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PROPOSED RESIDENTIAL SUBDIVISION, RETIREMENT VILLAGE AND SOLAR FARMS

STATION ROAD, MATAMATA

GEOTECHNICAL INVESTIGATION REPORT

Matamata Development Limited C/O Maven Associates

HAM2023-0124AI Rev 1



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EXECUTIVE SUMMARY

This report presents the results of geotechnical investigations and geohazards assessment for a proposed residential subdivision, retirement village and solar farms along Station Road, Matamata.

Based on the investigation results, the site is underlain by Hinuera Formation and Walton Subgroup soils.

Geotechnical recommendations for the proposed development are summarised as follows:

- Liquefaction analyses indicate the following liquefaction-induced settlement during a ULS event:
 - o Between 75mm to 155mm for IL2 structures.
 - Between 90mm to 150mm IL3 structures.
- There is high potential for lateral spreading near the riverbank.
- Load induced settlements based on cut and fill levels indicate the following:
 - Between 5mm to 20mm primary settlement for single storey structures.
 - o Between 15mm to 40mm total settlement single storey structures.
 - Settlement at the solar farms was negligible.
- TC2 foundations will be required at the Retirement Village and Residential Subdivision except at the sloping hill underlain by Walton Subgroup which will require TC1 foundations.
- The predominantly stiff and non-sensitive silt and clay ashes across the rolling hills to depths of nominally 2m to 3m should be suitable for re-use as engineered fills with appropriate moisture conditioning and compaction. Excavation of these materials will be readily achieved with normal earthworks plant, such as scrapers and bulldozers with scoops. Below those depths, sensitive Puketoka silts are susceptible to rapid strength loss when disturbed and will require significantly more conditioning to remove and compact.



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1 INTRODUCTION

CMW Geosciences (CMW) was engaged by Matamata Development Limited C/O Maven Associates to prepare a Geotechnical Investigation Report for a site located at Station Road, Matamata, which is being considered for the development of a residential subdivision, a retirement village and two solar farms.

The scope of work and associated terms and conditions of our engagement were detailed in our services contract referenced HAM2023-0124AH VO1 Rev0, dated 29 April 2025.

This report presents factual data, presents geotechnical assessments, and recommendations for managing geotechnical risks, including possible mitigation measures, to support a Resource Consent application to Matamata-Piako District Council.

2 SITE DESCRIPTION

2.1 Site Location

The site covers multiple legal lots as explained below and as shown on Figure 1:

- Solar Farm A legally described as LOT 2 DP 567678, with an area of approximately 13.5 ha.
- Solar Farm B legally described as PT Lot 1 DP 21055, Lot 2 DP 32066 and Lot 3 DPS 14362, with an area of approximately 30 ha.
- Residential Subdivision legally described as Lot 1 DPS 65481, Lot 5 DP 384886, Lot 204 DP 535395, Lot 4 DP 384886 and Lot 3 DPS 14362. With an area of approximately 42.8Ha
- Retirement Village legally described as Lot 2 DP 21055 and Lot 2 DP567678, with an area of approximately 20 ha.

2.2 Landform

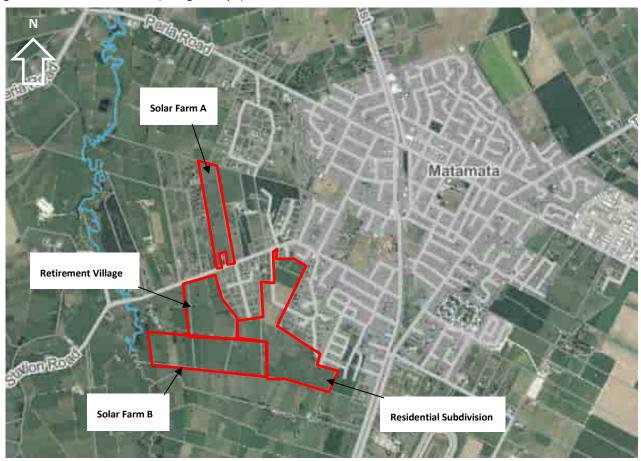
The current general landform, together with associated features located within and adjacent to the site is presented on the attached Site Plans as *Drawing 01 to 03* in *Appendix A*.

- The sites can generally be described as near level with existing ground levels ranging from RL64m (Moturiki Datum) in the northern section of Solar Farm A to RL72m in the southern section of the Residential Subdivision.
- Approximate elevation at Solar Farm B is RL68m, except close to the western boundary where the landform
 grades at approximately 1V:5H to RL59m towards the Waitoa River which runs parallel to the western
 boundary.
- Approximate elevation of the northern section of the Residential Subdivision is RL67m. There is a
 graduating sloping hill in the southwestern corner of the site with an approximate grade of 1V:10H and a
 maximum elevation of RL72m.
- Solar Farm A, the Retirement Village and the Residential Subdivision are adjoined by Station Road. Access to Solar Farm B is through farm raceways north of the proposed block.
- Existing residential developments are located to the east and west of the Residential Subdivision and to the east of the Retirement Village.
- Each section is currently utilised as pastoral land and predominantly grass covered with sporadic mature trees. Grazing stock was present during previous CMW site visits.



- Landforms and land use remains consistent in the last 50 years as observed on historical aerial photographs (Retrolens images circa 1971).
- Stormwater swales (up to 2m deep) are located along the western boundary of the Residential Subdivision, northern boundary of Solar Farm A and the eastern and western boundary of Retirement Village.
- Swale drains are currently located within each development area. The drains at the Residential Subdivision, the Retirement Village, Solar Farm A and Solar Farm B will be infilled as per drawings provided by Maven Associates.

Figure 1: Site Location Plan (Google Maps)



3 PROPOSED DEVELOPMENT

The drawings provided by Maven Associates Ref. 289001 and J00606 detail the layout for the proposed developments and are presented in *Appendix B*.

- The Residential Subdivision will have lots that range from 350m² to 800m², and will have a commercial area, and associated roads and infrastructure. Cuts up to 3.52m deep and fills up to 2.58m thick are proposed in the Residential Subdivision.
- The Retirement Village will consist of 218 single storey villas and health care facilities. A green way is proposed to collect stormwater runoff from the residential subdivision and the retirement village. The greenway is oriented in a general east to west direction along the southern boundary and flows towards the river. One stormwater pond is proposed in the northeastern corner and another stormwater pond is proposed in the southwestern corner. A maximum cut of 3.4m and fill of 2.8m is proposed at the retirement village.



• It is understood that a Solar Farm A comprises of 156 solar panel strings and Solar Farm B comprises of 110 solar panel strings as well as power transformers at each farm. At solar farm B there is a proposed water treatment plant located near the eastern boundary, close to the residential subdivision. Fill thicknesses are typically 0.5m, with maximum fill thickness of 1.5m, and cut depths are typically 0.5m.

4 INVESTIGATION SCOPE

4.1 Previous Investigations

CMW have previously conducted investigations and issued the following reports below:

- Geotechnical Investigation Report. 35-39 Peakedale Road, Station Road, Matamata. CMW Ref HAM2023-0124AB Rev 1. Dated 12 December 2023.
- Preliminary Geotechnical Investigation Report. Proposed Residential Subdivision and Solar Farm. Station Road, Matamata. CMW Ref. HAM2023-0124AE Rev 0. Dated 5 July 2024.

All fieldwork was carried out under the direction of CMW Geosciences in general accordance with the NZGS specifications¹ and logged in accordance with NZGS guidance². The scope of fieldwork completed was as follows:

- Undertook a walkover survey of the site to assess the general landform, site conditions and adjacent structures / infrastructure;
- An on-site services search was carried out by a specialist contractor to identify the presence of any
 underground obstructions or hazards prior to the field investigation program commencing;
- 33 no. hand auger boreholes, denoted HA23-01 to HA23-08, HA24-09 to HA24-25 and HA24-26 to HA24-33, were drilled using a 50mm diameter auger to target depths of up to 5.0m below existing ground levels to visually observe the near surface soil profile and to facilitate in-situ permeability / vane shear strength testing. Engineering logs of the hand auger boreholes, together with peak and remoulded vane shear strengths are presented in *Appendix C*;
- Dynamic cone (Scala) penetrometer (DCP) tests were carried out adjacent to the hand auger borehole to
 depths of up to 5m to provide soil density profiles, for use as a comparison with the CPT data and to provide
 a subgrade CBR value for pavement design purposes. These were not done for SOA24-13 to SOA24-24.
 Graphical results of the DCP testing are presented on the borehole logs in *Appendix C*;
- 24 in-situ falling head permeability tests were completed in the open standpipe piezometers denotated SOA23-01 SOA23-04, SOA24-05 to SOA24-12 and SOA24-13 to SOA24-24. Results of the permeability tests are presented in *Appendix D*;
- 12 Cone Penetrometer Tests (CPT) and four seismic CPTs denoted CPT23-01 to CPT23-03b, CPT24-04 to CPT24-10 and SCPT24-01 to SCPT-04 respectively, were pushed to depths ranging between 4.06m to 30m to define the ground model at depths. Results of the tests are presented as traces of tip resistance (q_c), friction resistance (f_s) and friction ratio are presented in **Appendix E**;
- Groundwater monitoring of five piezometers within hand auger holes took place between 10 December 2024 to 24 January 2025. HA24-26 had silted up at 0.7m as had the shallow piezometer at HA24-33 at 1.8m.
 Water was measured at HA24-33 at 1.35m in December 2024 and 2.2m in January 2025. HA24-28 was

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¹ NZ Geotechnical Society (2017) NZ Ground Investigation Specification, Volume 1 – Master Specification.

² NZ Geotechnical Society (2005), Field Description of Soil and Rock, Guideline for the field classification and description of soil and rock for engineering purposes.



dipped dry to 3.11m depth and HA24-30 was dipped dry to 4.87m depth in both December 2024 and January 2025. HA24-32A was dipped dry at 1.76m in January 2025.

The approximate locations of the respective investigation sites referred to above are shown on the Site Plans presented within Appendix A. Test locations were recorded using handheld GPS.

GROUND MODEL

5.1 **Published Geology**

Published geological maps³ for the area depict the regional geology as comprising cross-bedded pumice sand, silt and gravel of the Hinuera Formation.

Stratigraphic Units 5.2

The ground conditions encountered and inferred from the investigations were generally consistent with the published geology for the area as well as CMW's aforementioned reports (HAM2023-0124AB and HAM2023-0124AE) and can be generalised according to the following subsurface sequences.

The distribution of the various units encountered is presented in the appended Geological Sections presented as **Drawings 4 to 7** within **Appendix A**.

5.2.1 Summary

The distribution of these units is illustrated on the appended Geological Cross Sections and presented below in Table 1.

Table 1: Summary of Strata Encountered

The is	Depth to	o base (m)	Thickness (m)**	
Unit	Min	Max	Min	Max
Topsoil/Fill	0.1	0.5	0.1	0.5
Firm to Stiff Silt/Sandy Silt	0.8	1.2	0.5	0.9
Stiff to Very Stiff Silt (Hinuera Formation)	1.0	1.2	0.5	1.0
Loose to Medium Dense Sand/Silty Sand (Hinuera Formation)	1.4	2.5	0.6	1.7
Dense to Very Dense Sand with interbedded Silt (Hinuera Formation)	5.9	17.3	4.9	16.3
Very Stiff to Hard Silt/Clay (Walton Subgroup)	0.1	18.1	9*	18*
Very Dense Silty Sand (Walton Subgroup)	-	-	**	**

³ Edbrooke, S.W (compiler) 2005: Geology of the Waikato area. Institute of Geological & Nuclear Scienced 1:250,000 Geological Map 4.



5.3 Groundwater

During the investigations, which were completed in late spring (November 2023) at the residential subdivision and early winter conditions (May / June 2024) for the remaining development, groundwater was encountered within the CPTs and boreholes at the depths provided in Table 2. It is noted that groundwater levels do fluctuate seasonally and following periods of heavy / extended periods of rain by up to a metre. Interpreted groundwater levels are presented in Table 2.

Table 2: Inferred Groundwater Table

Area	Perched Water Table RLm (m)	Global Water Table RLm (m)
Residential Subdivision	65.5 - 66.4 (1.0m – 2.5m)	63.2 - 64.5 (3.5m – 8.3m)
Solar Farm A	-	63.1 - 64.0 (1.8m - 4.2m)
Solar Farm B	66.2 - 66.5 (1.3m – 1.8m)	63.3 - 65.2 (3.3m - 3.8m)
Retirement Village	-	64.8 - 65.3 (1.6m – 2.6m)

6 GEOHAZARDS ASSESSMENT

6.1 Context

Section 106 of the Resource Management Act (RMA) requires an assessment of the risk from natural hazards to be carried out when considering the granting of a subdivision consent. S106 RMA specifically states that the assessment must consider the combined effect of the natural hazard likelihood and material damage to land or structures (consequence).

The following sections of this report provide an assessment of the geohazards relevant to this site and provide the basis for the Natural Hazards Risk Assessment presented in *Appendix F*.

6.2 Seismicity

Reference to NZGS Guidance⁴ was made to determine peak horizontal ground acceleration or PGA (a_{max}) values based on a 50-year design life in accordance with the New Zealand Building Code⁵ and importance level (IL) 2 for the residential and retirement village development and IL3 structures for the proposed solar farms. The PGA values for the serviceability limit state (SLS) and ultimate limit state (ULS) earthquake scenarios are as follows:

Table 3: Design Peak Ground Acceleration (PGA) for Various Limit States

Limit State	AEP	PGA(g) Magnitude _{eff}				
IL2 Structures						
SLS	1/25	0.07	5.9			
ULS	1/500	0.28	5.9			
IL3 Structures						
SLS	1/25	0.07	5.9			

⁴ NZ Geotechnical Society publication "Earthquake geotechnical engineering practice, Module 1: Overview of the standards", (March 2016).

⁵ Ministry of Business, Innovation and Employment (1992) NZ Building Code Handbook, Third Edition, Amendment 13 (effective from 14 February 2014)



Limit State	AEP	PGA(g)	Magnitude _{eff}
ULS	1/1,000	0.36	5.9

6.3 Fault Rupture

The nearest active fault to the site is the Kerepehi Fault. This fault is approximately 5km east of the site. The Kerepehi Fault has a recurrence interval of between 2,000 years to 3,500 years. We consider the site to be low risk, with respect to fault rupture.

6.4 Liquefaction

6.4.1 Methodology

In accordance with MBIE/NZGS guidance⁶ the liquefaction susceptibility of the soils at this site was assessed with respect to geological age and compositional (soil fabric and density) criteria, based on the following assumptions:

- Saturated soils below 1m to 4m below ground level (bgl) were modelled as being susceptible to liquefaction. Saturated soils below 4.75m to 6m bgl were modelled at CPT24-09 and CPT23-3b.
- A site-specific assessment was carried out using the seismic CPTs to account for soil microstructure in accordance with Robertson⁷. Results in *Appendix G* suggest that "no soil microstructure can be justified" and therefore no strength gain factor has been applied.
- Soils are also classified with respect to their grain size and plasticity to assess liquefaction susceptibility. For this project, a cut-off threshold soil behaviour type index value (I_c) of 2.6 was used to distinguish between liquefiable (I_c<2.6) and non-liquefiable (I_c>2.6) soils.
- Specific liquefaction analyses were undertaken for IL2 and IL3 structures, using the software package CLiq using the Boulanger and Idriss (2014) method. The cyclic stress ratio (CSR), being a function of the earthquake magnitude for the design return period event, was compared to the cyclic resistance ratio (CRR), being a function of the CPT cone resistance (q_c) and friction ratio (F_r).
- Free-field liquefaction induced settlements were determined in accordance with Zhang et al. (2002). With respect to liquefaction response, consideration was given to a 10m cut-off depth to estimate index settlements as per MBIE⁸ guidance (foundation technical categories). These were compared to liquefaction settlement estimates over the full depth range of the CPT's with a depth weighting factor ranging from 1 at the ground surface to 0 at 18m depth applied to the volumetric strains (e_v) in accordance with Cetin et al (2009)⁹.
- Proposed fill and cut levels have been considered at each CPT location. This is based on the cut and fill drawings supplied by Maven Associates.

6.4.2 Results

Results are presented in *Appendix G* and can be summarised as in Table 4:

⁶ Earthquake Geotechnical Engineering Practice, Module 3: Identification, assessment and mitigation of liquefaction hazards", (November 2021)

⁷ P.K. Robertson (2015). Comparing CPT and Vs Liquefaction Triggering Methods, Journal of Geotechnical and Geoenvironmental Engineering.

⁸ Repairing and Rebuilding House affected by the Canterbury Earthquakes", (December 2012)

⁹ Cetin, K., Bilge, H., Wu, J., Kammerer, A., and Seed, R. (2009). Probabilistic Model for the Assessment of Cyclically Induced Reconsolidation (Volumetric) Settlements, Journal of Geotechnical and Geoenvironmental Engineering, Vol. 135(3), pp. 387-398.



Table 4: Liquefaction Analyses Results

	SLS	IL2 Str	uctures	IL3 Structures		
Development	Settlement (mm)	Total Settlement (mm)	Index Settlement (mm)	Total Settlement (mm)	Index Settlement (mm)	
Retirement Village	<5	120	85 - 90	-	-	
Residential Subdivision	<5 75	75 - 155	30 - 95	-	-	
Solar Farm A	<5	-	-	100 - 145	85 - 150	
Solar Farm B	<5	-	-	90 - 150	95 - 165	

Note: All settlements and depths based on existing ground profile.

Index settlements are calculated based on the upper 10m of the soil profile using no depth weighting factor.

Total ULS settlements are based on the full depth of the CPT trace with a depth weighting factor applied.

Index settlements are for assessment of the site against the MBIE site Technical Category guidelines and are not comparable to the total ULS settlements.

The calculations indicate that liquefaction may occur in some soil layers during a ULS earthquake event. In the ULS cases, the liquefaction results indicate a high risk of liquefaction occurring at the site. Recommendations to mitigate effects of liquefaction settlements on the proposed development are provided below in Section 7.

6.5 Lateral Spread

Following the onset of liquefaction, the liquefied soils behave as a very weak undrained material, which can give rise to lateral spreading where a free face is present within the vicinity of the site or where proposed cut and fill batters are proposed over or within liquefied soils.

The existing drains (assumed to be 2m deep) at the Residential Subdivision, the Retirement Village, Solar Farm A, Solar Farm B and the proposed greenway (assumed to be 2m deep) will be infilled as per drawings provided by Maven Associates. Hence, lateral spreading is not considered to be an issue.

The riverbank along the western boundary of the Solar Farm B is approximately 5m high. Based on the current landform, free face height and continuity of liquefiable layers, there is high potential for lateral spreading.

6.6 Slope Stability

The general landform across the site is flat to gently grading, therefore we do not consider slope stability will be problematic on this site.

6.7 Load Induced Settlement

Static settlements were estimated based on the CPT data using the methodology outlined below. Calculations were undertaken using the commercially available software CPeT-IT.

$$S = \sum \frac{\Delta \sigma_v}{M_{CPT}} \Delta z$$

Where $\Delta \sigma_v$ = change in total vertical stress, Δz = layer thickness, M_{CPT} = constrained modulus estimated from the CPT data as follows:

$$M = \alpha_M (q_t - \sigma_{vo})$$



Where: α_M = factor, derived according to Robertson (2009)¹⁰; q_t = cone resistance; σ_{vo} = total vertical stress.

Table 5 below shows estimated foundation settlements. The dwellings and villas for the residential subdivision and retirement village are assumed to be light weight single story structures, as is the medical centre proposed at the retirement village. Fill load has been added to the CPTs based on the cut and fill drawings for each development. Out has been presented in *Appendix H*.

Table 5: Load Induced Settlement Results.

Development	Cut/Fill	Applied Working Pressure (kPa)	Foundation Dimensions	Primary Settlement (mm)	Total Settlement (mm)
Residential Subdivision and Retirement Village	3.52m cut to 2.8m fill	10 kPa	10m x 15m	<5 to 20	<15 to 40
Medical Centre	+1.5m Fill	10 kPa	100m x 150m	3	45
Solar Farms A and B	-	1 kPa	1m x 1m*	<5	<5

Notes: Fill weight = 18kN/m³. Assumed 0.5m embedment depth for foundations. * Piled foundations are likely to be used at the solar farms.

Settlement estimates are based on CPTs at discrete locations and should be updated as part of the detailed design.

New Zealand Building Code, Clause B.1.0.2 of B1/VM4 provides the following differential settlement criteria for design of shallow foundations 'Foundation design should limit the probable maximum differential settlement over a horizontal distance of 6m to no more than 25mm under serviceability limit state load combinations of AS/NZS 1170 Part 0, unless the structure is specifically designed to prevent damage under a greater settlement.'

