culvert 1.1	The crest height was adjusted slightly in our formal response to the Panel, the model is currently been re-run to address this change as to ensure the design and model is consistent. We are going to be in a position to show this model tomorrow for review / input before the meeting.	The crest height was adjusted slightly in our formal response to the Panel, the model is currently been re-run to address this change as to ensure the design and model is consistent. Model was sent through on the 22/10/25
25	Provide a list of scenarios	The scenario name and relevant metadata should be included to enable effective review and interpretation of the model setup and results.			A list of scenarios run and the models to which they relate will be produced, including the additional modelled scenarios currently underway will be provided tomorrow with a list. The new models (in changes to platforms, and the 100-yr model with the amended culverts is included in the OneDrive link, refer to the folder marked yellow below to the right	A list of scenarios run and the models to which they relate will be provided.
26	Impact on existing dam at 49 Forest Road	Please assess the potential impact on the existing dam structure at No. 49 Forest Road, in terms of increase discharge, volume and flood duration	Figure 22	Failure of the dam structure can be detrimental to downstream properties at Riverhead	We have assessed the inflows (post development) for the catchments which flow into the existing pond at No. 49 Forest Road. In both instances, the flows are reduced from that of the pre-development modelled state, and the flood level has also reduced relative to depth in the pond area. As the peak flows are not increased in this catchment, no further assessment is warranted, or required in this instance. Whilst we appreciate that this part of the site is currently forested, the harvesting operation is going to occur December 2025 - March 2026. Thus, the pre-development state (as per all of Lots 1 and 2) will be clear felled, which will increase run-off from that off the pre-development model assumed. As the vegetation matures within the planted and revegetated areas, the proposal will deliver increased initial abstraction levels, and will progressively reduce runoff from the site. In comparison, if the rotation forestry was to occur, every 20 years, the site will be deforested, and increased runoff will occur. On this basis, we don't believe any further assessment relative to this existing feature is required.	We have assessed the inflows (post development) for the catchments which flow into the existing pond at No. 49 Forest Road. In both instances, the flows are reduced from that of the pre-development modelled state, and the flood level has also reduced relative to depth in the pond area. As the peak flows are not increased in this catchment, no further assessment is warranted, or required in this instance. Whilst we appreciate that this part of the site is currently forested, the harvesting operation is going to occur December 2025 - March 2026. Thus, the pre-development state (as per all of Lots 1 and 2) will be clear felled, which will increase run-off from that off the pre-development model assumed. As the vegetation matures within the planted and revegetated areas, the proposal will deliver increased initial abstraction levels, and will progressively reduce runoff from the site. In comparison, if the rotation forestry was to occur, every 20 years, the site will be deforested, and increased runoff will occur. On this basis, we don't believe any further assessment relative to this existing feature is required.

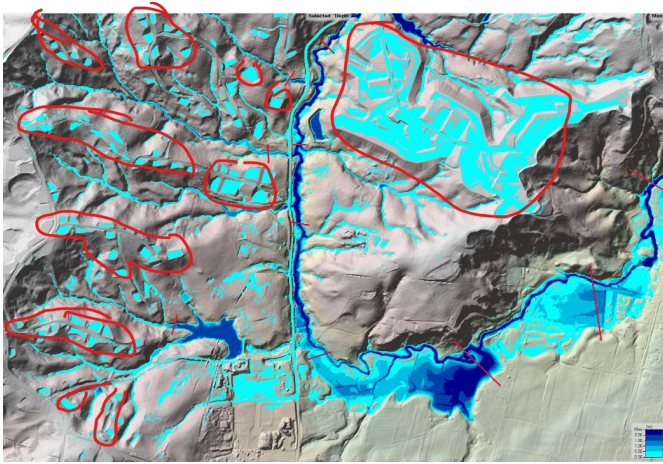


Figure 2 - RFM Rangitoto Water Level TS

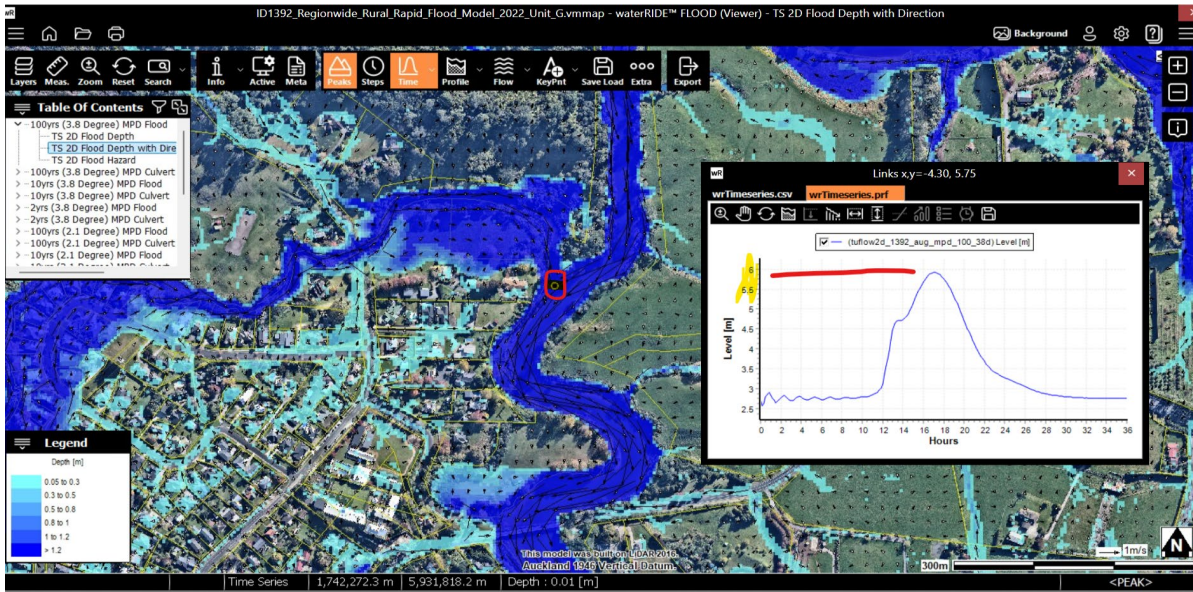
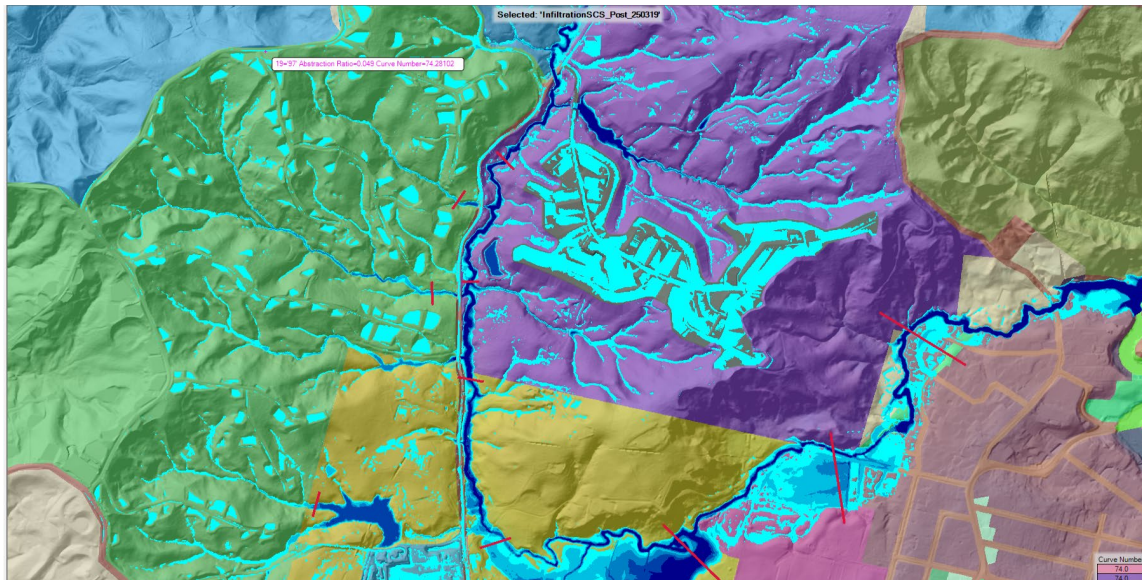