For the foundation size and working load combinations considered in Table 5, settlement across the site is expected to be relatively uniform and differential settlement is expected to be below the limit set by the New Zealand building code.

6.8 Sensitive Soils

The Hinuera Formation silt unit present across the site and encountered within the upper 1m is typically considered moderately sensitive to sensitive. These characteristics may make the silt unit challenging to earthwork and will require special consideration to plant movements during the construction period where exposed. Further recommendations are provided in Section 7.7 below.

7 RECOMMENDATIONS

7.1 Seismic Site Subsoil Category

The geological units encountered beneath the development areas comprise soil strength materials, which with respect to the seismic site subsoil category defined in Section 3.1.3 of NZS1170.5, is defined as having a UCS < 1MPa. Therefore, the seismic site subsoil category is assessed as being Class D (deep soil site).

¹⁰ Robertson, P.K., 2009. Interpretation of cone penetration tests – a unified approach. Canadian Geotechnical Journal, 46:1337-1355.



7.2 Liquefaction Mitigation

Based on the analysis results presented in Section 6.4, we consider the risk of liquefaction and liquefaction induced settlements to be moderate for the ULS cases. Foundation recommendations are provided in Section 7.6.

Liquefaction effects can possibly be reduced by performing laboratory testing to assess the fines content and plastic nature of the fine-grained soils at the site. Seismic CPTs can also be performed to assess microstructure of the Hinuera Formations and Walton Subgroup soils in detail.

7.3 Lateral Spread Mitigation

Based on the analysis results presented in Section 6.4 and Section 6.5, we consider the risk of lateral spreading to be high near the riverbank.

Appropriate setbacks are to be provided at the detailed design stage. Alternately, ground improvement in the form of rigid inclusions to at least 11m depth from the existing ground levels might be required based on the severity of lateral spreading.

Lateral spreading can possibly be reduced by performing laboratory testing to assess the fines content and plastic nature of the fine-grained soils at the site. Seismic CPTs can also be performed to assess microstructure of the Hinuera Formations and Walton Subgroup soils in detail.

7.4 Static Settlement

To minimise post construction static ground settlements for the residential subdivision and retirement village, a range of options may be considered, including the following:

- In our experience, preliminary settlement estimates from CPT testing can be conservative and reviewing settlement monitoring data may assist in optimising the calculated static settlement values. Hence, we recommend a fully instrumented trial embankment or fill monitoring during bulk earthworks. Adequate time must be available to enable a full assessment of settlement trends. Settlement plates will be required to monitor settlement during the placement of temporary surcharge or pre-load fill. Settlement will be considered adequately mitigated once T₉₀ settlements are achieved, and the resulting creep settlements are predicted to be below 25mm over the design life of the proposed structures (residential dwelling loads).
- Ground improvements are a feasible option, however this solution will be expensive based on the extent of the treatment area.

7.5 Stormwater Soakage

24 no. falling head permeability tests were undertaken across the sites to provide soakage rates. Results indicated that the permeability of soils ranged between 2 x 10^{-6} and 5 x 10^{-6} m/sec for the silty material and between 7 x 10^{-6} to 6 x 10^{-7} m/sec for the sandier material. HAS24-12 has not been considered based on low soakage rate for the insitu sandy soil. Results of testing are presented as **Appendix D**. The soils at this site are considered suitable to provide rain gardens / attenuation ponds.

7.6 Foundations

On this site, our provisional expectation is that provided earthworks are completed in accordance with the standards, the following will apply:

• A preliminary geotechnical ultimate bearing pressure of 300 kPa should be available in the static case for shallow strip and pad foundations constructed within both the natural cut ground and engineered fill areas.



Geotechnical ultimate bearing pressure in the ULS seismic case will be < 300 kPa based on the shallow liquefiable layers.

- There may be areas where localised variations in shear strength within the natural cut ground occur, particularly where the depth of cut varies across the building platforms. Further confirmation of available bearing pressures will be addressed at the time of post earthworks soil testing and will be presented in the Geotechnical Completion Report.
- To accommodate the liquefaction potential, TC2 foundations are anticipated for residential subdivision (excluding the area denoted in Drawing 09 which will be TC1) and for the retirement village based on liquefaction-induced settlements presented in Section 6.4. Foundations near the swale and the riverbank should be designed to sustain lateral spreading effects.
- Based on our experience with previous solar farm developments, solar panels are typically supported by 100-150mm UC driven steel piles embedded 3-4m into the ground. Based on the ground conditions observed at this site, driven piles will be suitable at this site. The ground conditions at the solar farms are stiff to very stiff silt within the first 1m then followed by a dense to very dense sand with interbedded silt.
- As required by section B1/VM4¹¹ of the New Zealand Building Code Handbook, the following strength reduction factors must be applied to all recommended geotechnical ultimate soil capacities in conjunction with their use in factored design load cases:
 - 0.8 for load combinations involving earthquake overstrength;
 - 0.5 for all other load combinations.

7.7 Earthworks

7.7.1 Excavatability

All earthwork activities must be carried out in general accordance with the requirements of NZS 4431¹² and the requirements of the Matamata Piako District Council Infrastructure Development Code under the guidance of a Chartered Professional Geotechnical Engineer.

The Hinuera Formation silts are sensitive to remoulding during earthworks. These soils rapidly loose strength if overworked, over trafficked or allowed to get excessively wet. Mitigation options can include avoiding directly trafficking over these soils, limiting the area exposed to water infiltration at any one time, shaping and compacting the cut surface to allow water to runoff rather than pond. Disturbed soils may regain strength if left to settle for a period of days in fine weather, or they may need to be undercut and replaced with appropriately conditioned materials.

Given the consistency of the soil units that will be encountered within the proposed earthworks cuts, it is expected that excavation of these materials will be readily achieved with normal earthworks plant, such as scrapers and bulldozers with scoops.

A shallow perched groundwater table was present between 1m to 4.2m below ground level. Should excavations encounter groundwater, underfill subsoil drains or granular drainage blanket layers may be required.

¹¹ Ministry of Business, Innovation and Employment (2019) *Acceptable Solutions and Verification Methods for NZ Building Code Clause B1 Structure*, B1/VM4, Amendment 19.

¹² NZS 4431:2022 Engineered fill construction for lightweight structures, New Zealand Standard.



7.7.2 Stockpiles

Careful consideration must be given to the location of temporary topsoil / unsuitables stockpiles to ensure that they are not located immediately above steep or unstable slopes or immediately above proposed stormwater pond excavations.

The location of all temporary stockpiles must be approved by the Geotechnical Engineer prior to placement. Where stockpiles cannot be avoided above sloping ground, they should be placed over a wide area with the height restricted under the direction of the Geotechnical Engineer.

7.7.3 Compaction

Earthfill must be placed, spread and compacted in controlled 250mm to 300mm thick (loose) lifts under the direction of a geotechnical engineer. The fill may comprise either granular or cohesive material subject to being free of any organic material and having no particles greater than 150mm diameter.

Most of the proposed cut material, including the natural and existing fill materials should be suitable for reuse as Engineer Certified Fill. Soil textures and moisture contents will however vary widely, and careful management, conditioning and compaction control will be required.

All earthfill must be placed to ensure adequate knitting of successive fill lifts by ripping any natural subgrade or fill surfaces that have become dry prior to placing the following fill lift.

7.8 Civil Works

7.8.1 Subgrade CBR

The proposed roading is shown to being in a combination of both cut and fill areas. Based on DCP results in insitu soils, a lower bound value of 3% is recommended for preliminary design. Higher CBR will be available for areas with fill.

8 SUITABILITY STATEMENT

Existing site investigations carried out are considered suitable for the assessment of geotechnical constraints and associated requirements in support of a Resource Consent application.

The post-development qualitative assessment of natural risk hazard for the site is low to medium for all hazards considered with the exception of lateral spread which will require further investigation.

Based on the findings of the geotechnical investigations, we consider that the site is suitable for the proposed development providing our recommendations are followed.

9 SAFETY IN DESIGN

The design landform requires site excavations that may include geotechnical works such as undercuts, temporary excavations, fill batters. Exposure to these works forms a significant safety risk for contractors and inspectors / testers.

In conducting our scope of work, we have considered and addressed Safety in Design (SiD) aspects relevant to our understanding of the proposed design and construction work. SiD must consider the construction, operation, maintenance, and ultimate demolition phases of the relevant works.

It is noted that CMW are focussed on design aspects, and whilst we have attempted to be comprehensive in our assessment, it is the Contractors responsibility to cover construction related risks in a more comprehensive manner (being the competent party in that respect).



Our SiD risk assessment is presented in *Appendix I*. This risk assessment must be communicated with all affected parties involved with the project and dealt with through specific on-site risk assessment plans.

10 FURTHER WORK

No further geotechnical work is required for Resource Consent Application. However, the following points need to be considered:

- Lateral spreading assessment near the riverbank.
- Preparation of an earthworks specification, followed by observations, testing, certification and preparation
 of a Geotechnical Completion Report will be required for the proposed development.
- DCP testing, CBR testing, pavement design will be required to support design of roading infrastructure at the site.
- Additional investigations and laboratory testing and analysis and/ or trial embankment may assist in reducing predicted liquefaction, lateral spreading and static settlements.
- Geotechnical analysis and reporting suitable to support building consent applications will be required.

11 CLOSURE

Additional important information regarding the use of your CMW report is provided in the 'Using your CMW Report' document attached to this report.

This report has been prepared for use by Matamata Development Limited C/O Maven Associates in relation to the Proposed Residential Subdivision, Retirement Village and Solar Farms Station Road, Matamata project in accordance with the scope, proposed uses and limitations described in the report. Should you have further questions relating to the use of your report please do not hesitate to contact us.

Where a party other than Matamata Development Limited C/O Maven Associates seeks to rely upon or otherwise use this report, the consent of CMW should be sought prior to any such use. CMW can then advise whether the report and its contents are suitable for the intended use by the other party.



USING YOUR CMW GEOTECHNICAL REPORT

Geotechnical reporting relies on interpretation of facts and collected information using experience, professional judgement, and opinion. As such it generally has a level of uncertainty attached to it, which is often far less exact than other engineering design disciplines. The notes below provide general advice on what can be reasonably expected from your report and the inherent limitations of a geotechnical report.

Preparation of your report

Your geotechnical report has been written for your use on your project. The contents of your report may not meet the needs of others who may have different objectives or requirements. The report has been prepared using generally accepted Geotechnical Engineering and Engineering Geology practices and procedures. The opinions and conclusions reached in your report are made in accordance with these accepted principles. Specific items of geotechnical or geological importance are highlighted in the report.

In producing your report, we have relied on the information which is referenced or summarised in the report. If further information becomes available or the nature of your project changes, then the findings in this report may no longer be appropriate. In such cases the report must be reviewed, and any necessary changes must be made by us.

Your geotechnical report is based on your project's requirements

Your geotechnical report has been developed based on your specific project requirements and only applies to the site in this report. Project requirements could include the type of works being undertaken; project locality, size and configuration; the location of any structures on or around the site; the presence of underground utilities; proposed design methodology; the duration or design life of the works; and construction method and/or sequencing.

The information or advice in your geotechnical report should not be applied to any other project given the intrinsic differences between different projects and site locations. Similarly geotechnical information, data and conclusions from other sites and projects may not be relevant or appropriate for your project.

Interpretation of geotechnical data

Site investigations identify subsurface conditions at discrete locations. Additional geotechnical information (e.g. literature and external data source review, laboratory testing etc) are interpreted by Geologists or Engineers to provide an opinion about a site specific ground models, their likely impact on the proposed development and recommended actions. Actual conditions may differ from those inferred to exist due to the variability of geological environments. The actual interface between materials may be far more gradual or abrupt than assumed based on the facts obtained. Nothing can be done to change the actual site conditions which exist, but steps can be taken to reduce the impact of unexpected conditions. Interpretation of factual data can be influenced by design and/or construction methods. Where these methods change review of the interpretation in the report may be required.

Subsurface conditions can change

Subsurface conditions are created by natural processes and then can be altered anthropically or over time. For example, groundwater levels can vary with time or activities adjacent to your site, fill may be placed on a site, or the consistency of near surface conditions might be susceptible to seasonal changes. The report is based on conditions which existed at the time of investigation. It is important to confirm whether conditions may have changed, particularly when large periods of time have elapsed since the investigations were performed.

Interpretation and use by other design professionals

Costly problems can occur when other design professionals develop their plans based on misinterpretations of a geotechnical report. To help avoid misinterpretations, it is important to retain the assistance of CMW to work with other project design professionals who are affected by the contents of your report. CMW staff can explain the report implications to design professionals and then review design plans and specifications to see that they have correctly incorporated the findings of this report.

Your report's recommendations require confirmation during construction

Your report is based on site conditions as revealed through selective point sampling. Engineering judgement is then applied to assess how indicative of actual conditions throughout an area the point sampling might be. Any assumptions made cannot be substantiated until construction is complete. For this reason, you should retain geotechnical services throughout the construction stage, to identify variances from previous assumption, conduct additional tests if required and recommend solutions to problems encountered on site.

A Geotechnical Engineer, who is fully familiar with the site and the background information, can assess whether the report's recommendations remain valid and whether changes should be considered as the project develops. An unfamiliar party using this report increases the risk that the report will be misinterpreted.

Environmental Matters Are Not Covered

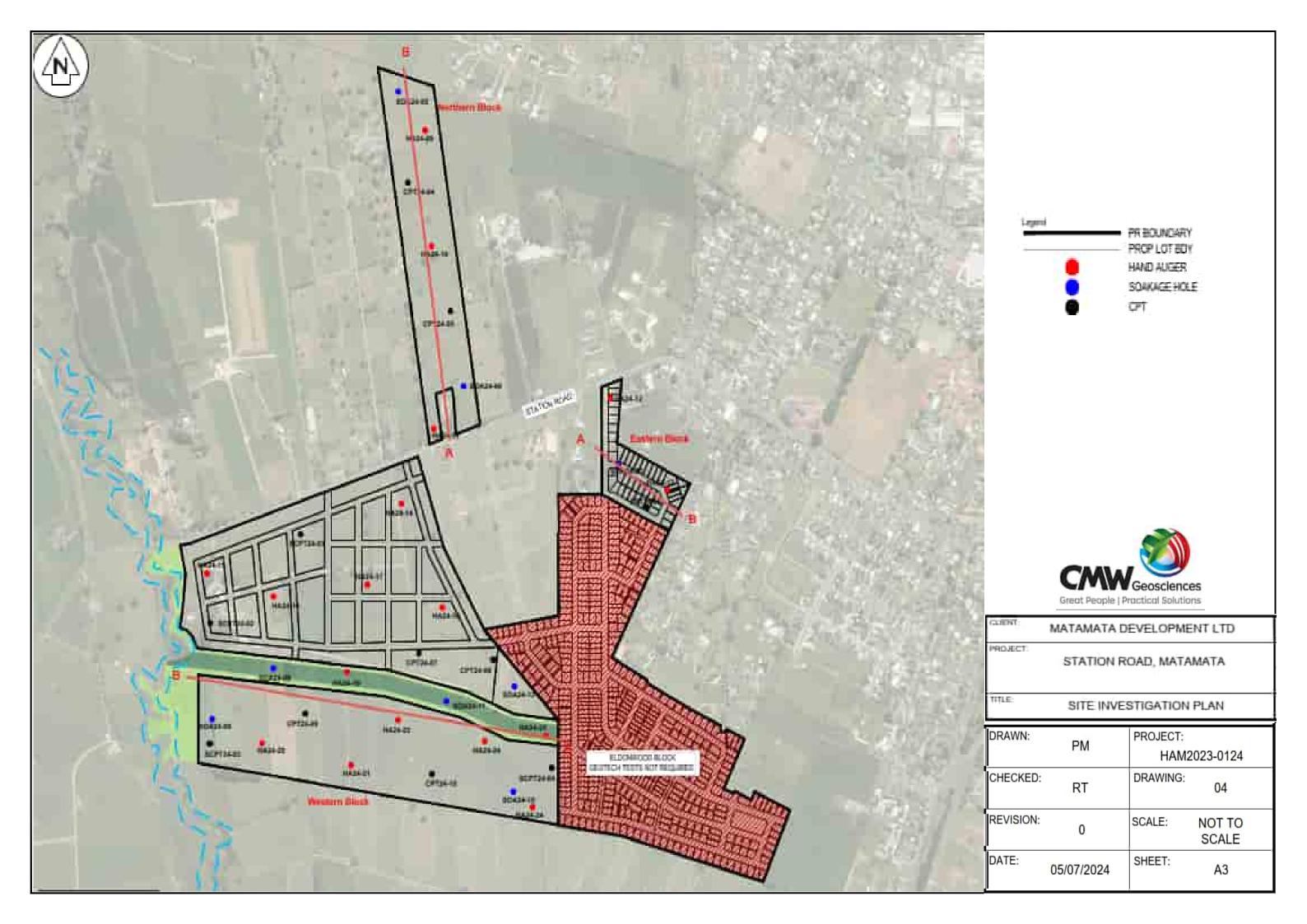
Unless specifically discussed in your report environmental matters are not covered by a CMW Geotechnical Report. Environmental matters might include the level of contaminants present of the site covered by this report, potential uses or treatment of contaminated materials or the disposal of contaminated materials. These matters can be complex and are often governed by specific legislation.

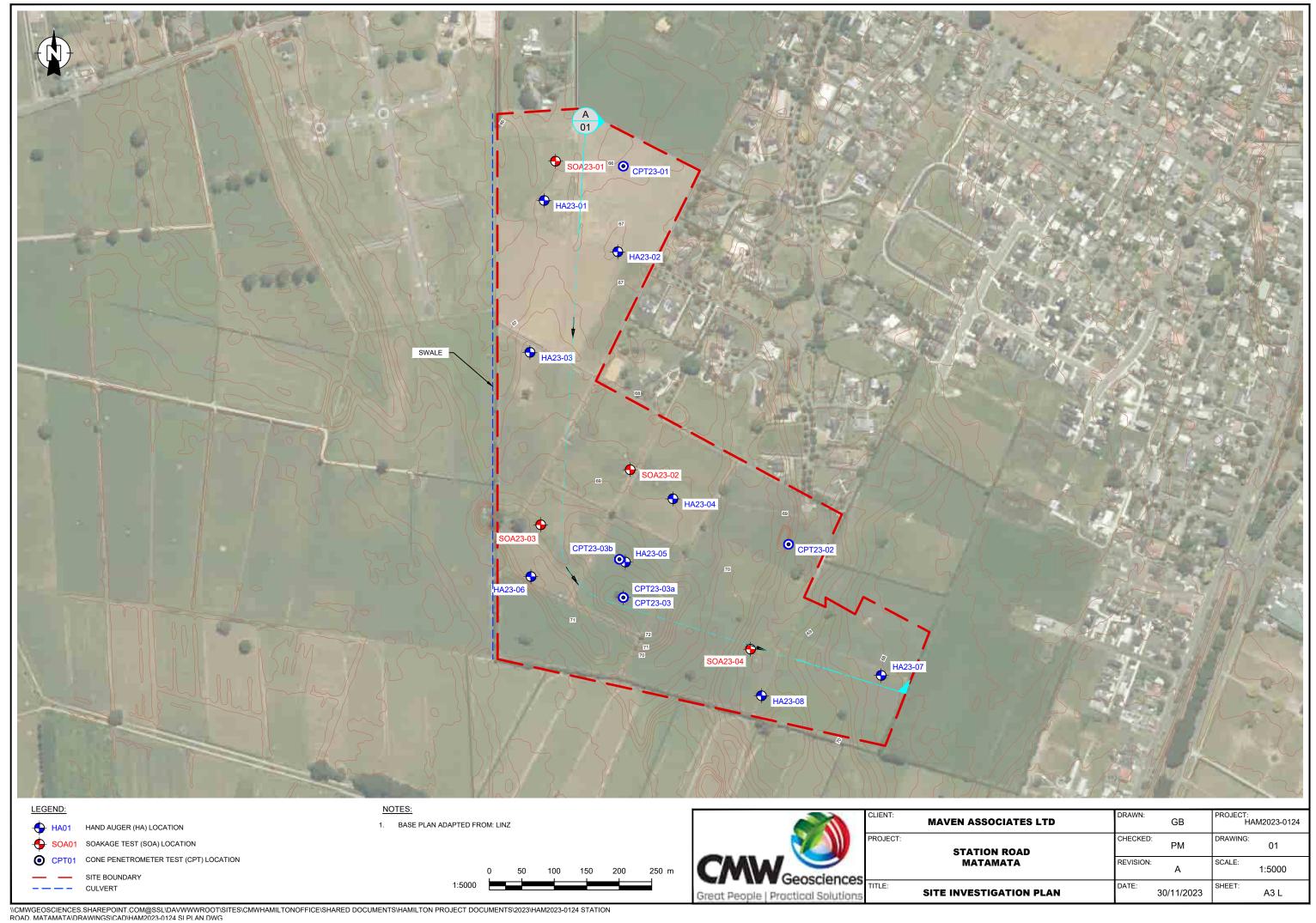
The personnel, equipment, and techniques used to perform an environmental study can differ significantly from those used in this report. For that reason, our report does not provide environmental recommendations. Unanticipated subsurface environmental problems can have large consequences for your site. If you have not obtained your own environmental information about the project site, ask your CMW contact about how to find environmental risk-management guidance.

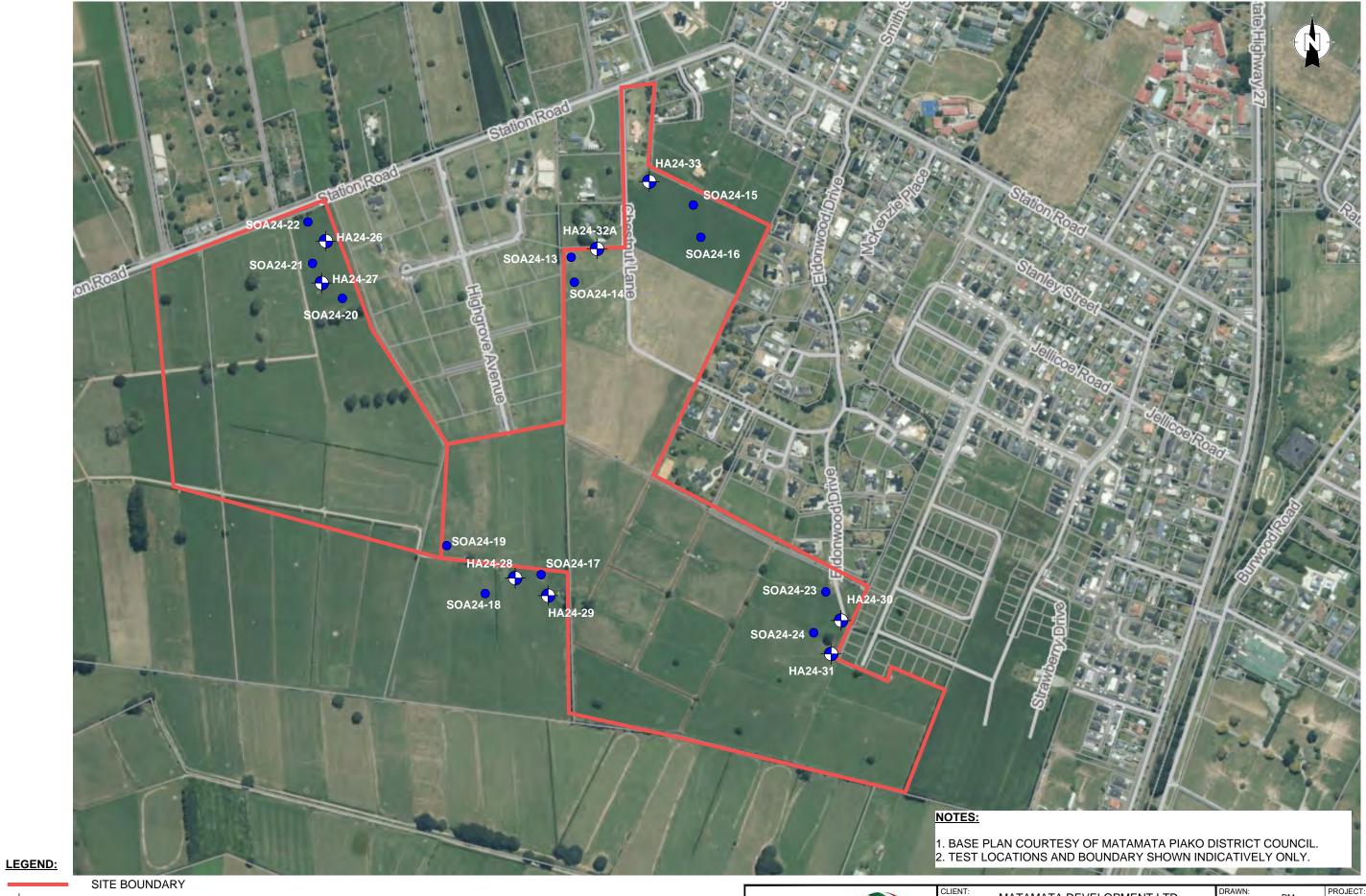


APPENDIX A: DRAWINGS

Title	Reference No.	Date	Revision
Site Investigation Plan	1	30/11/2023	0
Site Investigation Plan	2	05/07/2024	0
Site Investigation Plan	3	30/04/2025	0
Residential subdivision Landforms	4	30/11/2023	0
Cross Section A	5	04/12/2023	0
Cross Section – Eastern Block	6	05/07/2024	0
Cross Section – Northern Block	7	05/07/2024	0
Cross Section – Western Block	8	05/07/2024	0
Foundation Types Plan	9	22/05/2025	1





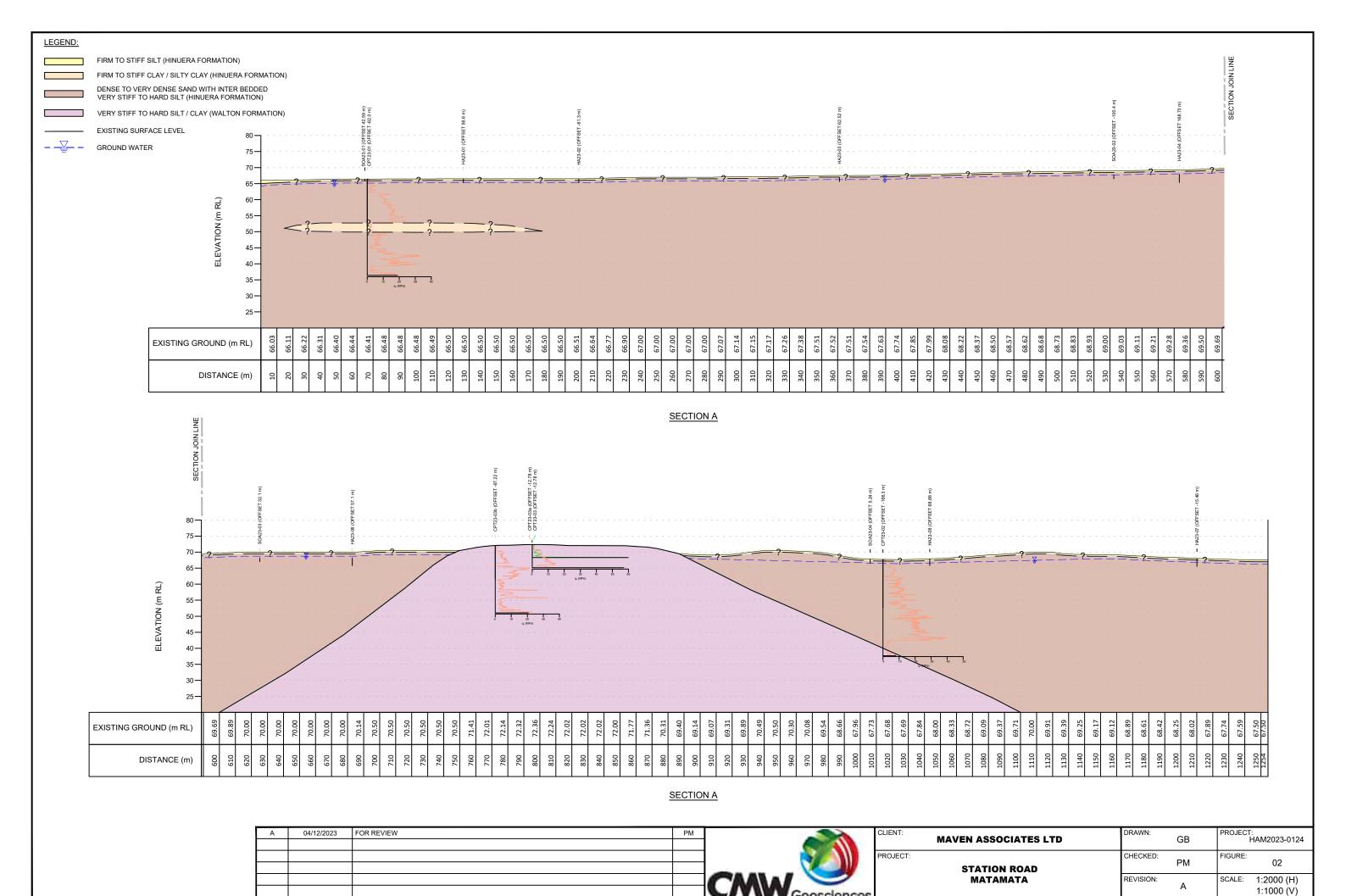


HA24-XX HAND AUGER (HA) LOCATION

DCPXX SOAKAGE TESTING (SOA) LOCATION



CLIENT: MATAMATA DEVELOPMENT LTD		DRAWN:	PM	PROJECT:	
	MATAMATA DEVELOPMENT LID		FIVI	HAM202	23-0124
PROJECT	Τ:	CHECKED:	HP	DRAWING:	03
STATION ROAD, MATAMATA			T IF	03	
		REVISION:	Α	SCALE:	NA
			^		INA
TITLE:	SITE INVESTIGATION PLAN - TO SUPPORT SOAKAGE	DATE:	30/04/2025	SHEET:	A3
			30/0-/2023		7.0



BY

Great People | Practical Solutions

REV

DATE

DESCRIPTION

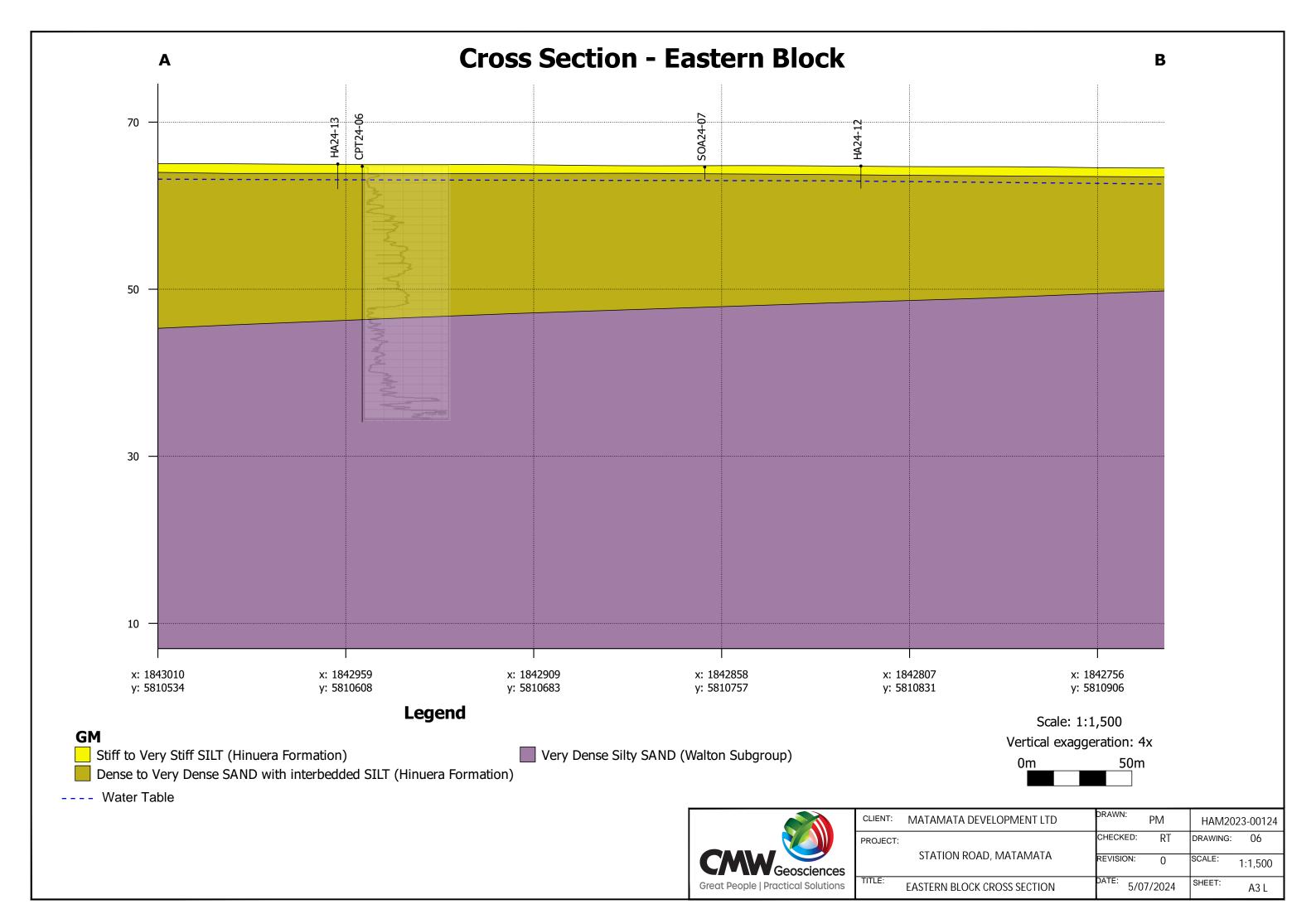
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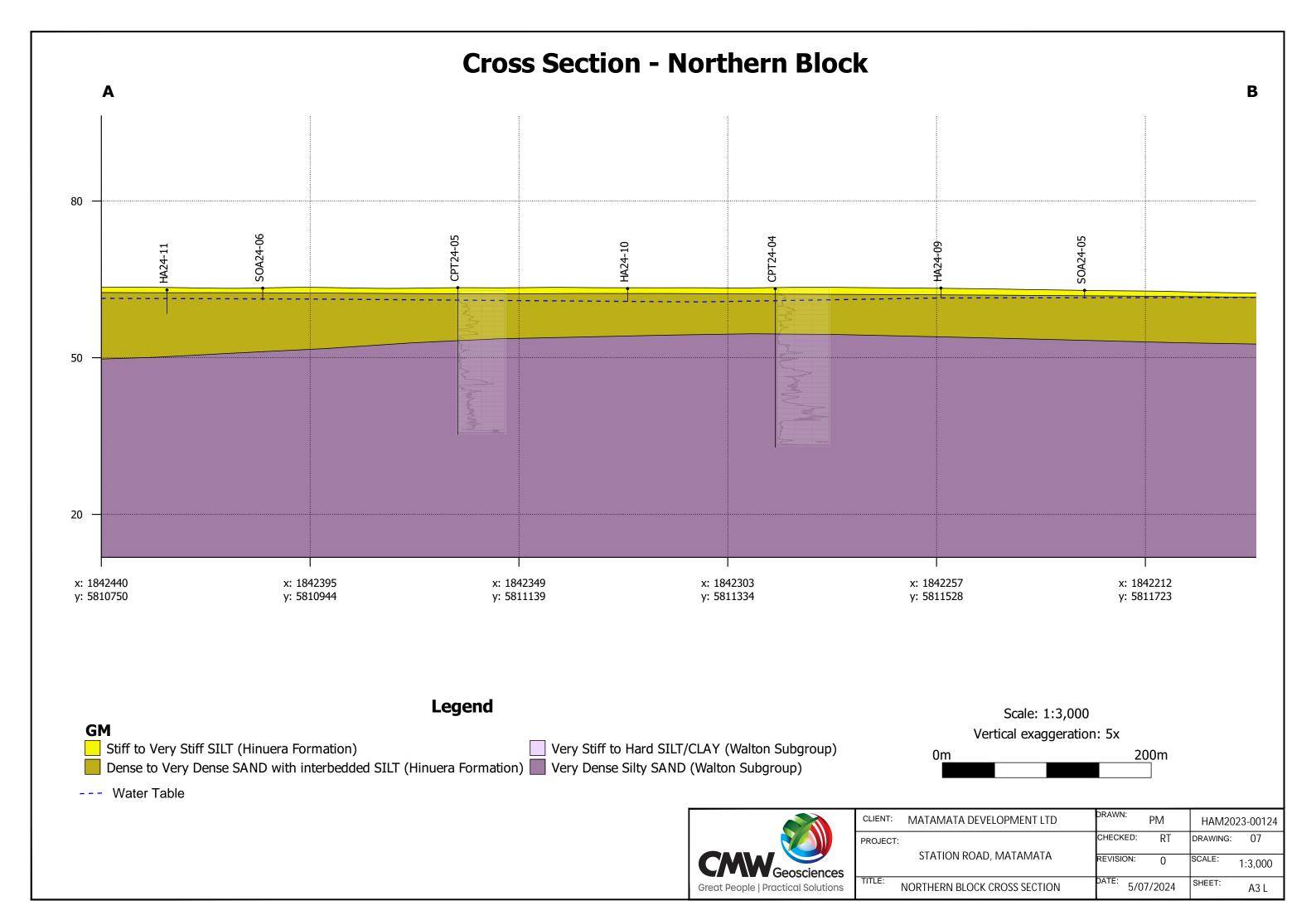
SECTION A

SHEET:

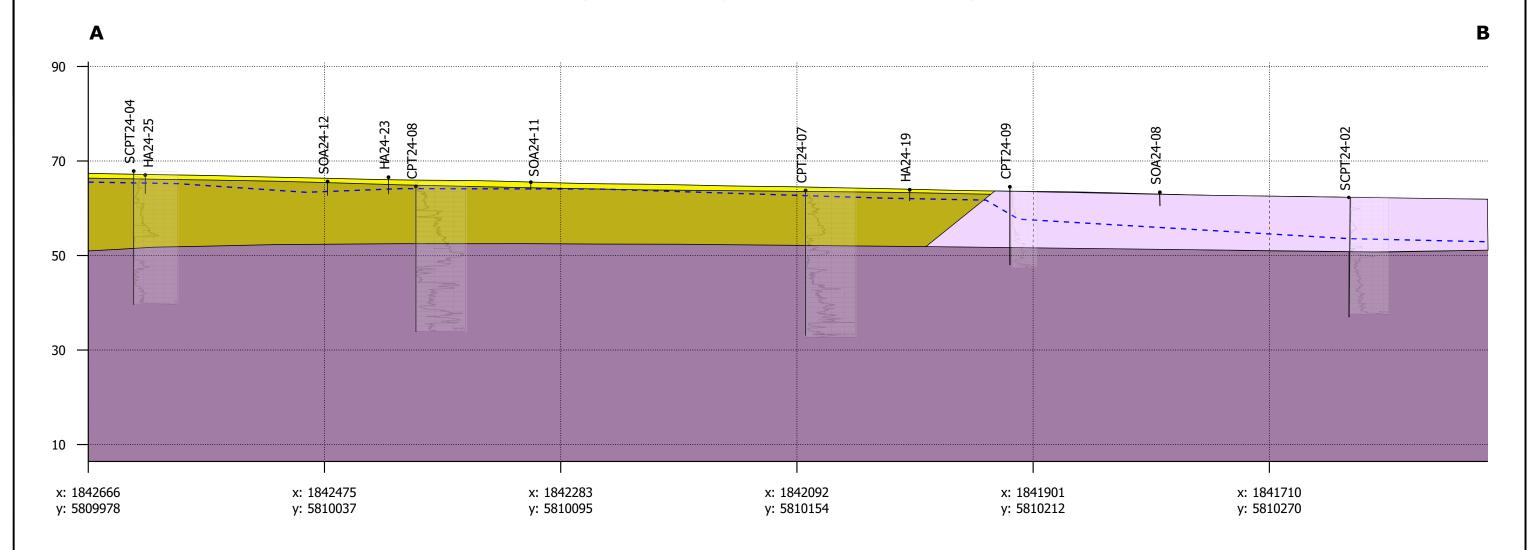
A3 L

04/12/2023

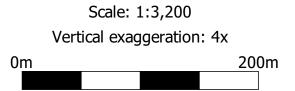




Cross Section - Western Block

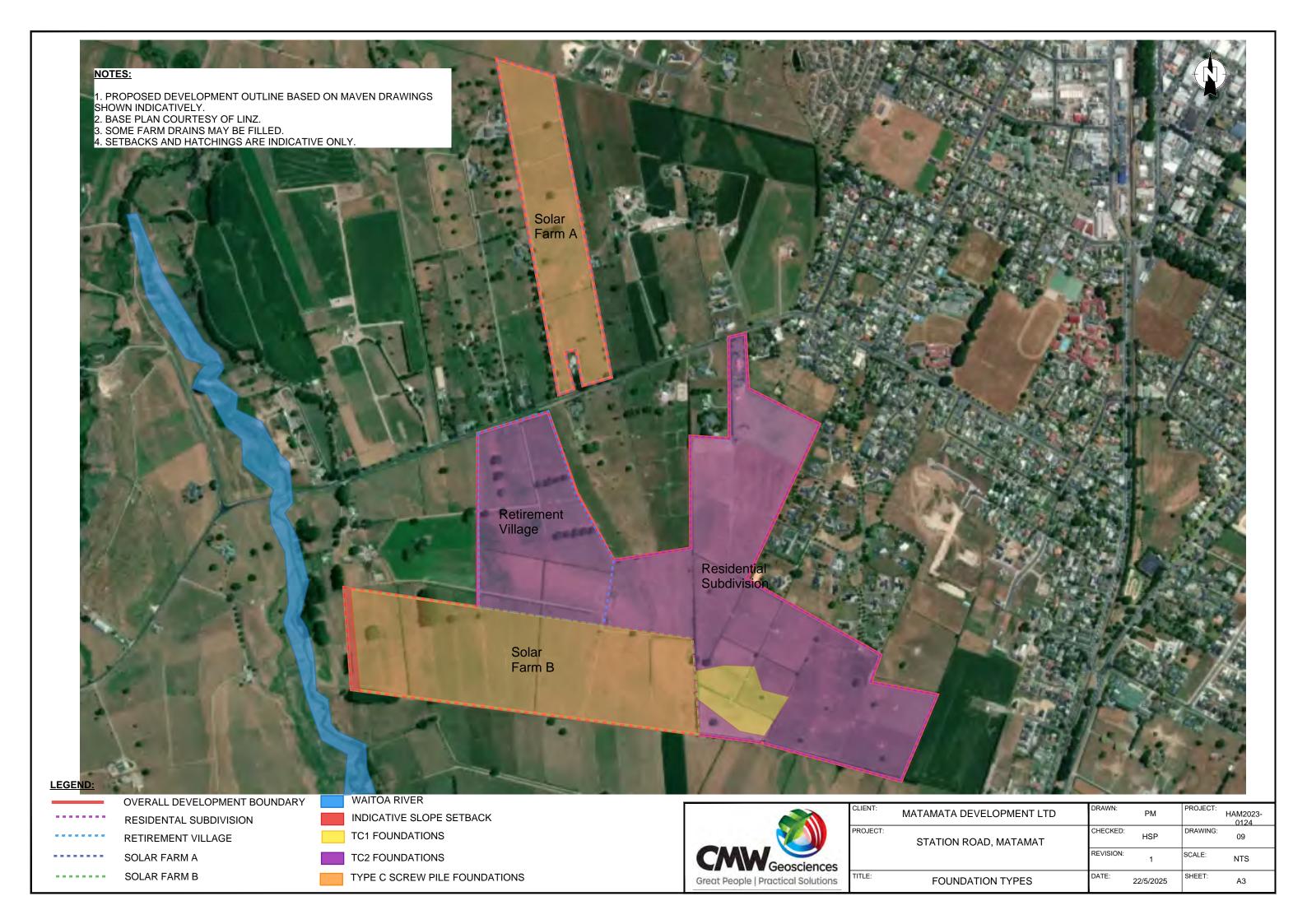


Legend GM Stiff to Very Stiff SILT (Hinuera Formation) Dense to Very Dense SAND with interbedded SILT (Hinuera Formation) Wery Stiff to Hard SILT/CLAY (Walton Subgroup) Very Dense Silty SAND (Walton Subgroup) Water Table



Geosciences	5
Great People Practical Solutions	ò

CLIENT:	: MATAMATA DEVELOPMENT LTD	DRAWN:	PM	HAM20	23-00124
PROJEC		CHECKED:	RT	DRAWING:	80
	STATION ROAD, MATAMATA	REVISION:	0	SCALE:	1:3,200
TITLE:	WESTERN BLOCK CROSS SECTION	DATE: 5/0	7/2024	SHEET:	A3 L

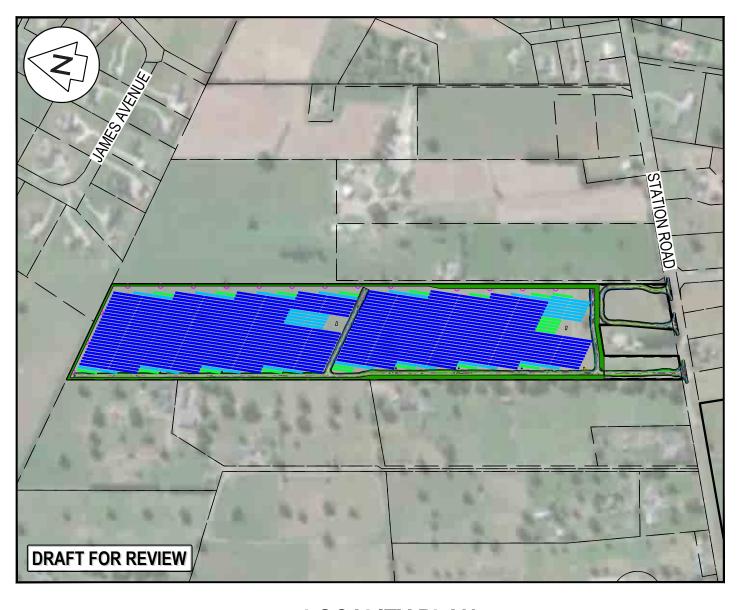




APPENDIX B: MAVEN ASSOCIATES DEVELOPMENT PLAN



ASHBOURNE SOLAR FARM A FOR MATAMATA DEVELOPMENTS LTD

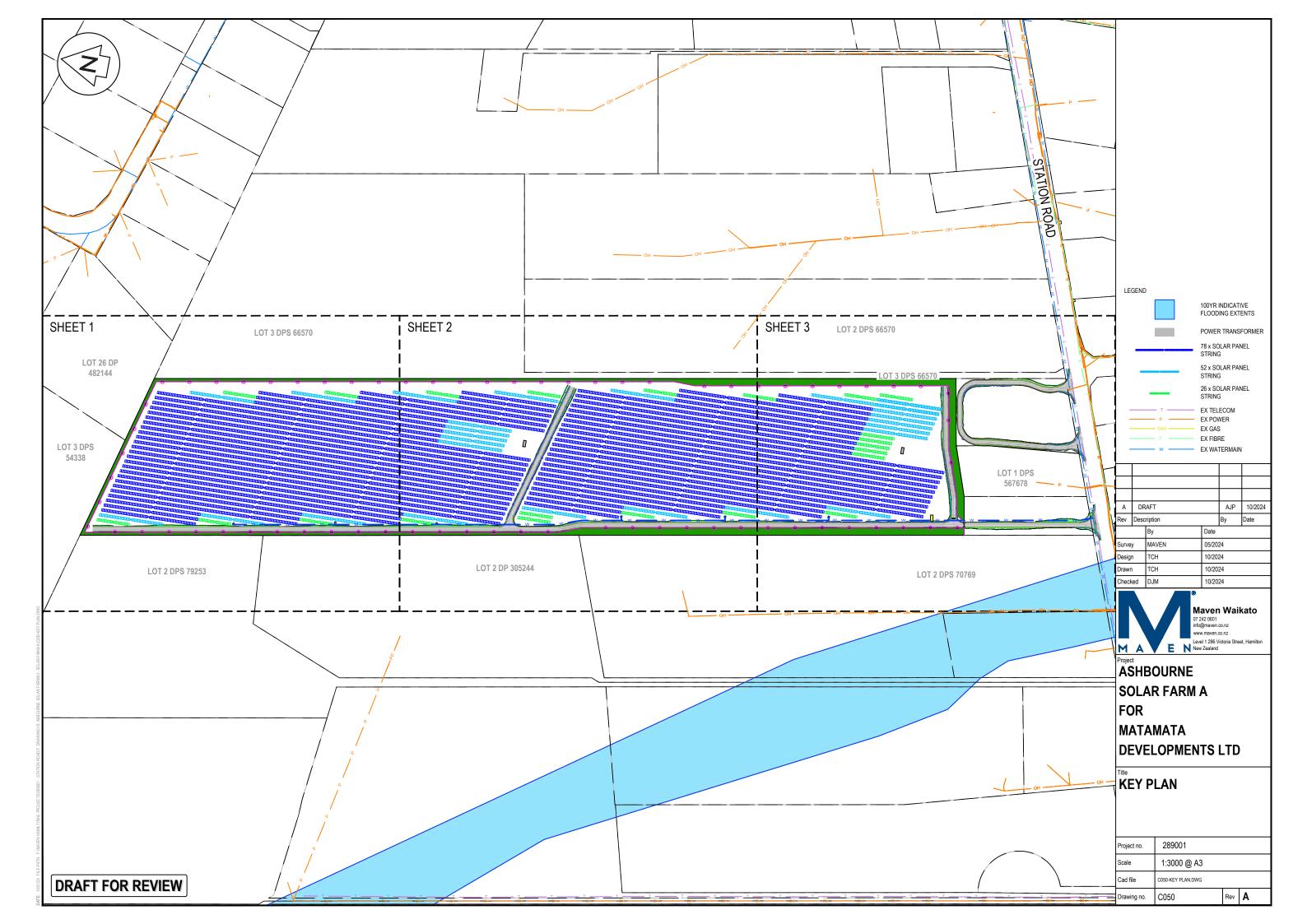


SHT.NO.	SHEET TITLE	REVISION
C000	COVER SHEET & INDEX	A
C050	KEY PLAN	А
C090	EXISTING FEATURES AND REMOVAL PLAN	А
C200	EXISTING CONTOUR PLAN	А
C210	PROPOSED CONTOUR PLAN	А
C220	CUT-FILL PLAN	А
C230	SEDIMENT EROSION CONTROL PLAN	А
C240	SEDIMENT EROSION CONTROL DETAILS	А
C300	ROADING LAYOUT PLAN	А
C320	ROADING LONGSECTION PLAN	A
C340	ROADING TYPICAL CROSS SECTION	А
C380	ROADING DETAILS	A
C480	STORM WATER DETAILS	A
C700	SERVICES LAYOUT PLAN	А
C710	SERVICES DETAILS	А