Figure 3 - Post Development CN Number - Infiltration layer



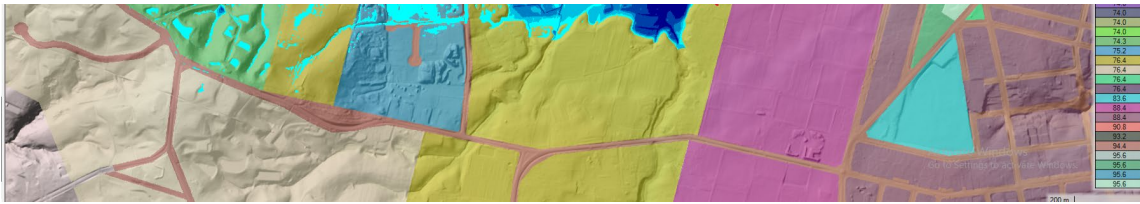


Figure 4 - Culverts Proposed - Not in the HEC-RAS Model

HEC-RAS CULVERT RESULTS SUMMARY										
CULVERT No.	TYPE	SIZE	CULVERT LENGTH (m)	Flow (m³/s)	VELOCITY (m/s)	RIP-RAP THICKNESS (m)	POOL LENGTH (m)	APRON LENGTH (m)	BAISIN LENGTH(m)	BAISIN WIDTH (m)
1	BOX CULVERT	5m x 2m	13.57	15.47	3.95	0.30	15.00	5.00	20.00	18.30
2	BOX CULVERT	1.5m x 1.5m	12.43	1.56	3.46	0.45	4.50	1.50	6.00	5.50
3	BOX CULVERT	4m x 2m	14.94	9.93	2.17	0.45	12.00	4.00	16.00	14.67
4	BOX CULVERT	6m x 2m	14.16	16.69	4.15	0.45	18.00	6.00	24.00	22.00
5	BOX CULVERT	2m x 2m	15.82	1.88	2.65	0.15	6.00	2.00	8.00	7.33
6	BOX CULVERT	2.5m x 2m	14.09	7.52	3.66	0.45	22.32	6.16	18.49	13.82
7	PRIMARY CIR-CULVERT	0.6m	29.08	1.22	4.34	0.45	4.57	2.28	6.85	5.17
COUNTRYSIDE										
13-4	CIR-CULVERT	1.20m	14.33	1.58	3.35	0.30	5.58	2.79	8.36	
14-1	BOX CULVERT	1.5m x 1.5m	13.31	4.07	5.28	0.45	9.11	4.56	13.67	10.61
14-6	CIR-CULVERT	1.05m	13.41	1.26	3.13	0.36	3.15	1.46	4.61	4.13
1-1	PRIMARY CIR-CULVERT	1.20m	15.09	3.42	3.89	0.36	3.60	1.20	4.80	4.40
1-1	SECONDARY CIR-CULVERT	4m x 1.5m	12.36							

Specific energy dissipation arrangement to be confirmed for the secondary culvert

Figure 5- Culvert 4

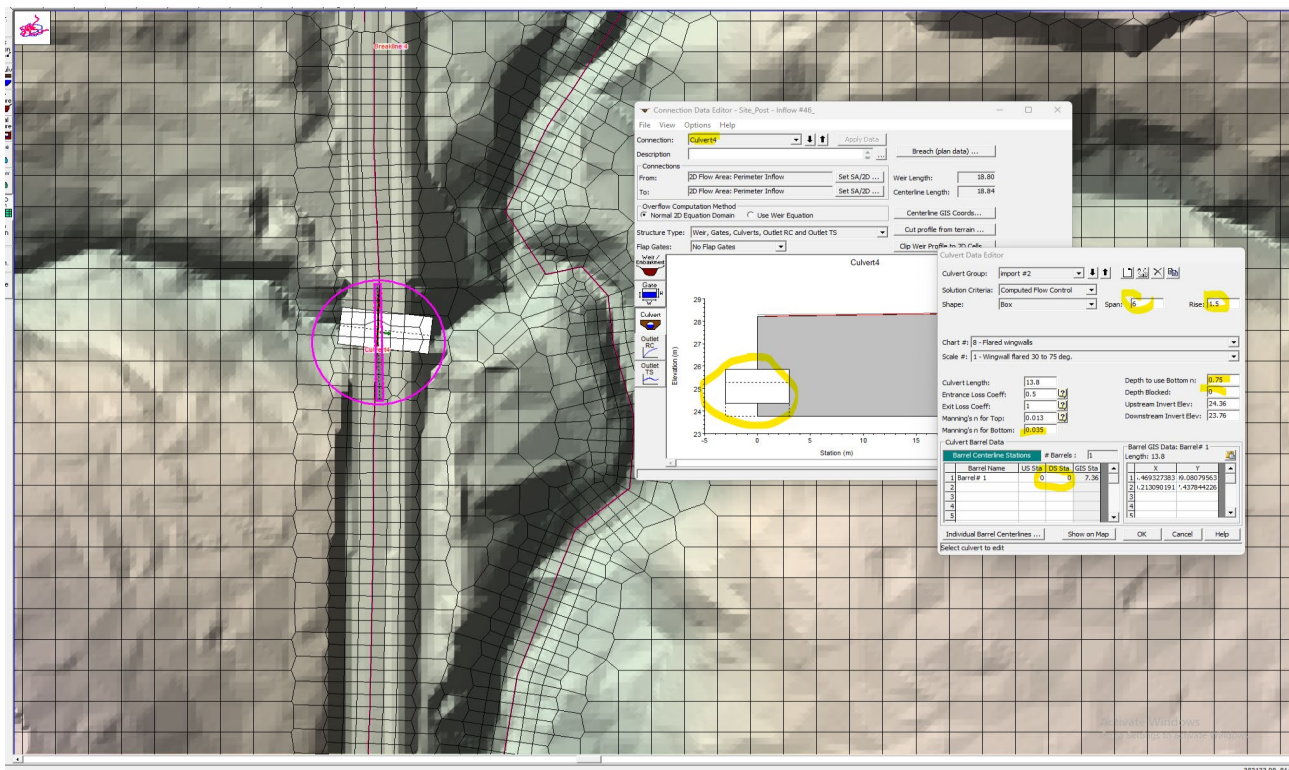
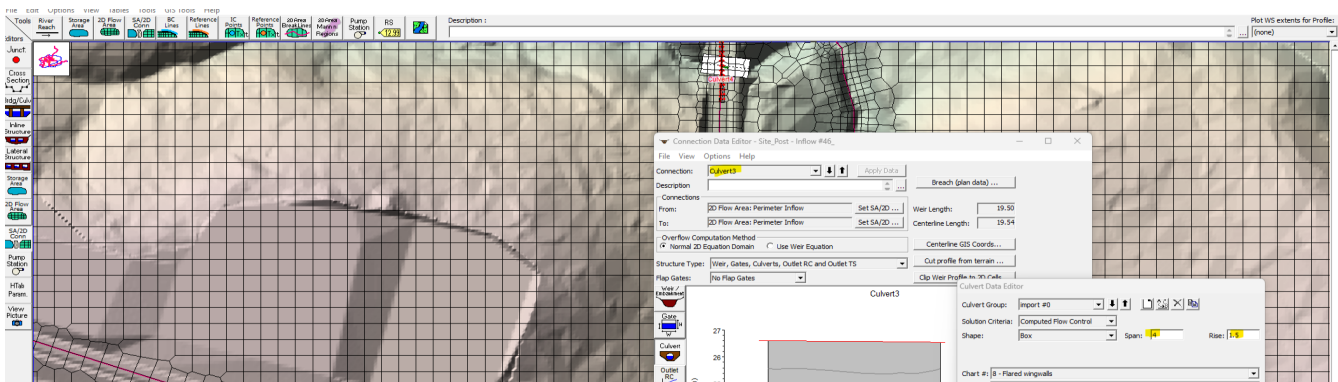


Figure 6- Culvert 3 size



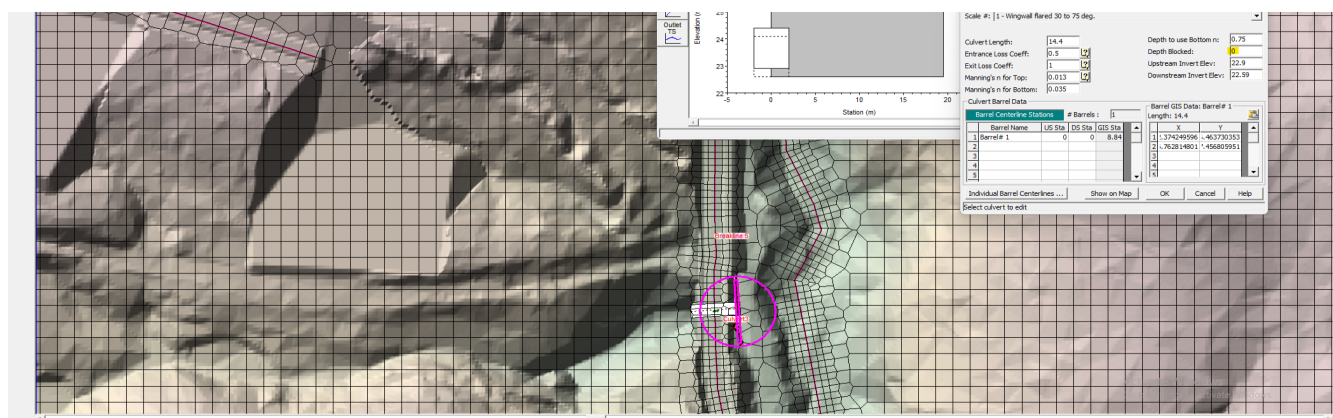


Figure 7 - Culvert 2

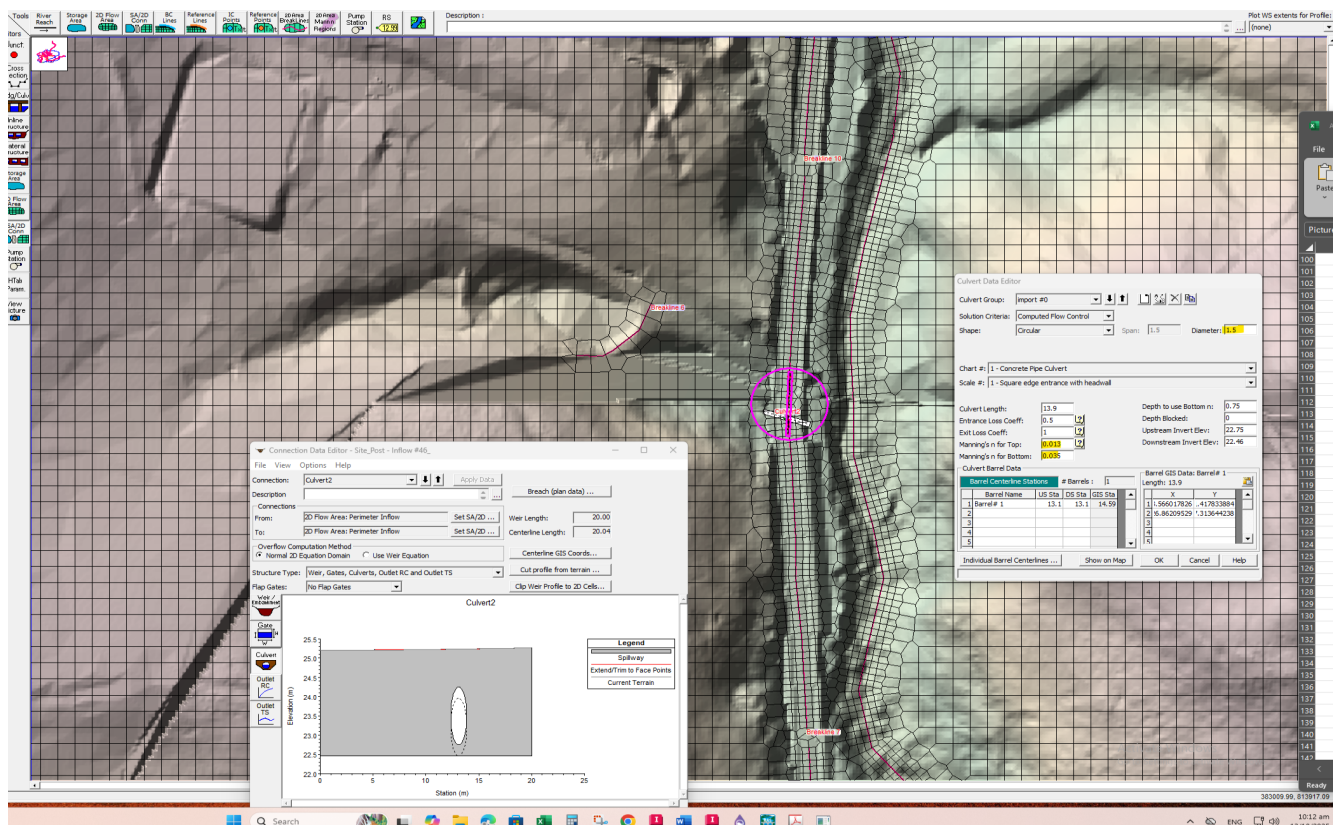


Figure 8 - Culvert 1 Size

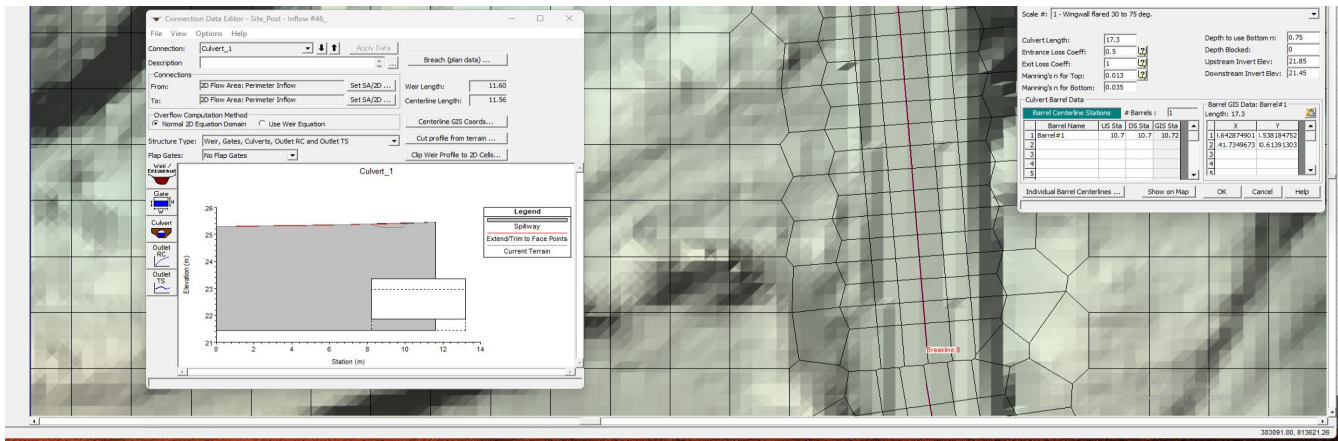


Figure 9- culvert 6