LOCALITY PLAN

PROJECT NUMBER: 289001

ISSUED DATE: NOVEMBER 2024 ISSUE FOR: RESOURCE CONSENT

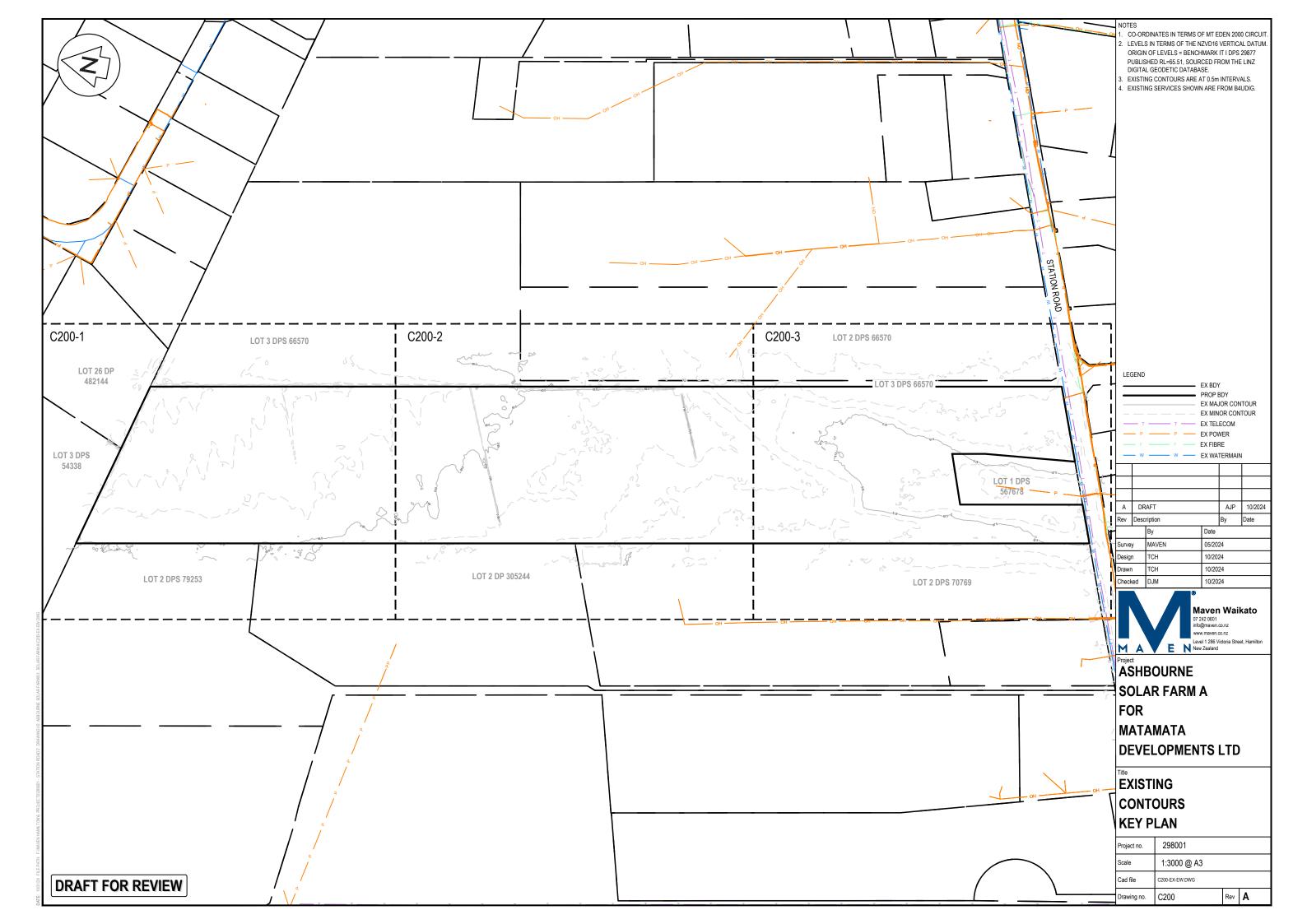


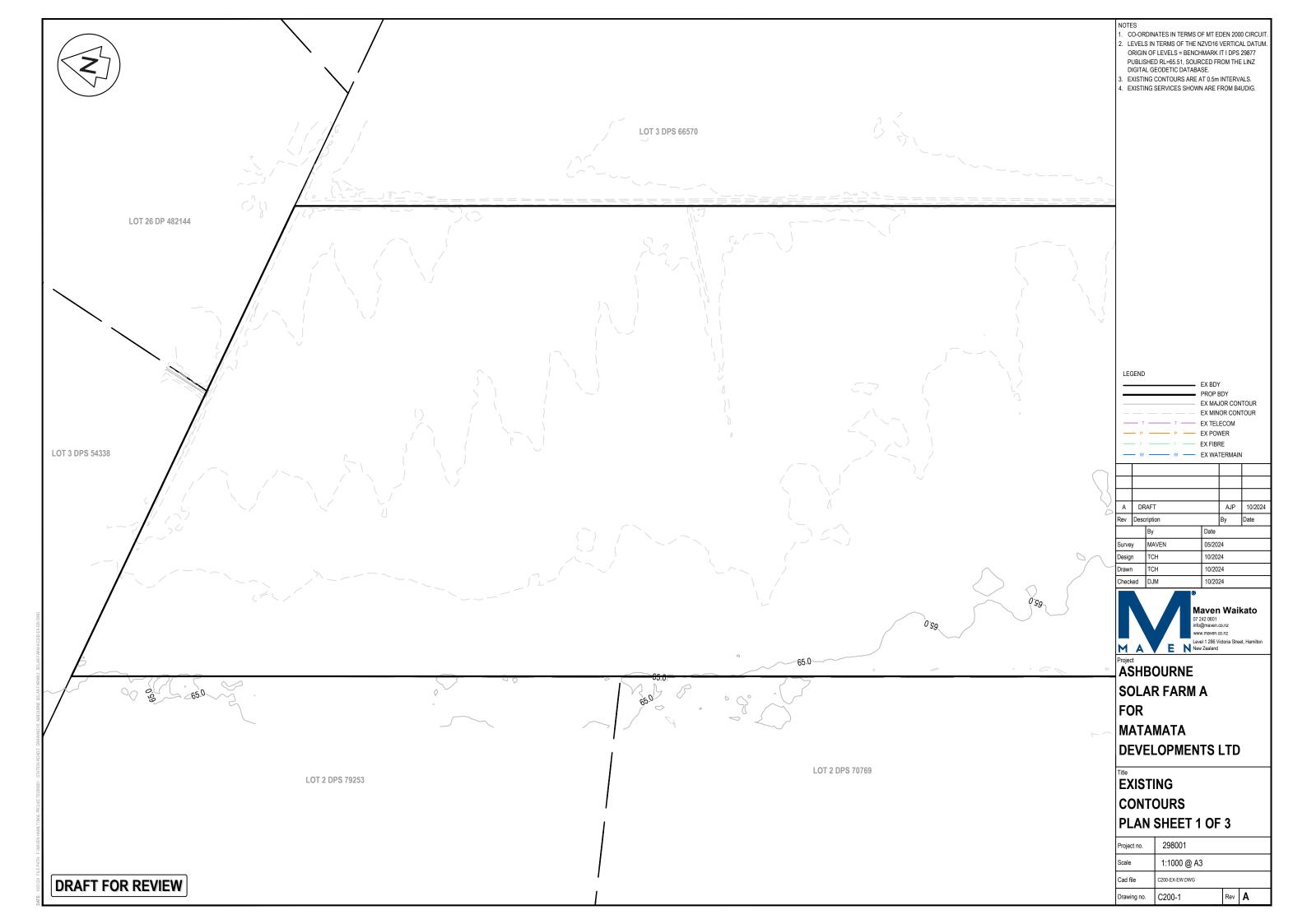


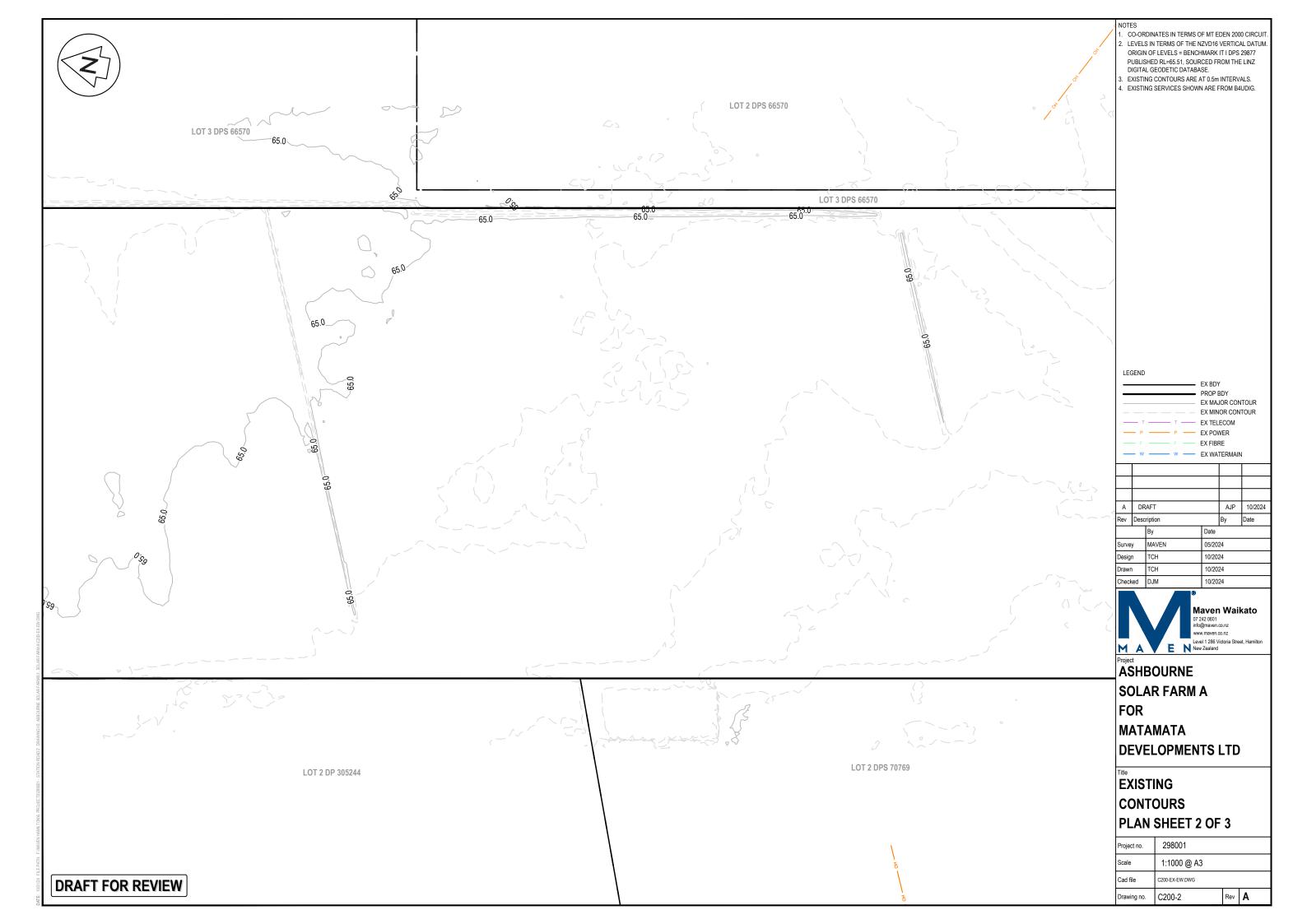




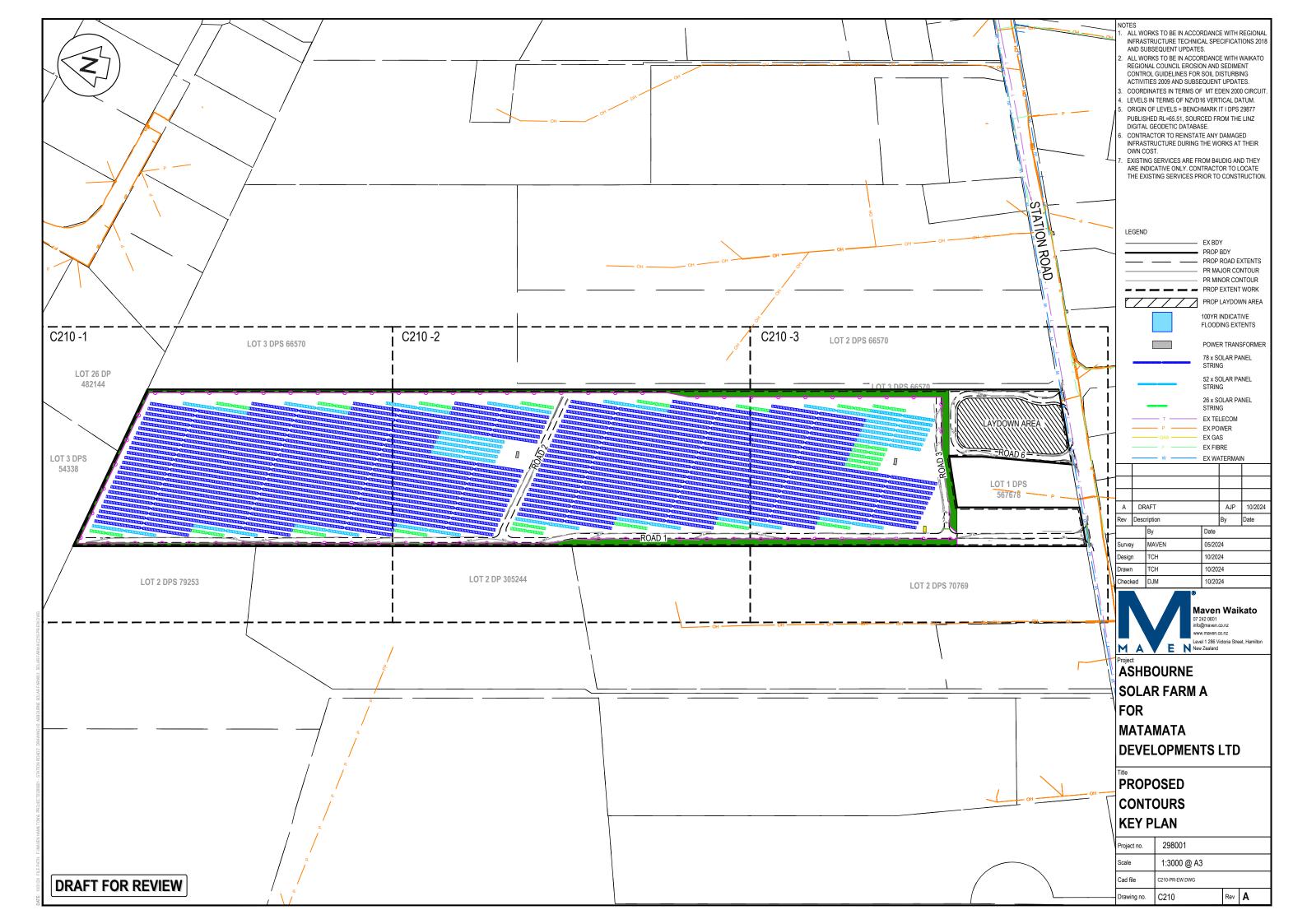


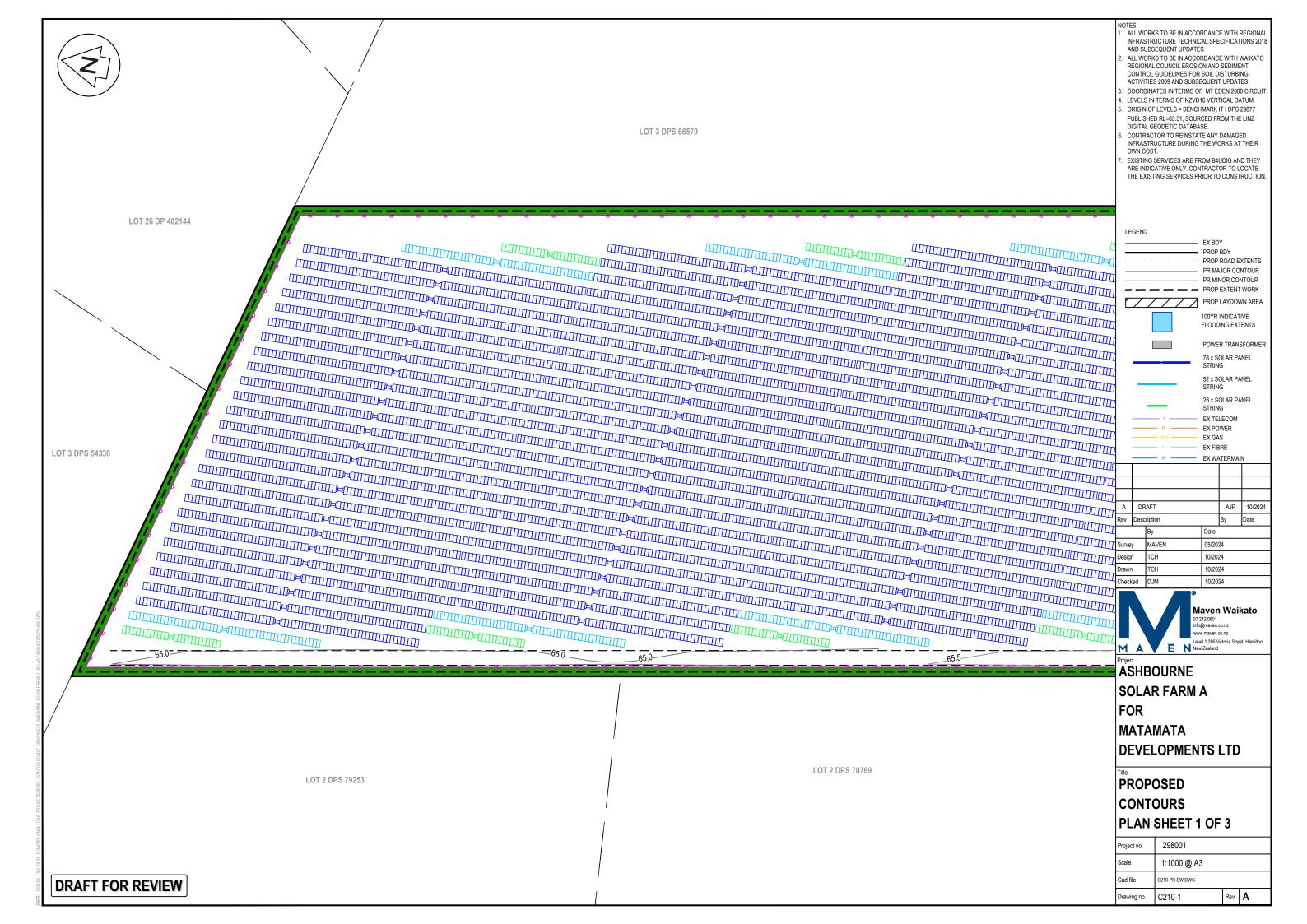


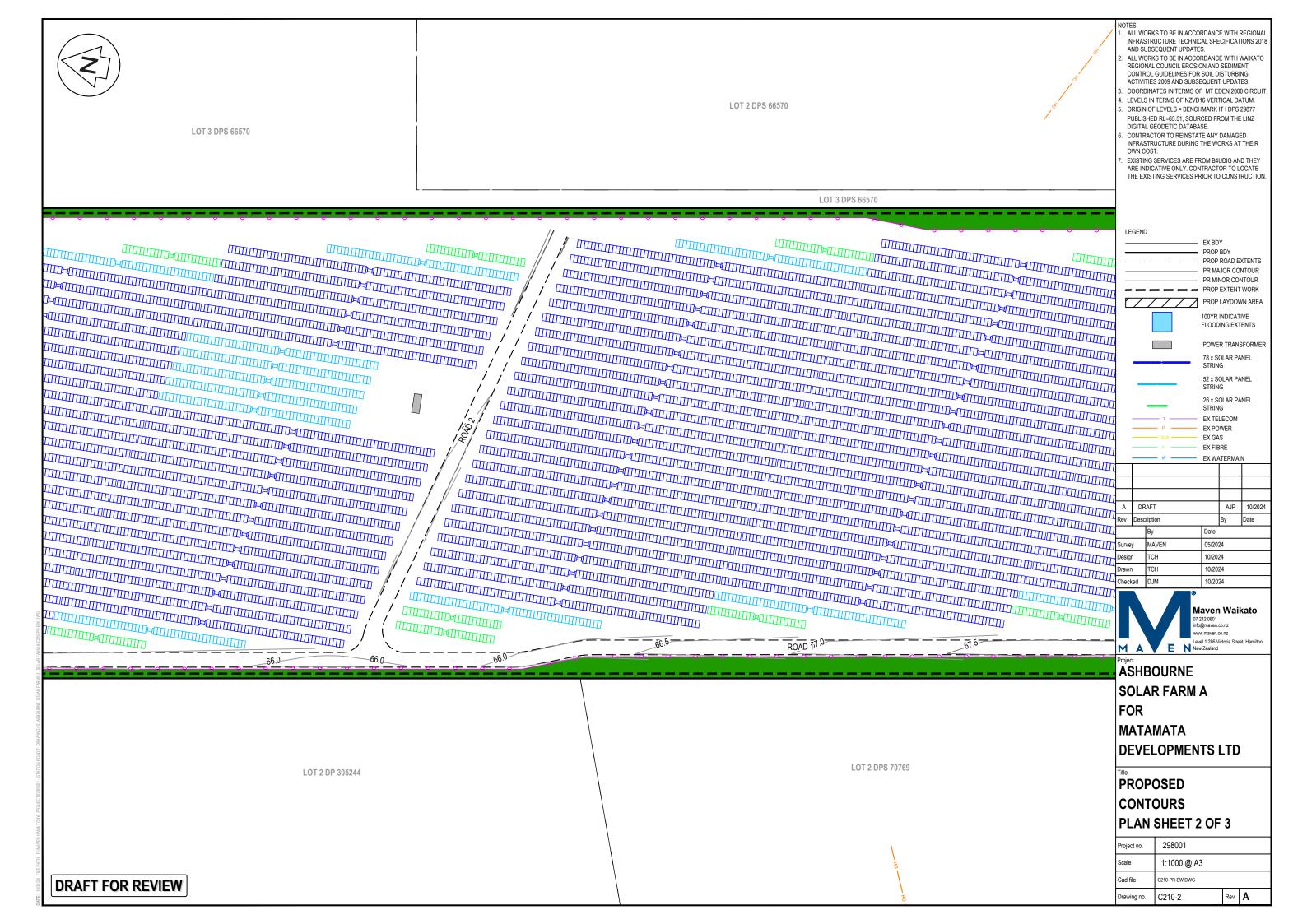


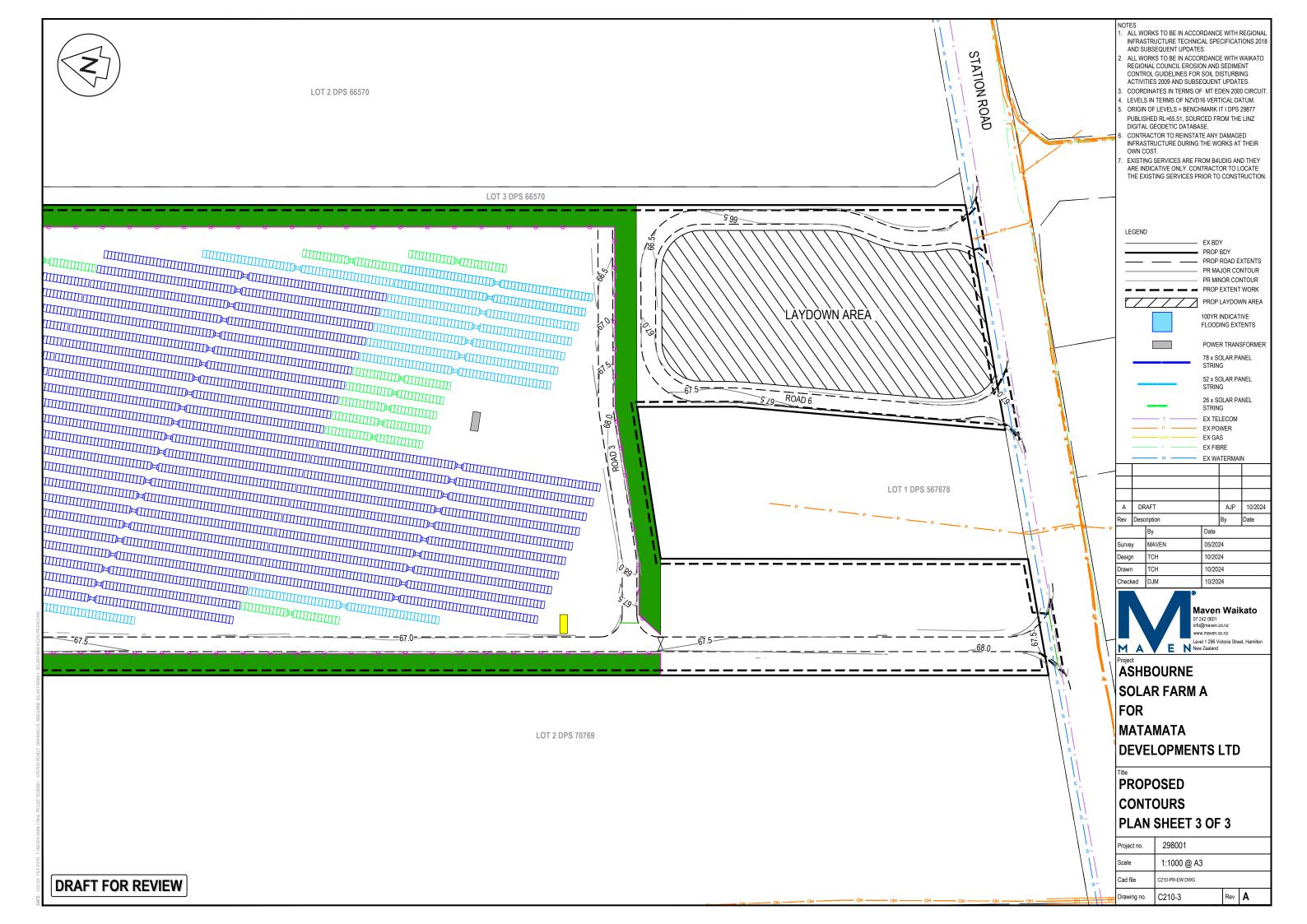


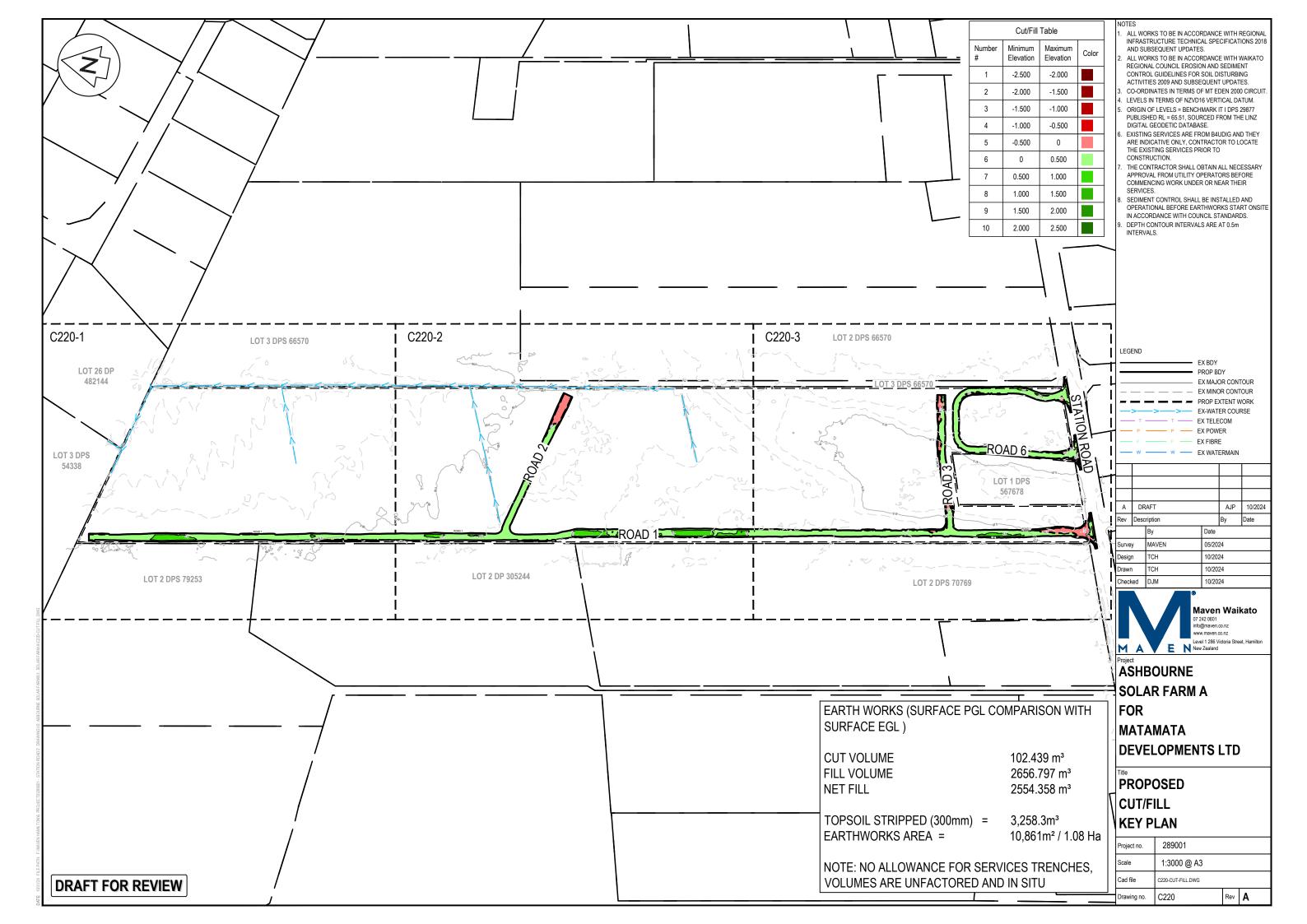


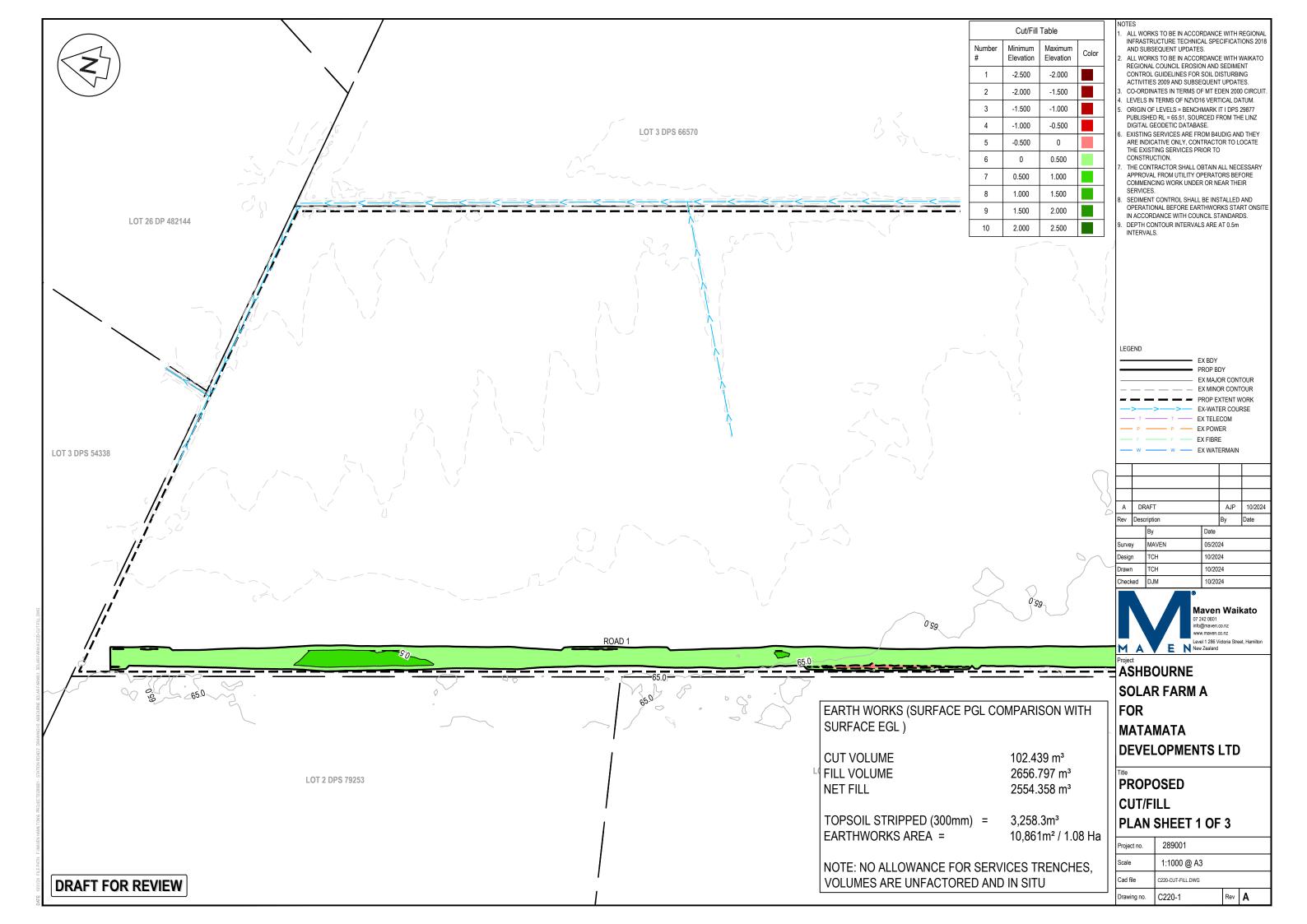


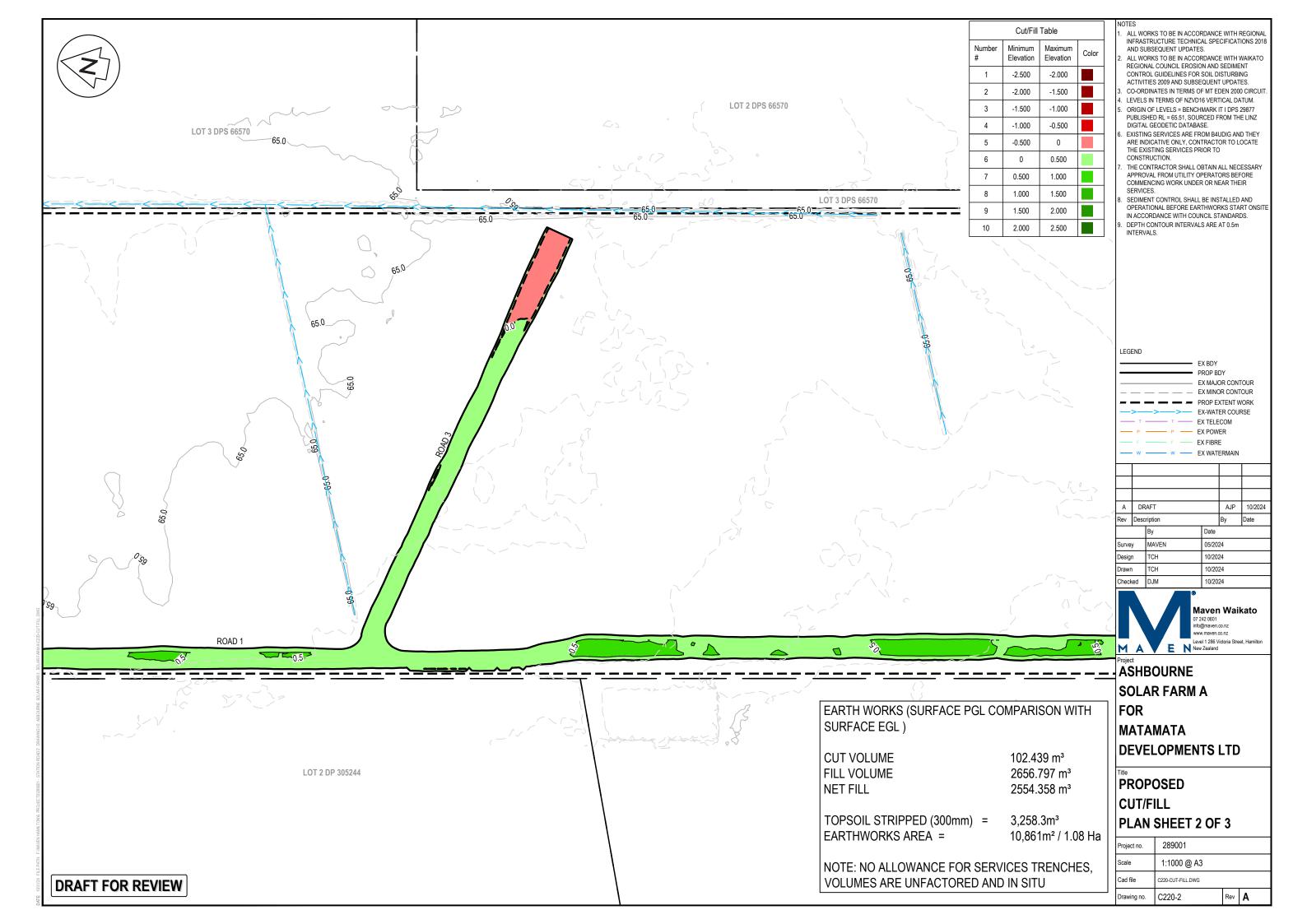


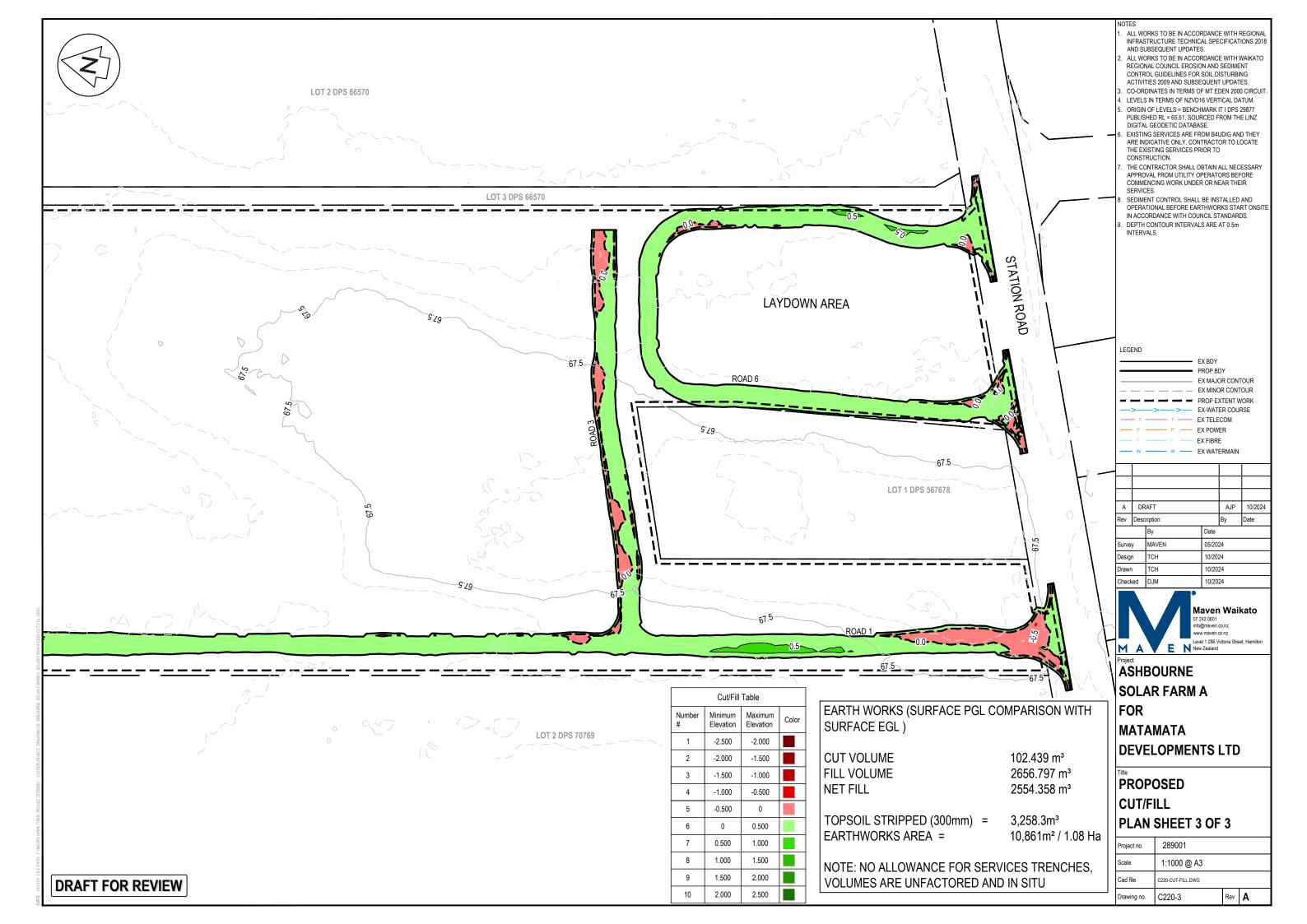




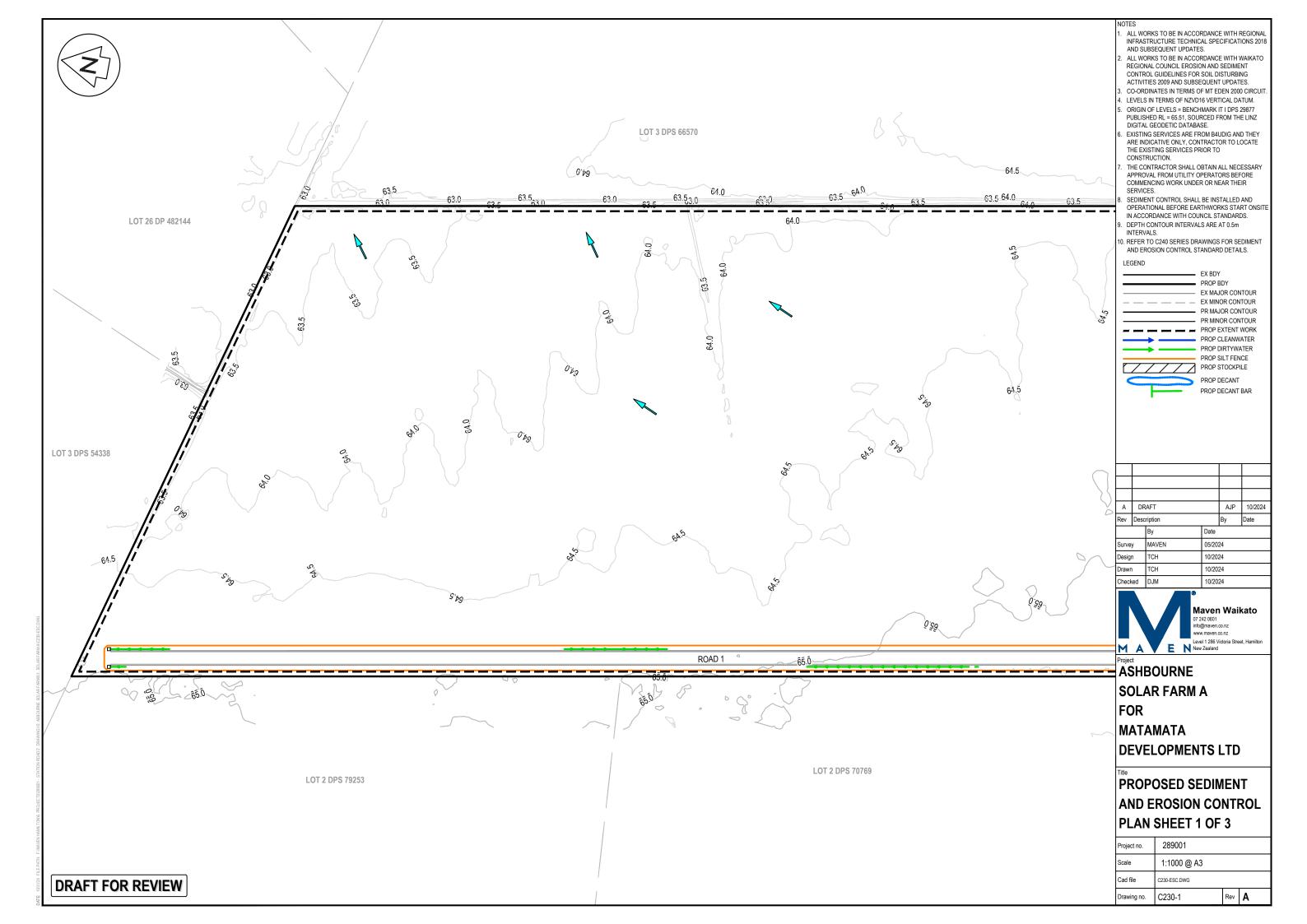


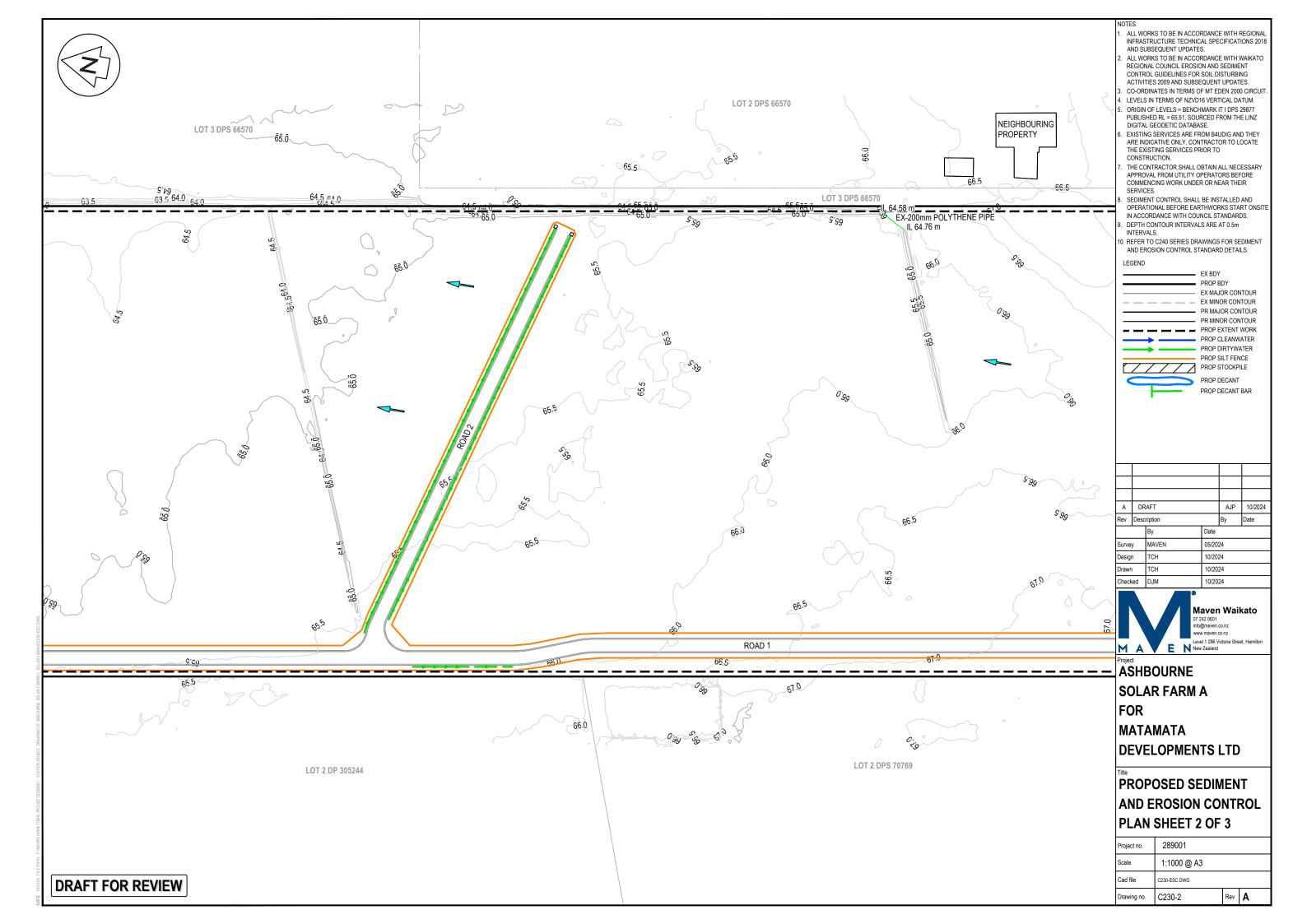




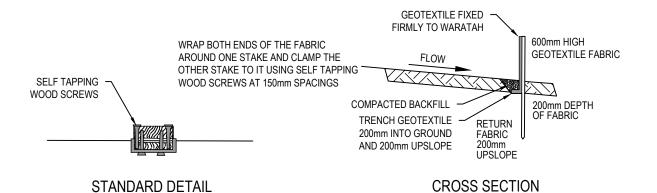






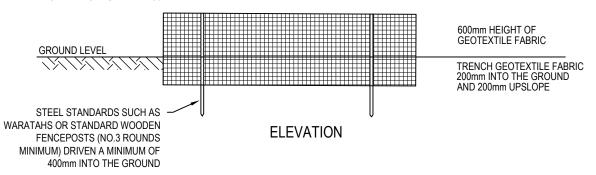