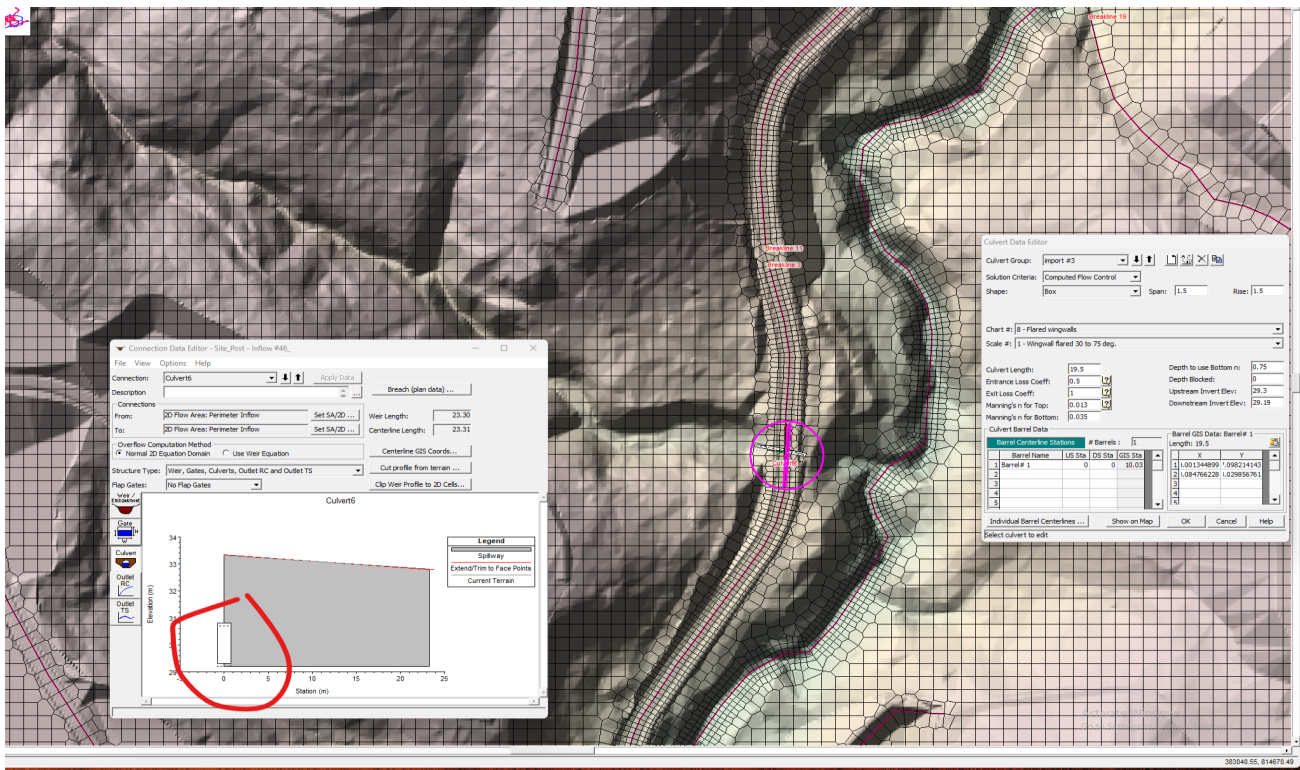


Figure 10
No culvert for the main stream channel under Forestry Road for Post Development Scenario



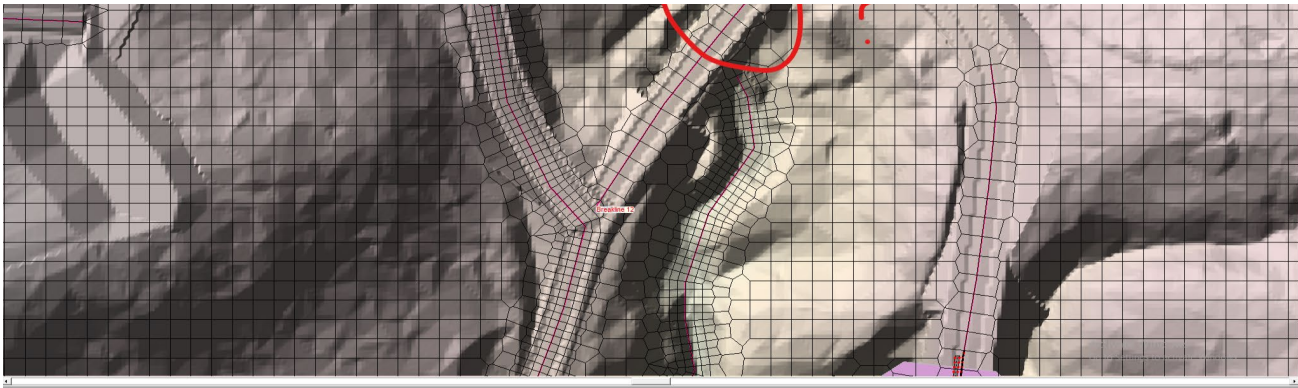
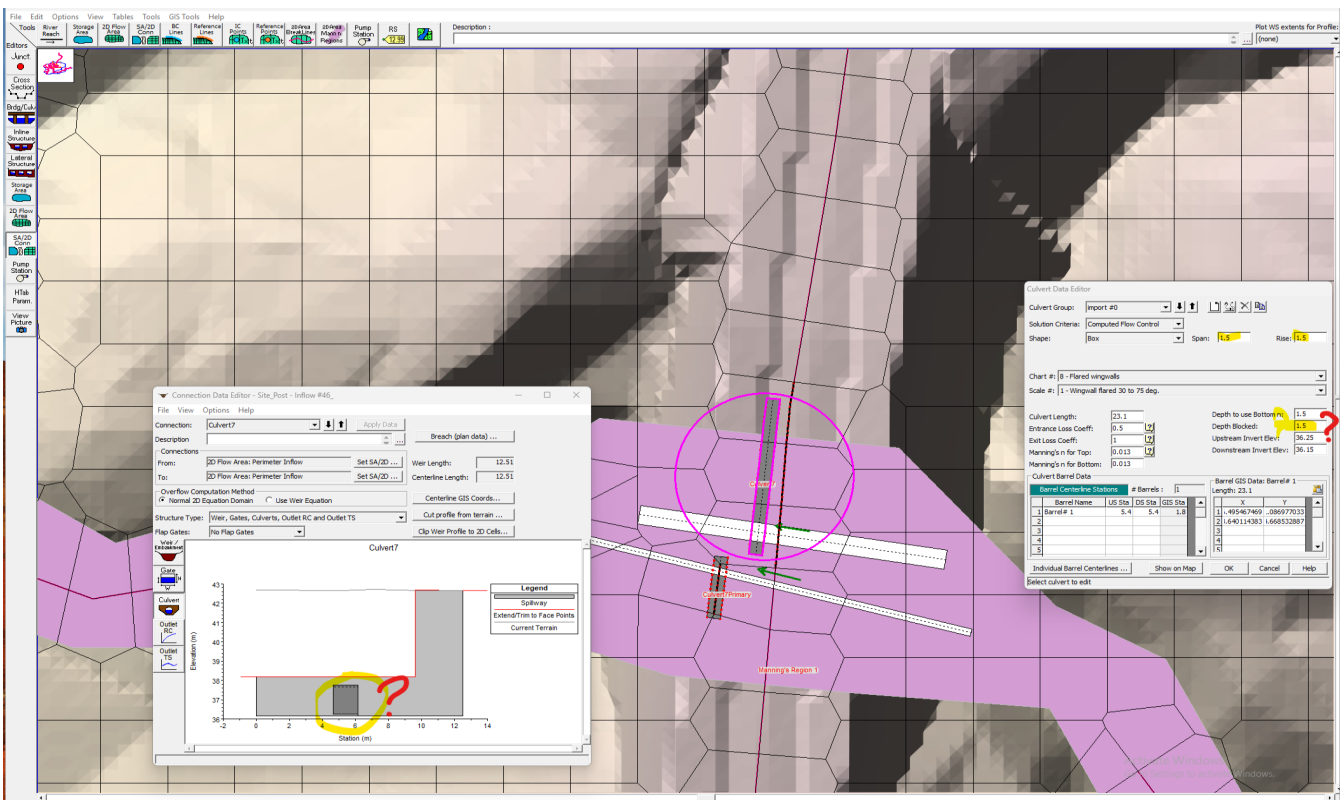
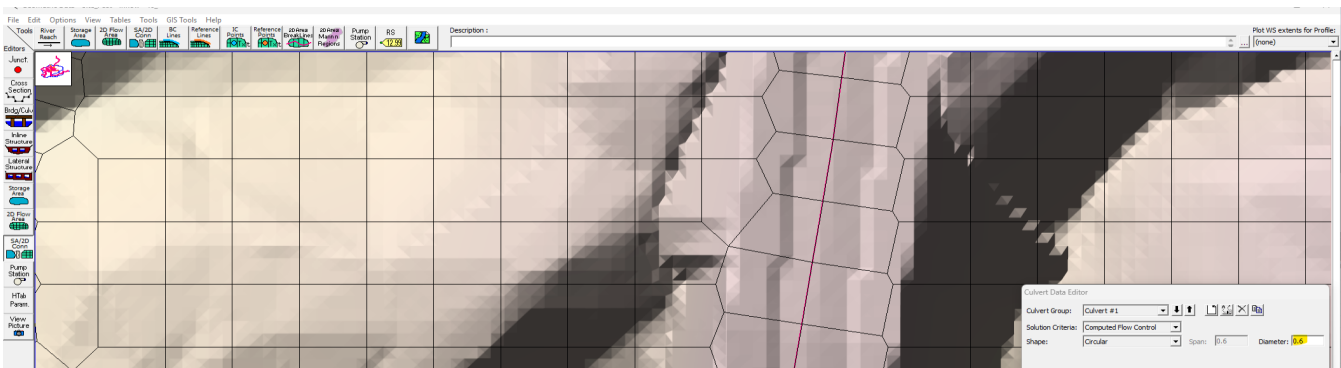


Figure 11
Culvert 7 - Blocked completely?



Culvert 7 - Primary



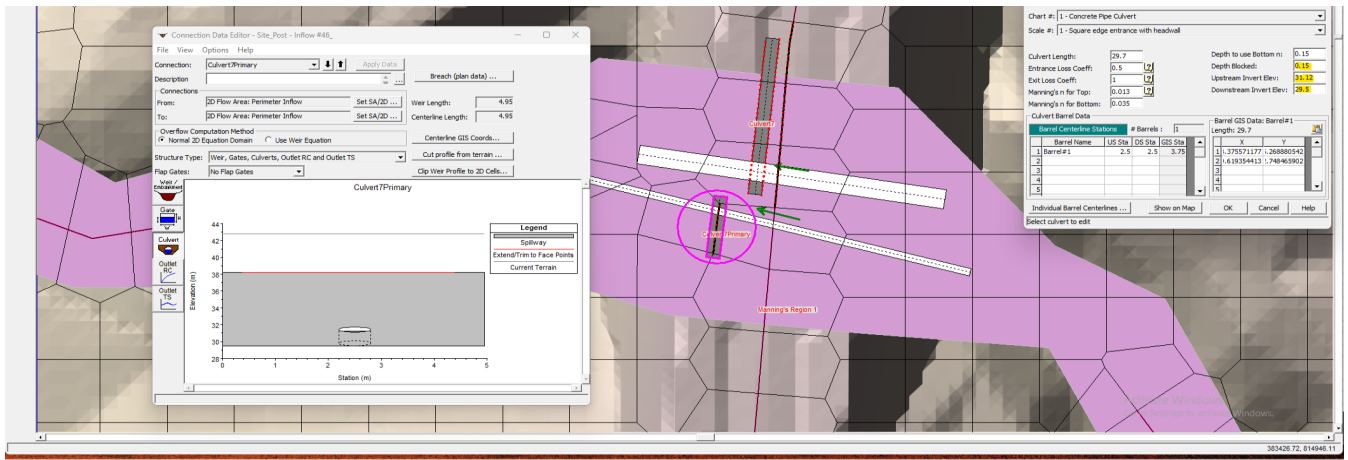
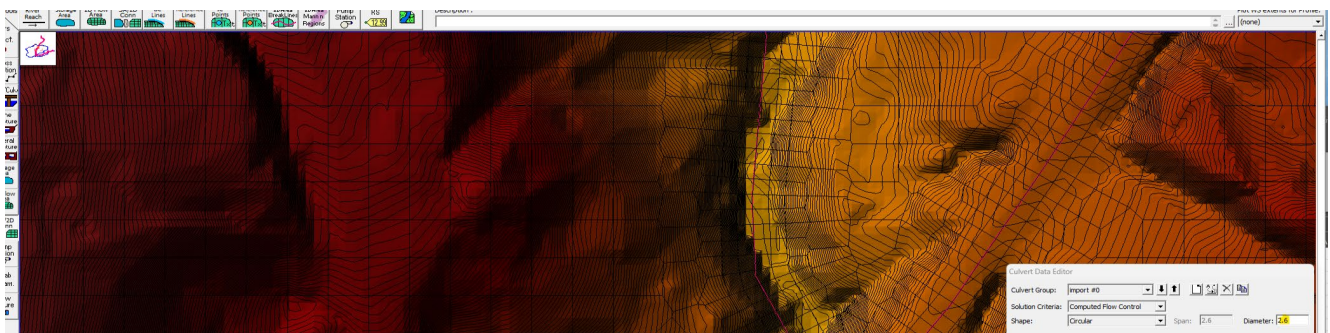