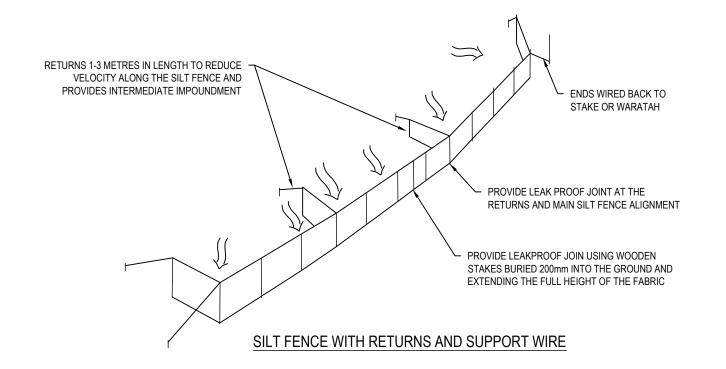




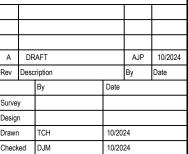
FABRIC JOIN

POST SPACING CAN BE INCREASED FROM 2 METRES TO 4 METRES IF SUPPPORTED BY A 2.5mm DIAMETER HIGH TENSILE WIRE ALONG THE TOP WITH CLIPS EVERY 200mm





- . ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 201 AND SUBSEQUENT UPDATES.
- . ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2009 AND SUBSEQUENT UPDATES.
- 3. COORDINATES IN TERMS OF MT EDEN 2000 CIRCUI
- . LEVELS IN TERMS OF NZVD16 VERTICAL DATUM. . ORIGIN OF LEVELS = BENCHMARK IT I DPS 29877
- PUBLISHED RL=65.51, SOURCED FROM THE LINZ DIGITAL GEODETIC DATABASE.
- REFER TO C230 SERIES DRAWINGS FOR SEDIMENT AND EROSION CONTROL LAYOUT DRAWINGS.