Figure 12
No Outlet from dry detention Pond



Figure 13
Pre-Development only one Culvert included in the model



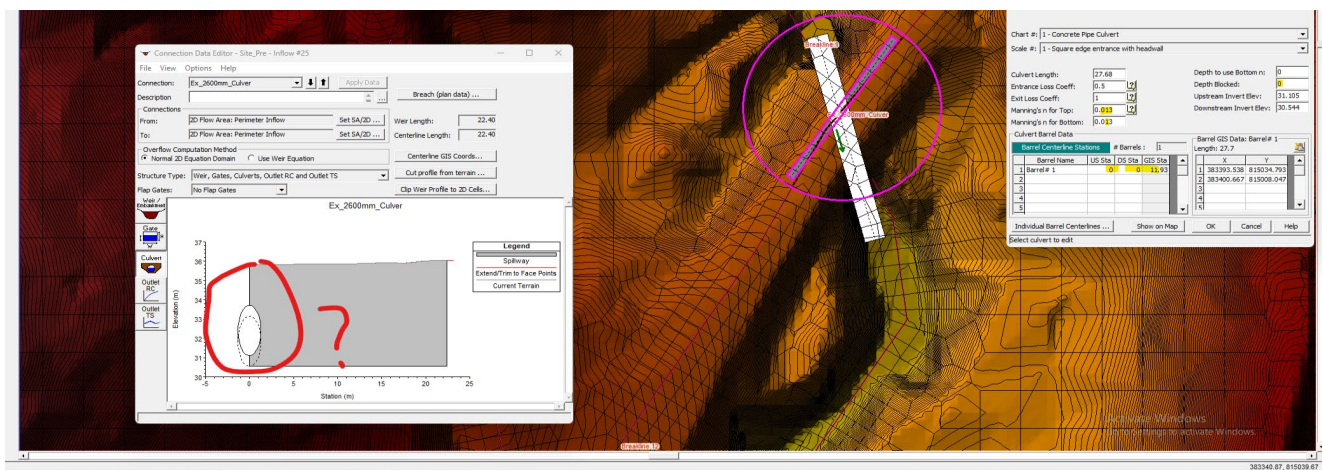


Figure 14
Culvert 5

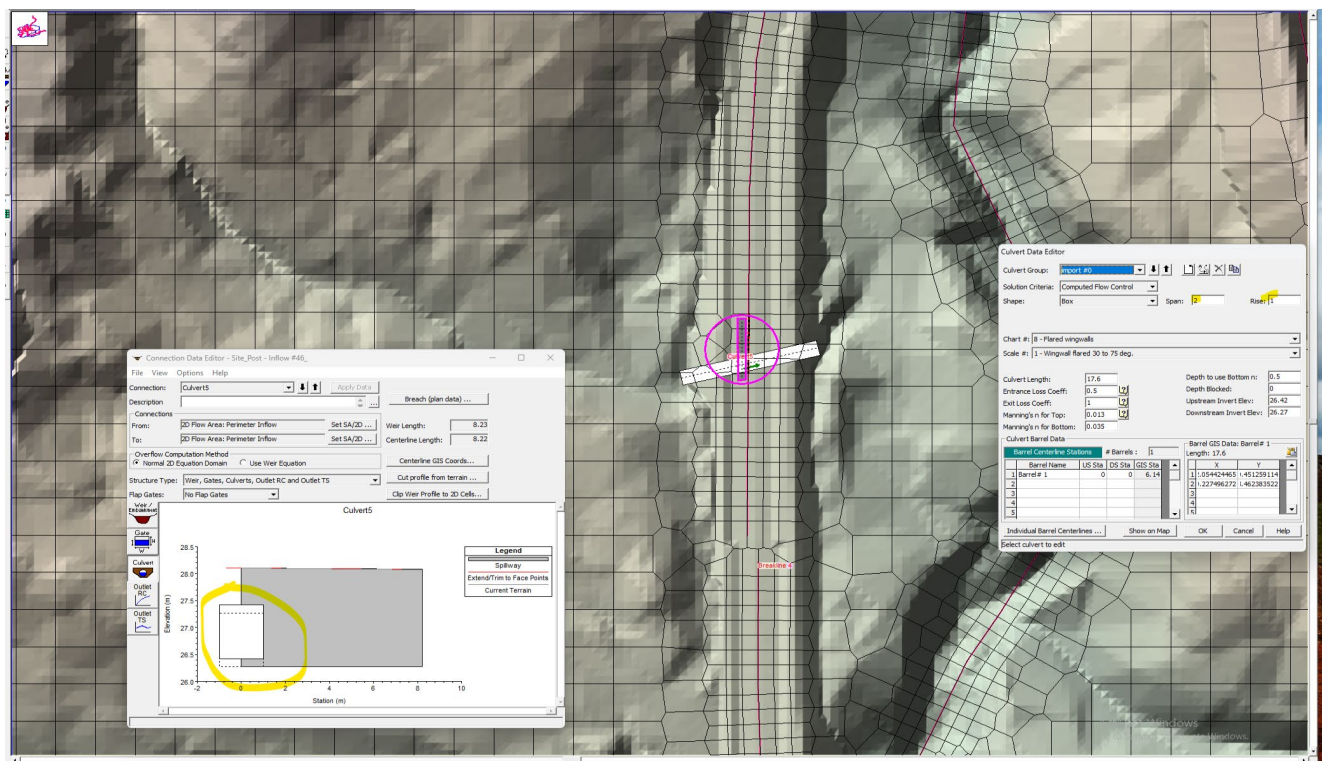


Figure 15 - Software for ecology design for all culverts

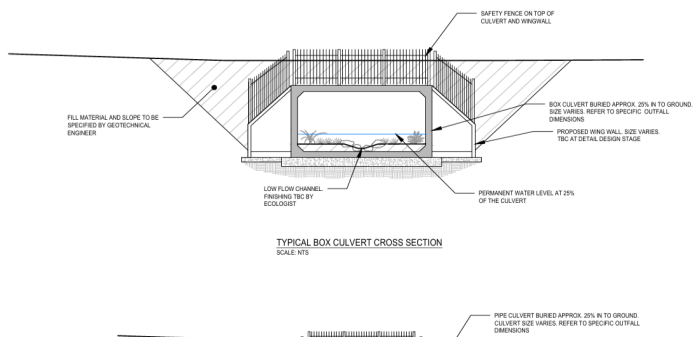




Figure 16 - Duke Road Bridge not included & DTM Issues with Stream corridor



Figure 17 - Private bridge to 17 Wautaiti Drive not included.



Figure 18 - Private Bridge to 22 School Road not included





Figure 19 - Catchment Landcover 2011

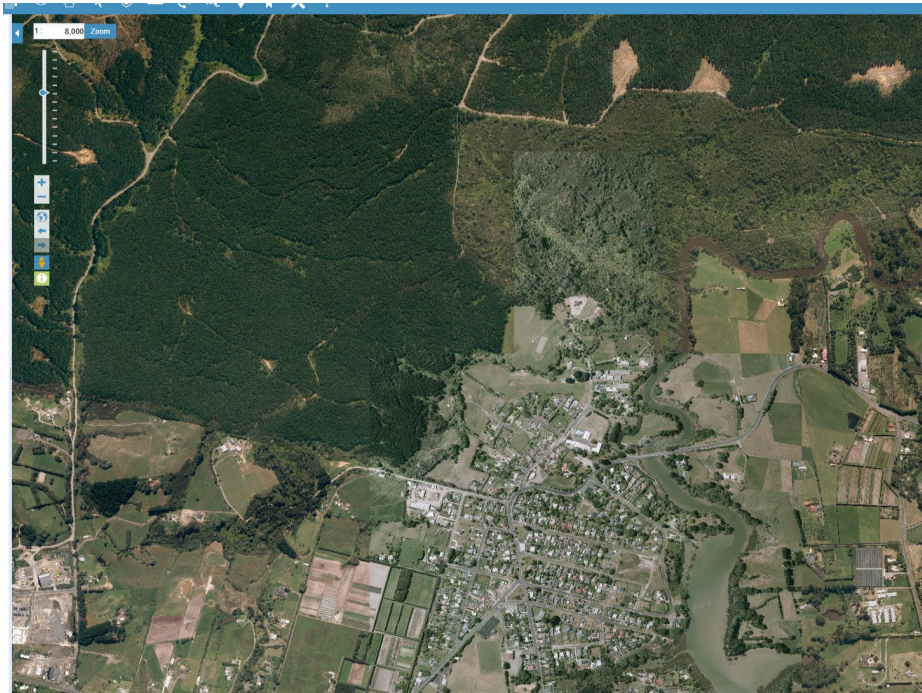


Figure 20 - Catchment Land Cover 2017



Figure 21 - Catchment Landcover 2024

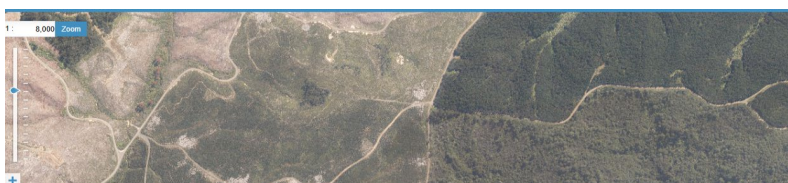


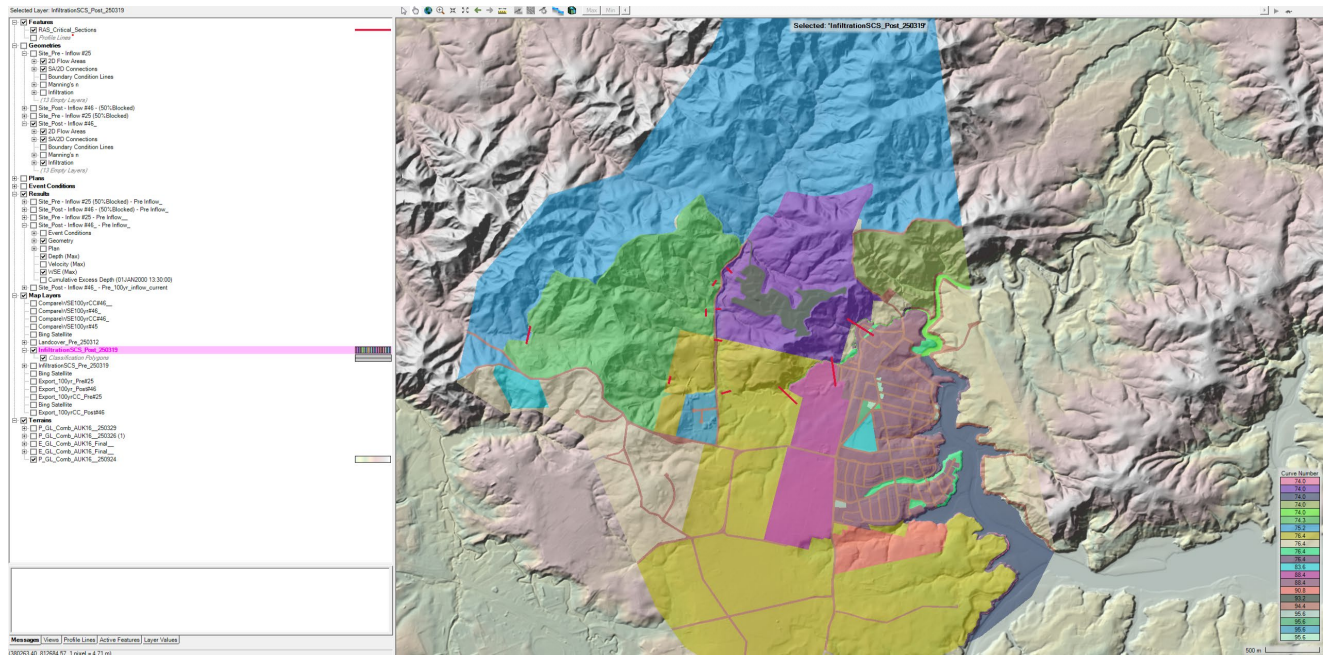


Figure 22 - Existing Dam Structure at No. 49 Forest Road



NoData		la
11	0.057	76.4 4.47226
3	0.057	76.4 4.47226
27	0.05	94.4 0.75339
44	0.043	95.6 0.50269
32	0.057	76.4 4.47226
33	0.06	83.6 2.98967
31	0.057	76.4 4.47226
59	0.056	74 4.99762
4	0.06	88.4 1.99982
19	0.06	88.4 1.99982
7	0.043	95.6 0.50269
30	0.056	74 4.99762
63	0.058	90.8 1.49267
17	0.043	95.6 0.50269
25	0.056	74 4.99762
15	0.056	74 4.99762
16	0.057	75.2 4.77466
12	0.043	95.6 0.50269
99	0.056	74 4.99762

NoData		la
11	0.057	76.4 4.47226
3	0.057	76.4 4.47226
27	0.05	94.4 0.75339
44	0.043	95.6 0.50269
32	0.057	76.4 4.47226
33	0.06	83.6 2.98967
31	0.057	76.4 4.47226
59	0.056	74 4.99762
4	0.06	88.4 1.99982
19	0.06	88.4 1.99982
7	0.043	95.6 0.50269
30	0.056	74 4.99762
63	0.058	90.8 1.49267
17	0.043	95.6 0.50269
25	0.056	74 4.99762
15	0.056	74 4.99762
16	0.057	75.2 4.77466
12	0.043	95.6 0.50269
99	0.056	74 4.99762



Time	(tuflow2d_1392_aug_mpd_100_38d) Level [m]
0	2.76
0.166667	2.577182
0.333333	2.636068
0.5	2.807314
0.666667	2.87597
0.833333	2.91023
1	2.841956
1.166667	2.766957
1.333333	2.71912
1.5	2.657488
1.666667	2.688811
1.833333	2.719011
2	2.785682
2.166667	2.816715
2.333333	2.828649
2.5	2.80516
2.666667	2.766113
2.833333	2.729264
3	2.710896
3.166667	2.706303
3.333333	2.737365
3.5	2.772203
3.666667	2.795065
3.833333	2.809331
4	2.792046
4.166667	2.765421
4.333333	2.741124
4.5	2.724181
4.666667	2.724931
4.833333	2.741152
5	2.766767
5.166667	2.787342
5.333333	2.79593
5.5	2.784798
5.666667	2.767904
5.833333	2.746042
6	2.732352
6.166667	2.733498
6.333333	2.747022
6.5	2.767271
6.666667	2.785126
6.833333	2.791134
7	2.782945

7.166667	2.768214
7.333333	2.75275
7.5	2.74387
7.666667	2.742537
7.833333	2.751061
8	2.766697
8.166667	2.780262
8.333334	2.786925
8.5	2.782892
8.666667	2.77281
8.833334	2.762426
9	2.756364
9.166667	2.758385
9.333334	2.766872
9.5	2.778969
9.666667	2.789091
9.833334	2.796251
10	2.795716
10.16667	2.795138
10.33333	2.796241
10.5	2.802296
10.66667	2.811493
10.83333	2.82834
11	2.846696
11.16667	2.868165
11.33333	2.895497
11.5	2.920159
11.66667	2.958719
11.83333	3.024479
12	3.137669
12.16667	3.37356
12.33333	3.584645
12.5	3.748502
12.66667	3.942048
12.83333	4.221321
13	4.478392
13.16667	4.631721
13.33333	4.690969
13.5	4.70268
13.66667	4.703316
13.83333	4.70985
14	4.727952
14.16667	4.761673
14.33333	4.813999

14.5	4.882728
14.66667	4.963463
14.83333	5.050848
15	5.142869
15.16667	5.250101
15.33333	5.346433
15.5	5.440368
15.66667	5.531859
15.83333	5.607444
16	5.671848
16.16667	5.736785
16.33333	5.795597
16.5	5.843076
16.66667	5.880293
16.83333	5.906179
17	5.917976
17.16667	5.917687
17.33333	5.906859
17.5	5.884793
17.66667	5.853353
17.83333	5.810894
18	5.761385
18.16667	5.701399
18.33333	5.63595
18.5	5.569046
18.66667	5.501935
18.83333	5.419336
19	5.329371
19.16667	5.235523
19.33333	5.138614
19.5	5.023588
19.66667	4.923669
19.83333	4.822715
20	4.717747
20.16667	4.614311
20.33333	4.51224
20.5	4.415397
20.66667	4.313693
20.83333	4.216771
21	4.132327
21.16667	4.047659
21.33333	3.971339
21.5	3.897073
21.66667	3.823619