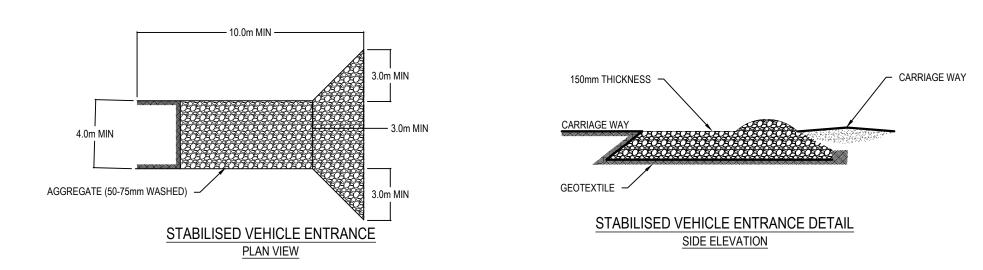


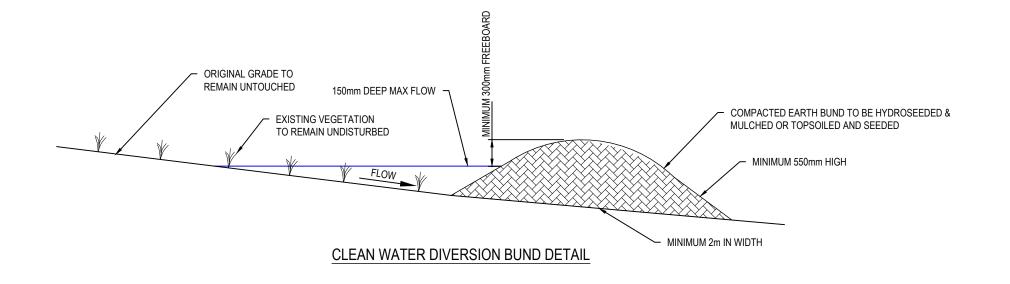


ASHBOURNE SOLAR FARM A FOR **MATAMATA DEVELOPMENTS LTD**

SEDIMENT EROSION **CONTROL DETAILS** SHEET 1 OF 2

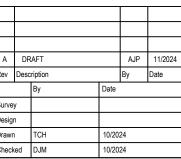
Project no.	289001				
Scale	N.T.S				
Cad file	C240-ESC DET.DWG				
Drawing no.	C240	Rev	Α		





NOTE

- ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 2018
 AND SUBSEQUENT UPDATES.
- ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2009 AND SUBSEQUENT UPDATES.
- 3. COORDINATES IN TERMS OF MT EDEN 2000 CIRCUIT
- 4. LEVELS IN TERMS OF NZVD16 VERTICAL DATUM.5. ORIGIN OF LEVELS = BENCHMARK IT I DPS 29877
- PUBLISHED RL=65.51, SOURCED FROM THE LINZ DIGITAL GEODETIC DATABASE.
- REFER TO C230 SERIES DRAWINGS FOR SEDIMENT AND EROSION CONTROL LAYOUT DRAWINGS.





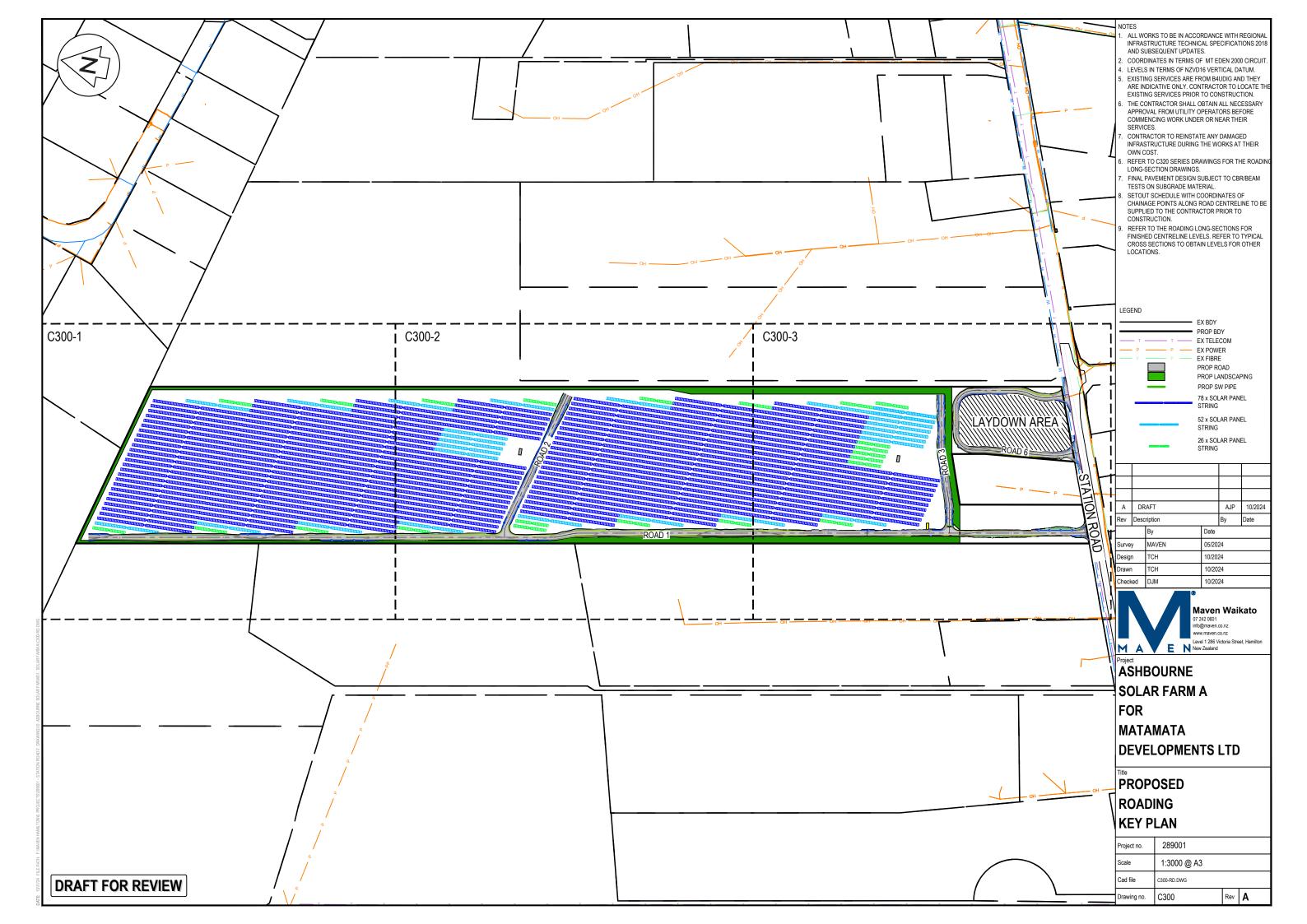
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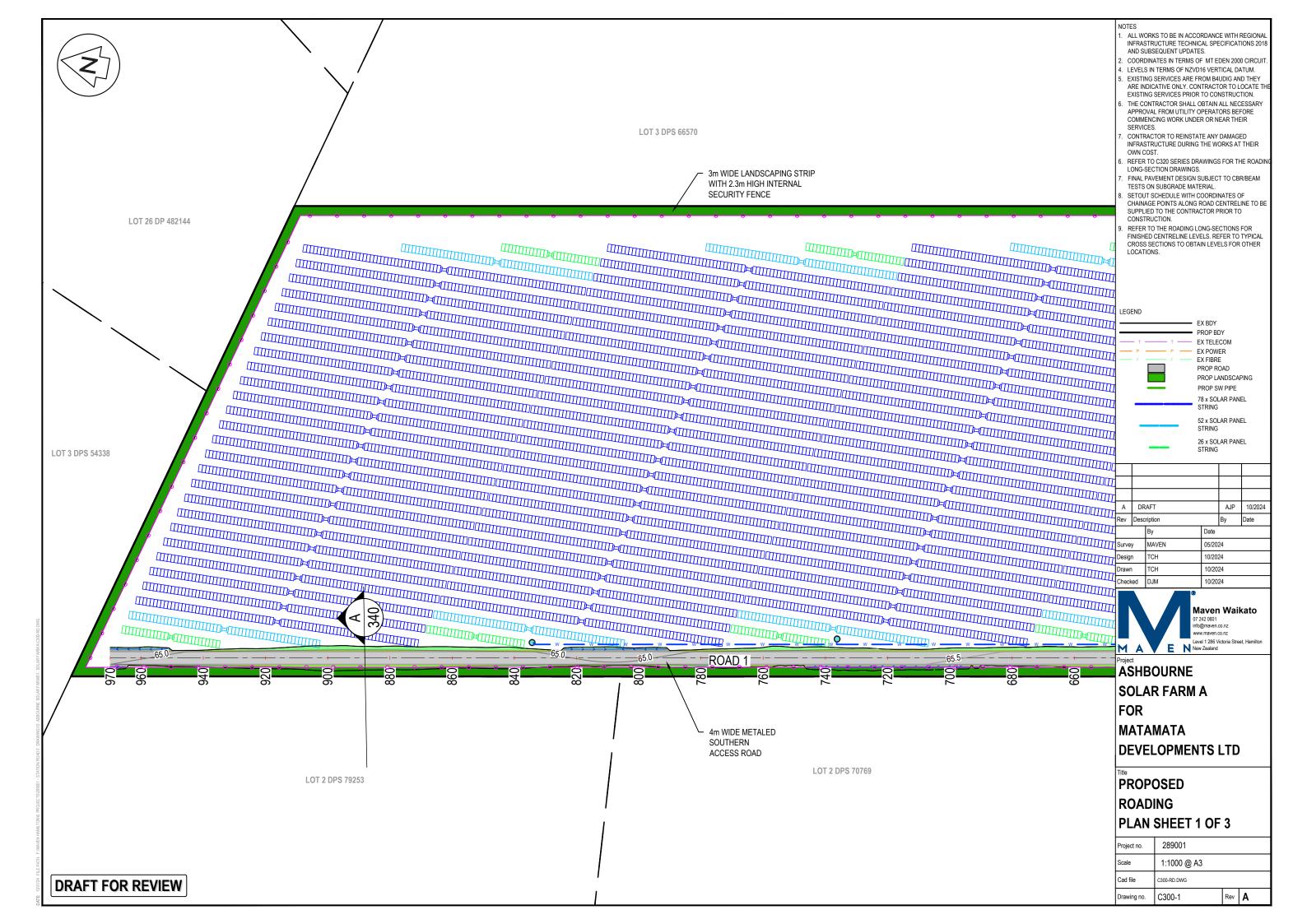
ASHBOURNE
SOLAR FARM A
FOR
MATAMATA
DEVELOPMENTS LTD

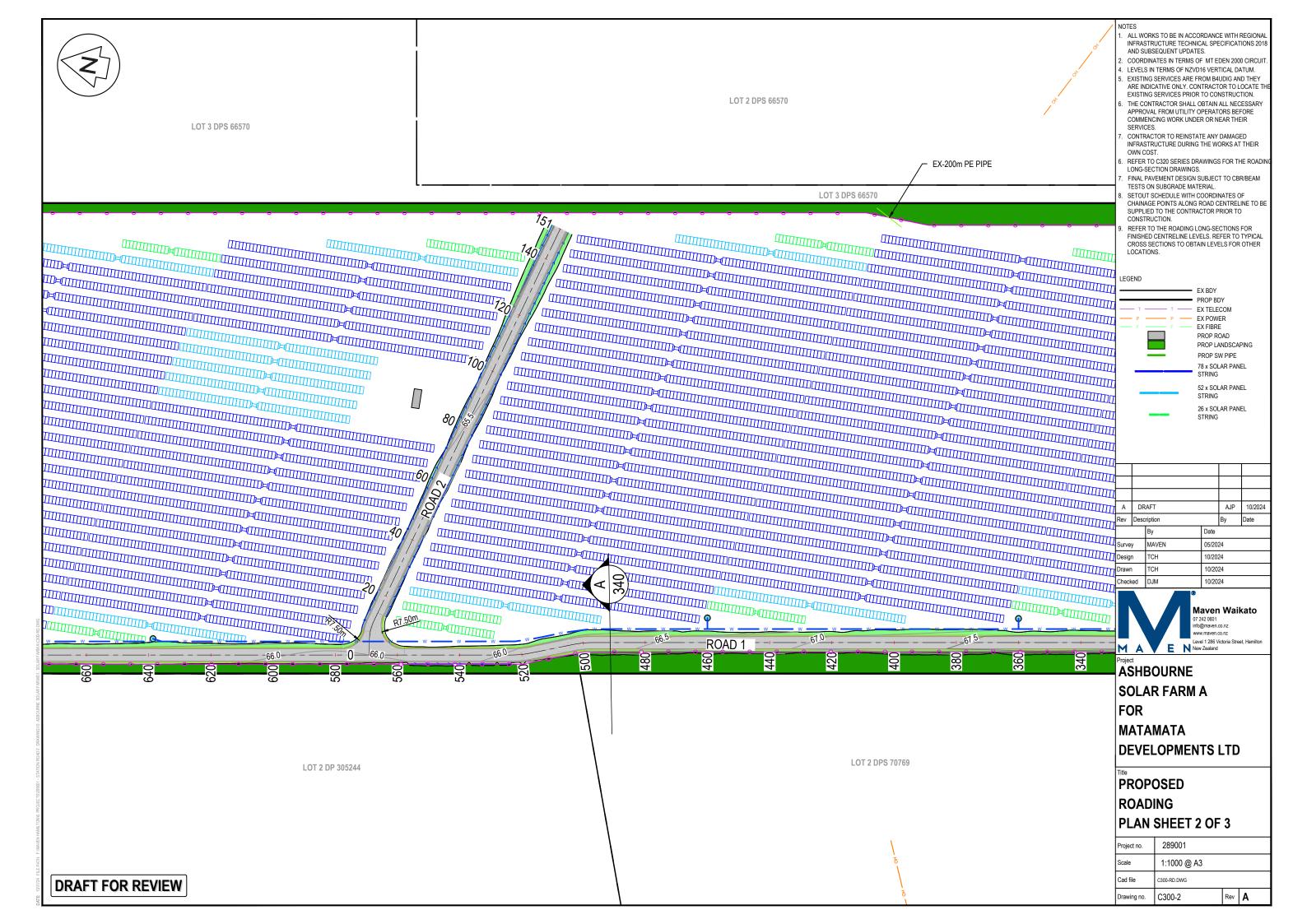
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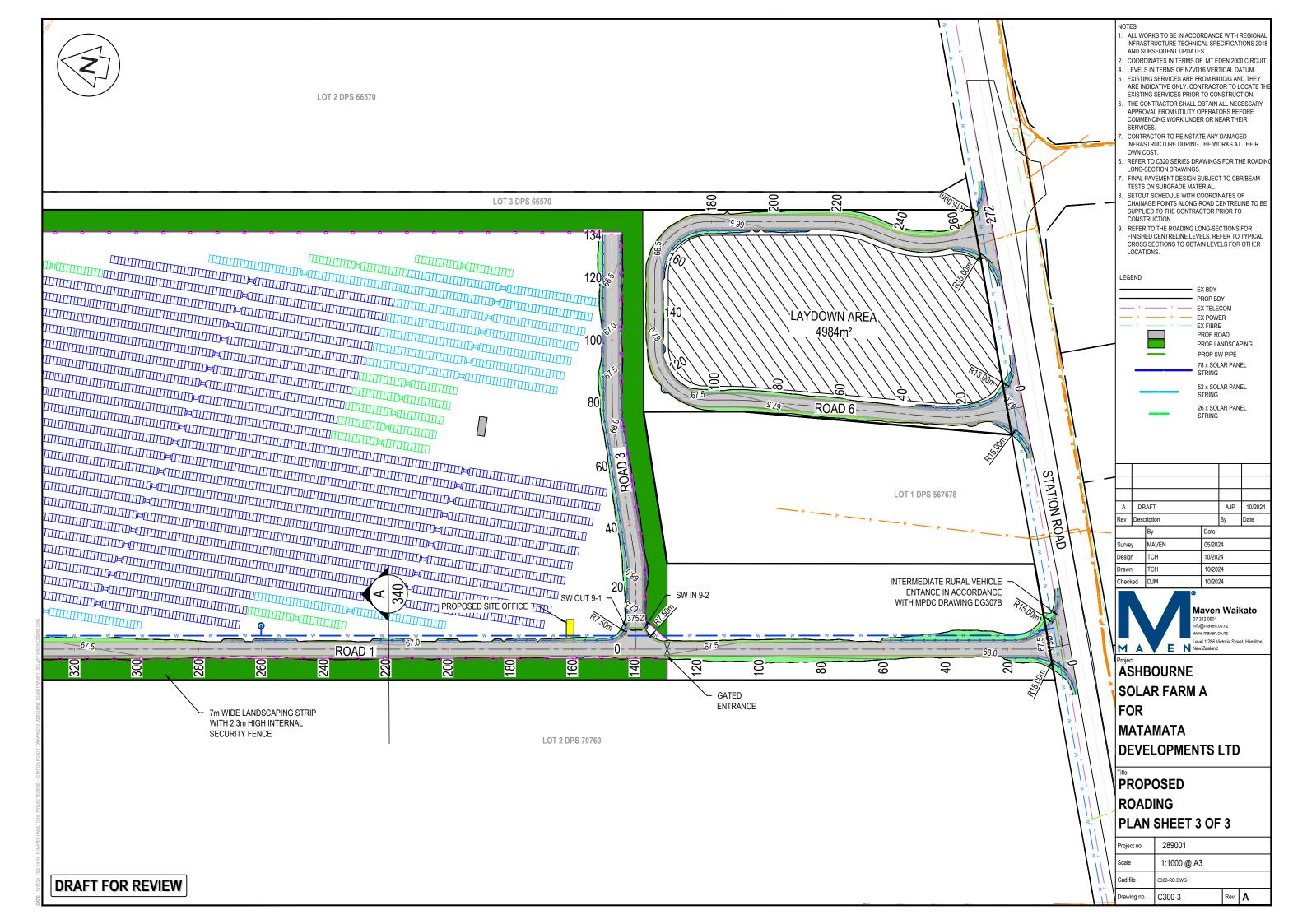
SEDIMENT EROSION
CONTROL DETAILS
SHEET 2 OF 2