21.83333	3.747824
22	3.684138
22.16667	3.624553
22.33333	3.579498
22.5	3.53382
22.66667	3.49125
22.83333	3.452202
23	3.42098
23.16667	3.392863
23.33333	3.366015
23.5	3.34269
23.66667	3.32115
23.83333	3.299946
24	3.282707
24.16667	3.268563
24.33333	3.24963
24.5	3.231318
24.66667	3.209002
24.83333	3.188999
25	3.170405
25.16667	3.150327
25.33333	3.134391
25.5	3.108566
25.66667	3.088754
25.83333	3.069761
26	3.052969
26.16667	3.032605
26.33333	3.014653
26.5	2.99549
26.66667	2.979256
26.83333	2.958658
27	2.943942
27.16667	2.932129
27.33333	2.920719
27.5	2.910524
27.66667	2.900152
27.83333	2.891231
28	2.880179
28.16667	2.872303
28.33333	2.863032
28.5	2.854908
28.66667	2.846177
28.83333	2.841058
29	2.835117

29.16667	2.829418
29.33333	2.823845
29.5	2.819174
29.66667	2.814257
29.83333	2.807929
30	2.80383
30.16667	2.801229
30.33333	2.796607
30.5	2.791747
30.66667	2.787362
30.83333	2.785198
31	2.783024
31.16667	2.780663
31.33333	2.779669
31.5	2.777079
31.66667	2.774664
31.83333	2.77076
32	2.772107
32.16667	2.770415
32.33334	2.768073
32.5	2.767014
32.66667	2.76712
32.83334	2.767138
33	2.766491
33.16667	2.765146
33.33334	2.763482
33.5	2.763458
33.66667	2.762593
33.83334	2.761094
34	2.761357
34.16667	2.761576
34.33334	2.761769
34.5	2.762032
34.66667	2.761481
34.83334	2.76216
35	2.760837
35.16667	2.760709
35.33334	2.759771
35.5	2.760586
35.66667	2.760373
35.83334	2.760607
36	2.760662

Two sensitivity checks completed below for 100yrCC scenario

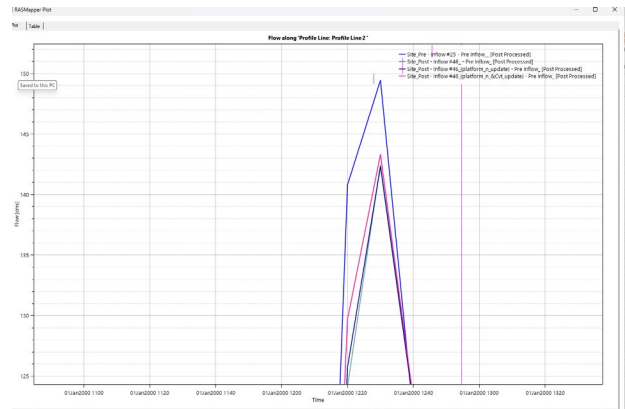
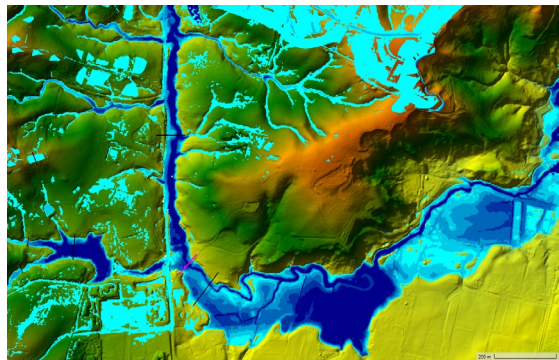
1 Update manning of CSL building platforms to 0.03

Results - Negligible impact on peak flows

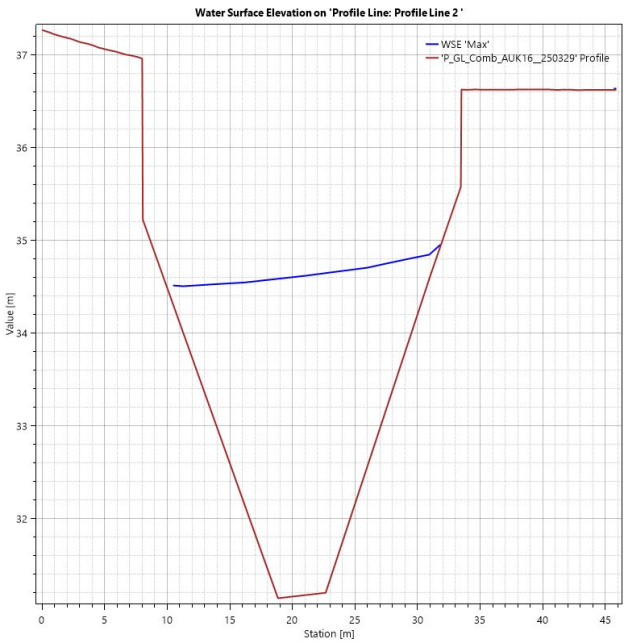
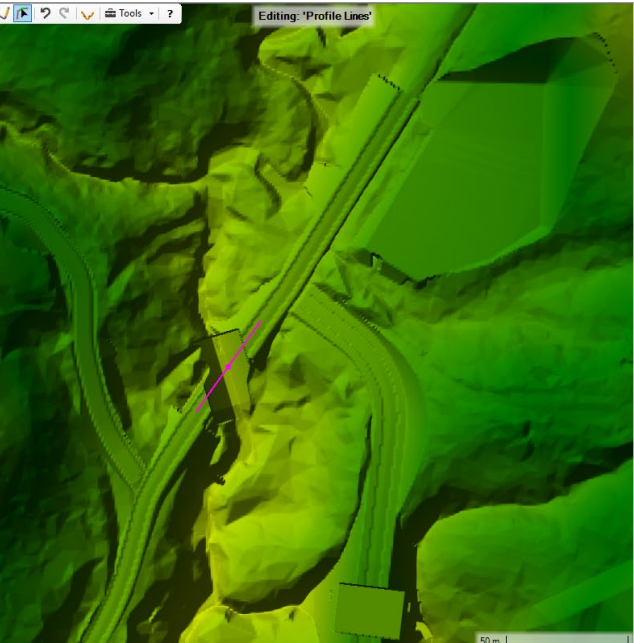
2 Update model to include Culverts 13-4, 14-1, 6-6 and updated culvert 2 to 1.5m x 1.5m box shape culvert

Results-addition of culvert 6-6, 14-1,13-4 results in peak flow at cross section (in figure below) increase of 0.1% from 142.48 to 142.62 m3/s, the peak flow is notably still 6.84 m3/s below predevelopment (149.46 m3/s).

CULVERT No.	TYPE	Plan SIZE	Model	Embedment
1	BOX CULVERT	5m x 2m	5m x 1.5m	0.5m
2	BOX CULVERT	1.5m x 2m	1.5m x 1.5m	0.5m
3	BOX CULVERT	4m x 2m	4m x 1.5m	0.5m
4	BOX CULVERT	6m x 2m	6m x 1.5m	0.5m
5	BOX CULVERT	2m x 1.5m	2m x 1m	0.5m
6	BOX CULVERT	1.5m x 2m	1.5m x 1.5m	0.5m
7	PRIMARY-CIR-CULVERT	0.6mø	0.6	0.15m
COUNTRYSIDE				
13-4	CIR-CULVERT	1.20mø	1.20mø	0
14-1	BOX CULVERT	1.5m x 1.5m	1.5m x 1.5m	0
6-6	CIR CULVERT	1.05mø	1.05mø	0
1-1	PRIMARY-CIR-CULVERT	1.20mø		
1-1	SECONDARY-BOX CULVERT	4m x 1.5m		



Screenshot of proposed bridge terrain modification replacting 2.6m existing culvert



Sensitivity checks for 100yrCC scenario CSL (post-development) with no upgrade to Forestry Road with no climate change
1 CSL (post-development) with no upgrade to Forestry Road.
Results - Negligible impact on peak flow - Post development peak flow = 67.405 Predevelopment peak flow at profile line 2 = 67.042 m3/s (increase of 0.5%)