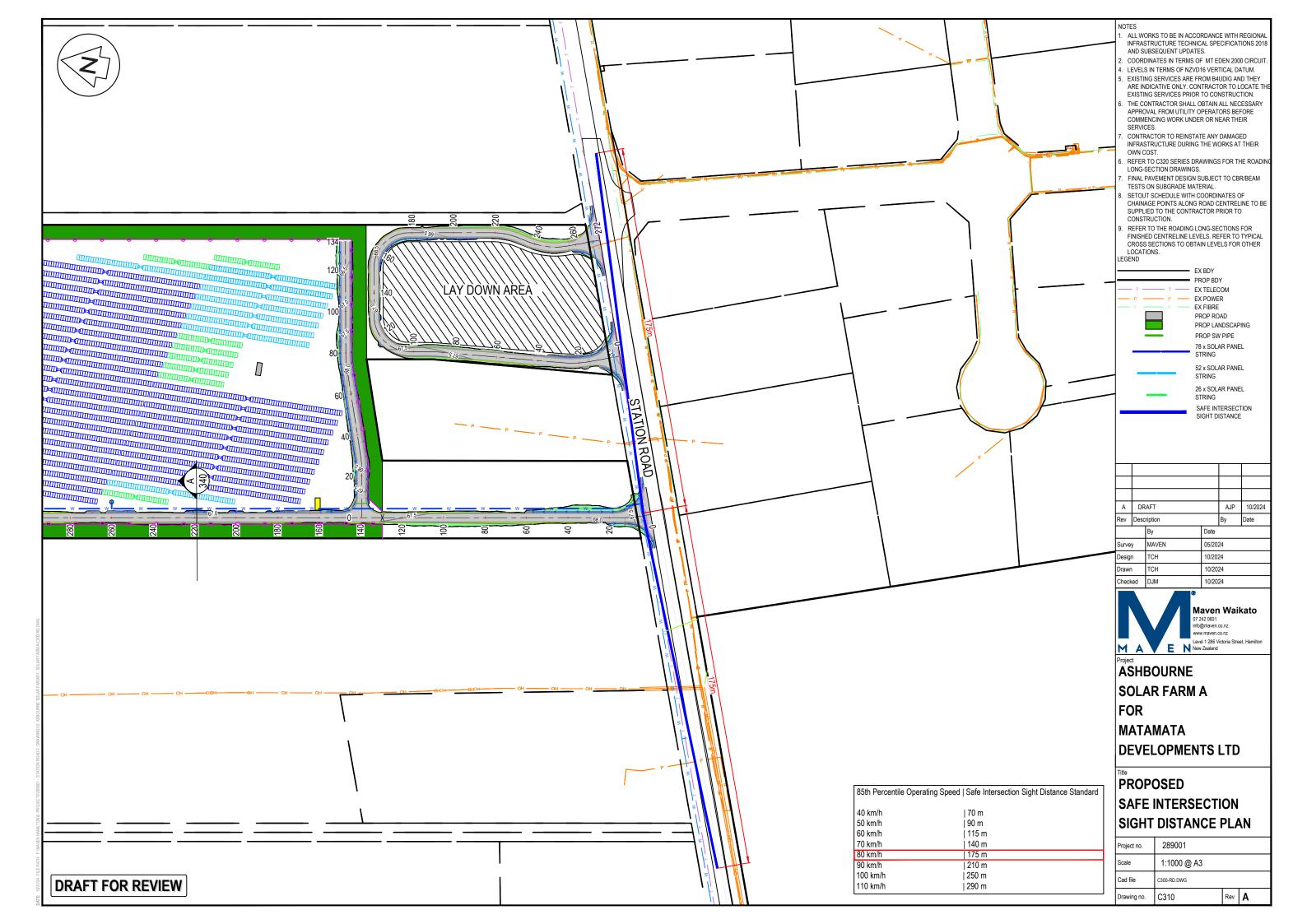
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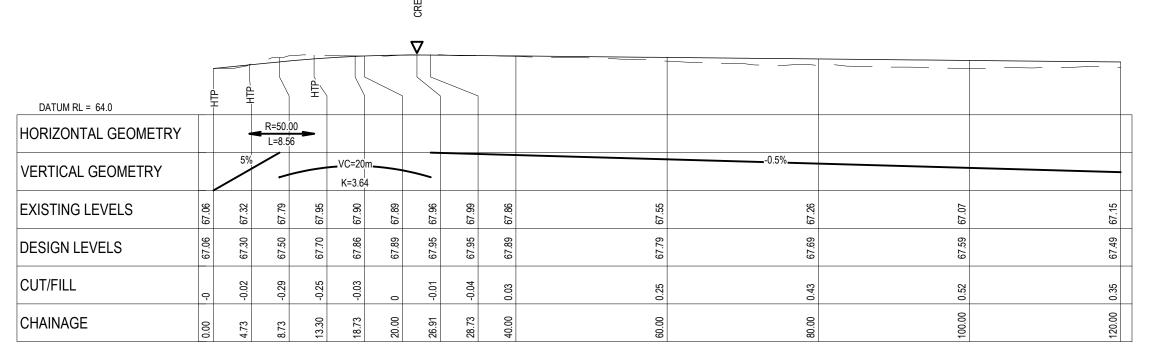












ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

Δ DATUM RL = 64.0 HORIZONTAL GEOMETRY -0.5% 0.5% VC=20m VERTICAL GEOMETRY K=20 67.15 **EXISTING LEVELS** 67.50 67.19 67.14 67.29 60 67.05 DESIGN LEVELS CUT/FILL 119.50 239.50 140.00 160.00 180.00 200.00 CHAINAGE

ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

NOTES

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE
 WITH REGIONAL INFRASTRUCTURE TECHNICAL
 SPECIFICATIONS STANDARDS.
- SPECIFICATIONS STANDARDS.

 2. COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM
 ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE ROADING LAYOUT DRAWINGS

end			
- —	—	_	EXISTING GROUND LEVEL

Α	DRA	AFT	AJP	11/2024			
Rev	Desc	ription	Ву	Date			
		Ву	Date				
Surve	y	MAVEN	05/20	24			
Desig	n	TCH	10/2024				
Drawi	n	TCH	10/20	24			
Chec	ked	DJM	10/20	24			

SAG

PROPOSED GROUND

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ASHBOURNE SOLAR FARM A FOR MATAMATA DEVELOPMENTS

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320	Rev	Α

									7	7	
	F										
DATUM RL = 64.0											
HORIZONTAL GEOMETRY											
VERTICAL GEOMETRY					0.5%					20m —— 3.33	
EXISTING LEVELS	08.99	82.99	67.11	67.06	67.04	67.13	67.12	67.17	67.19	67.19	67.17
DESIGN LEVELS	67.13	67.14	67.24	67.34	67.44	67.54	67.64	67.67	67.68	89'29	99'29
CUT/FILL	0.34	0.36	0.13	0.27	0.4	0.41	0.52	0.49	0.49	0.49	0.5
CHAINAGE	239.00	240.00	260.00	280.00	300.00	320.00	340.00	345.44	352.10	355.44	359.00

ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

	,								
DATUM RL = 63.0									
HORIZONTAL GEOMETRY									
VERTICAL GEOMETRY		VC=20r K=13.33	_				1%		
EXISTING LEVELS	67.18	67.14	67.14	66.90	66.73	66.59	66.41	66.17	65.91
DESIGN LEVELS	29.79	99'29	67.62	67.47	67.27	70.79	66.87	79:99	66.49
CUT/FILL	0.48	0.52	0.47	0.57	0.54	0.48	0.46	0.5	0.57
CHAINAGE	358.50	360.00	365.44	380.00	400.00	420.00	440.00	460.00	478.50

ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
 - COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
 - LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM. ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
 - REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE

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_	EXISTING GROUND LEVEL										
				ROPO EVEL	SED GF	ROUND					
١	DRA	AFT			AJP	11/2024					
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		Ву		Date							
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esigi	n	TCH		10/2024							

10/2024 10/2024

Maven Waikato 07 242 0601 info@maven.co.nz www.maven.co.nz

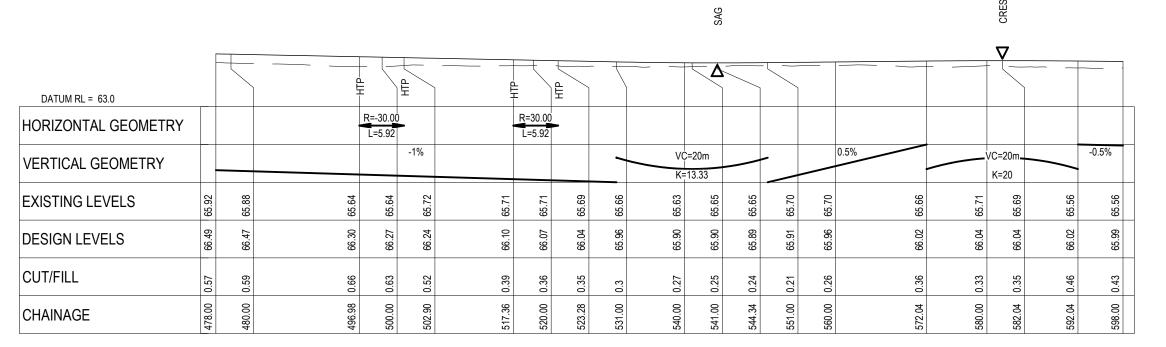
Level 1 286 Victoria Street, Hamilton New Zealand ASHBOURNE SOLAR FARM A FOR MATAMATA

DEVELOPMENTS

PROPOSED ROADING

LONGSECTIONS

Project no. 289001 AS SHOWN Cad file C320-RD LS.DWG C320-1



ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DATUM RL = 62.0								
HORIZONTAL GEOMETRY	,							
VERTICAL GEOMETRY					-0.5%			
EXISTING LEVELS	65.54	65.51	65.44	65.28	65.26	65.16	65.19	65.12
DESIGN LEVELS	62.99	65.98	65.88	65.78	65.68	65.58	65.48	65.39
CUT/FILL	0.44	0.46	0.43	0.5	0.42	0.41	0.28	0.27
CHAINAGE	597.50	00.009	620.00	640.00	00.00	00:089	00:002	717.50

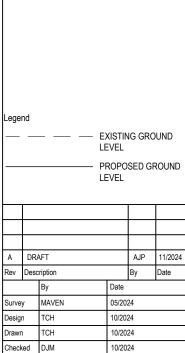
ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

NOTES

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE
 WITH REGIONAL INFRASTRUCTURE TECHNICAL
 SPECIFICATIONS STANDARDS.
- SPECIFICATIONS STANDARDS.

 COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM
 ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- 5. REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
- 6. REFER TO C300 SERIES DRAWINGS FOR THE ROADING LAYOUT DRAWINGS.



ASHBOURNE SOLAR FARM A FOR

Maven Waikato
07 242 0601
info@maven.co.nz
www.maven.co.nz
Level 1 286 Victoria Street, Hamilton
New Zealand

MATAMATA
DEVELOPMENTS

Title

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-2	Rev	Α

	F	_					_				
									2		
DATUM RL = 62.0											
HORIZONTAL GEOMETRY											
VERTICAL GEOMETRY				-0.5%			_	VC=	20m		0.5%
EXISTING LEVELS	65.13	65.13	64.86	64.83	64.78	64.80	64.79	64.84	64.86	64.87	64.68
DESIGN LEVELS	62.39	65.38	65.28	65.18	65.08	64.98	64.98	64.96	64.98	64.99	65.08
CUT/FILL	0.26	0.25	0.42	0.34	0.29	0.18	0.19	0.12	0.13	0.12	0.39
CHAINAGE	717.00	720.00	740.00	760.00	780.00	798.31	800.00	808.31	818.31	820.00	837.00

ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DATUM RL = 62.0 HORIZONTAL GEOMETRY -0.5% -VC=50m **VERTICAL GEOMETRY** K=50 **EXISTING LEVELS** 65.08 65.01 62.09 65.27 65.22 60: DESIGN LEVELS 65. CUT/FILL 0.15 836.50 956.50 840.00 864.53 880.00 889.53 900.00 914.53 920.00 940.00 CHAINAGE

> **ROAD 1 LONGITUDINAL SECTION** SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE
- ROADING LAYOUT DRAWINGS.

ge	nd					
_	_		KISTIN EVEL	IG GRO	DUND	
			ROPO EVEL	SED GI	ROUND	
	DRA	AFT		AJP	11/2024	
V	Desc	ription		Ву	Date	
		Ву	Date			
rve	у	MAVEN	05/2024			
sig	n	TCH	10/2024			

10/2024

10/2024

Maven Waikato

07 242 0601 info@maven.co.nz www.maven.co.nz Level 1 286 Victoria Street, Hamilton New Zealand ASHBOURNE SOLAR FARM A

FOR MATAMATA DEVELOPMENTS

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-3	Rev	Α

				_	
DATUM RL = 62.0					
HORIZONTAL GEOMETRY					
VERTICAL GEOMETRY			-0.5%		
EXISTING LEVELS	64.86	64.85		64.83	
DESIGN LEVELS	65.01	64.99		64.94	
CUT/FILL	0.15	0.14		0.11	
CHAINAGE	956.00	960.00		969.91	

ROAD 1 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

	E E	dL		d								
DATUM RL = 62.0												
HORIZONTAL GEOMETRY			R=15.00 L=6.65	-								
VERTICAL GEOMETRY	-3%	VC=5m K=2	,							-0.5%		
EXISTING LEVELS	65.65	65.60	65.59	65.58	65.57	65.58	65.55	65.47	65.50	65.18		65.30
DESIGN LEVELS	00.99	65.97	65.91	65.90	65.88	98.39	65.81	65.71	65.61	65.57	65.41	65.31
CUT/FILL	0.36	0.36	0.32	0.32	0.31	0.28	0.26	0.24	0.11	0.33	0.23	0.01
CHAINAGE	0.00	1.09	3.59	4.41	60.9	11.07	20.00	40.00	00:09	80.00	100.00	120.00

ROAD 2 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
 LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM.
- ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- THE ROAD CENTRE LINE.

 REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.

 REFER TO C300 SERIES DRAWINGS FOR THE ROADING LAYOUT DRAWINGS.

Legend		
	_	EXISTING GROUND LEVEL
	_	PROPOSED GROUND LEVEL

	Α	DRA	AFT		AJP	11/202
	Rev	Desc	ription		Ву	Date
			Ву	Date		
	Surve	y	MAVEN	05/202	24	
	Desig	า	TCH	10/202	24	
	Drawn	1	TCH	10/202	24	
ſ	Check	ed	DJM	10/20	24	

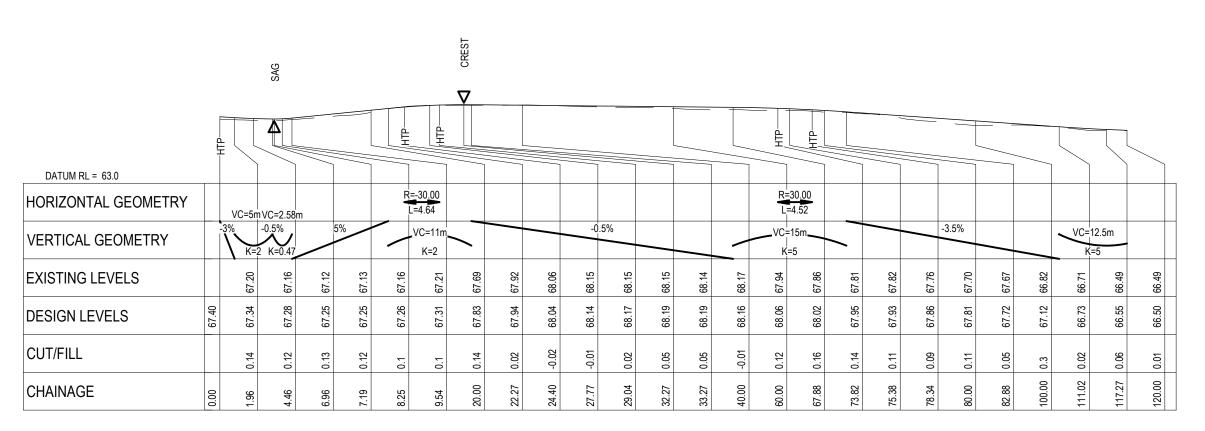


ASHBOURNE SOLAR FARM A FOR MATAMATA DEVELOPMENTS

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-4	Rev	Α

DATUM RL = 62.0				
DATUM RL = 62.0	_			
HORIZONTAL GEOMETRY				
VERTICAL GEOMETRY			-0.5%	
EXISTING LEVELS	65.29	65.30	65.37	65.30
DESIGN LEVELS	65.31	65.31	65.21	65.16
CUT/FILL	0.02	0.01	-0.15	-0.15
CHAINAGE	119.50	120.00	140.00	150.68

ROAD 2 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250



ROAD 3 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
 - COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
- REFER TO C300 SERIES DRAWINGS FOR THE

Legend EXISTING GROUND

LEVEL PROPOSED GROUND LEVEL

Α	DRA	AFT		AJP	11/2024
Rev	Desc	ription		Ву	Date
		Ву	Date		
Surve	y	MAVEN	05/20	24	
Desig	n	TCH	10/20	24	
Drawi	n	TCH	10/20	24	
Check	ked	DJM	10/20	24	



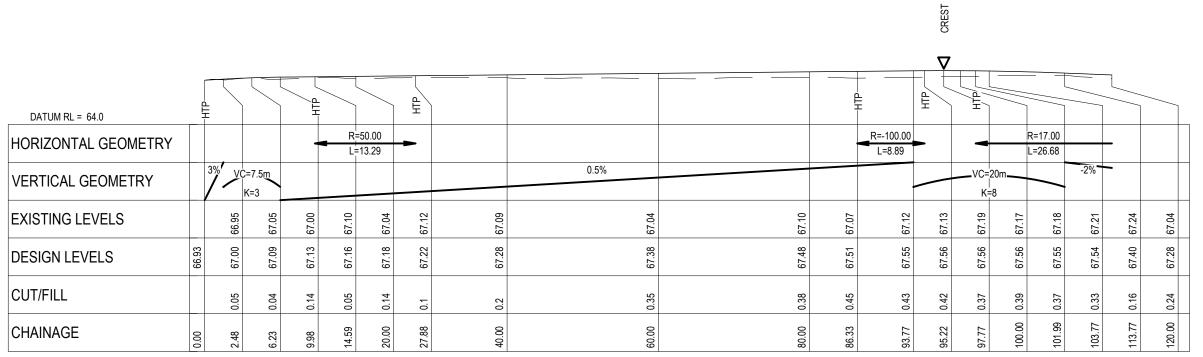
ASHBOURNE **SOLAR FARM A FOR MATAMATA DEVELOPMENTS**

PROPOSED **ROADING LONGSECTIONS**

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-5	Rev	Α

DATUM RL = 63.0				
HORIZONTAL GEOMETRY				
VERTICAL GEOMETRY	V	C=12.5m K=5	-1%	
EXISTING LEVELS	66.49	66.49	66.51	66.34
DESIGN LEVELS	66.51	09:99	66.45	66.35
CUT/FILL	0.02	0.01	-0.06	0.01
CHAINAGE	119.50	120.00	123.52	133.69

ROAD 3 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250



ROAD 6 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
 - COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE

Legend

EXISTING GROUND LEVEL PROPOSED GROUND LEVEL

Α	DRA	AFT		AJP	11/202
Rev	Desc	ription		Ву	Date
		Ву	Date		
Surve	еу	MAVEN	05/20	24	
Desig	gn	TCH	10/20	24	
Draw	n	TCH	10/20	24	
Chec	ked	DJM	10/20	24	



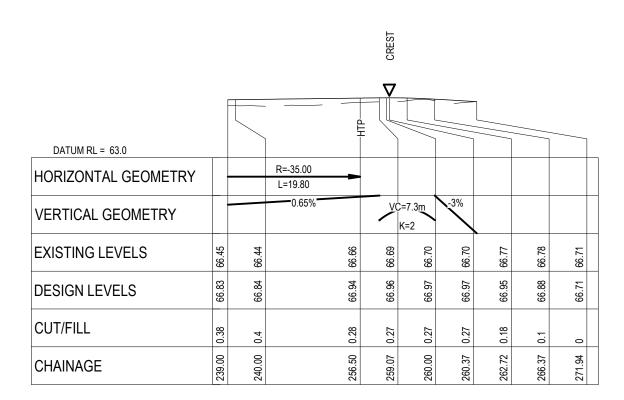
ASHBOURNE **SOLAR FARM A** FOR **MATAMATA DEVELOPMENTS**

PROPOSED **ROADING LONGSECTIONS**

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-6	Rev	Α

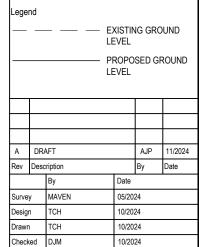
	F					_					17						 -		
			Ŧ		<u> </u>			, \	4	. \	HTP				H H H		- F - F - F	- E	-
DATUM RL = 63.0																			
HORIZONTAL GEOMETRY		R=17 L=26	.00			<		R=17.00 L=26.76			-					R=35.00 L=13.53	-	R= L:	-35.00 -19.80
VERTICAL GEOMETRY				-2%				VC=20 K=7.5	m					0.65%			_		
EXISTING LEVELS	67.05	67.04	96.99	66.57	66.41	66.37	66.31	06.30	66.30	66.32	66.33	66.38	66.35		66.35	66.35	66.44	66.48	66.45
DESIGN LEVELS	67.29	67.28	67.10	98.99	66.64	99:99	66.49	66.42	66.41	66.42	66.44	66.45	66.58		02.99	66.71	62.99	66.82	66.83
CUT/FILL	0.24	0.24	0.13	0.31	0.22	0.18		0.12	0.11	0.11	0.11	0.07	0.22		0.35	0.36	0.34	0.33	0.39
CHAINAGE	119.50	120.00	128.67	140.00	151.97	156.03	160.00	166.03	171.12	176.03	178.72	180.00	200.00		218.44	220.00	231.97	236.70	239.50

ROAD 6 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250



ROAD 6 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE

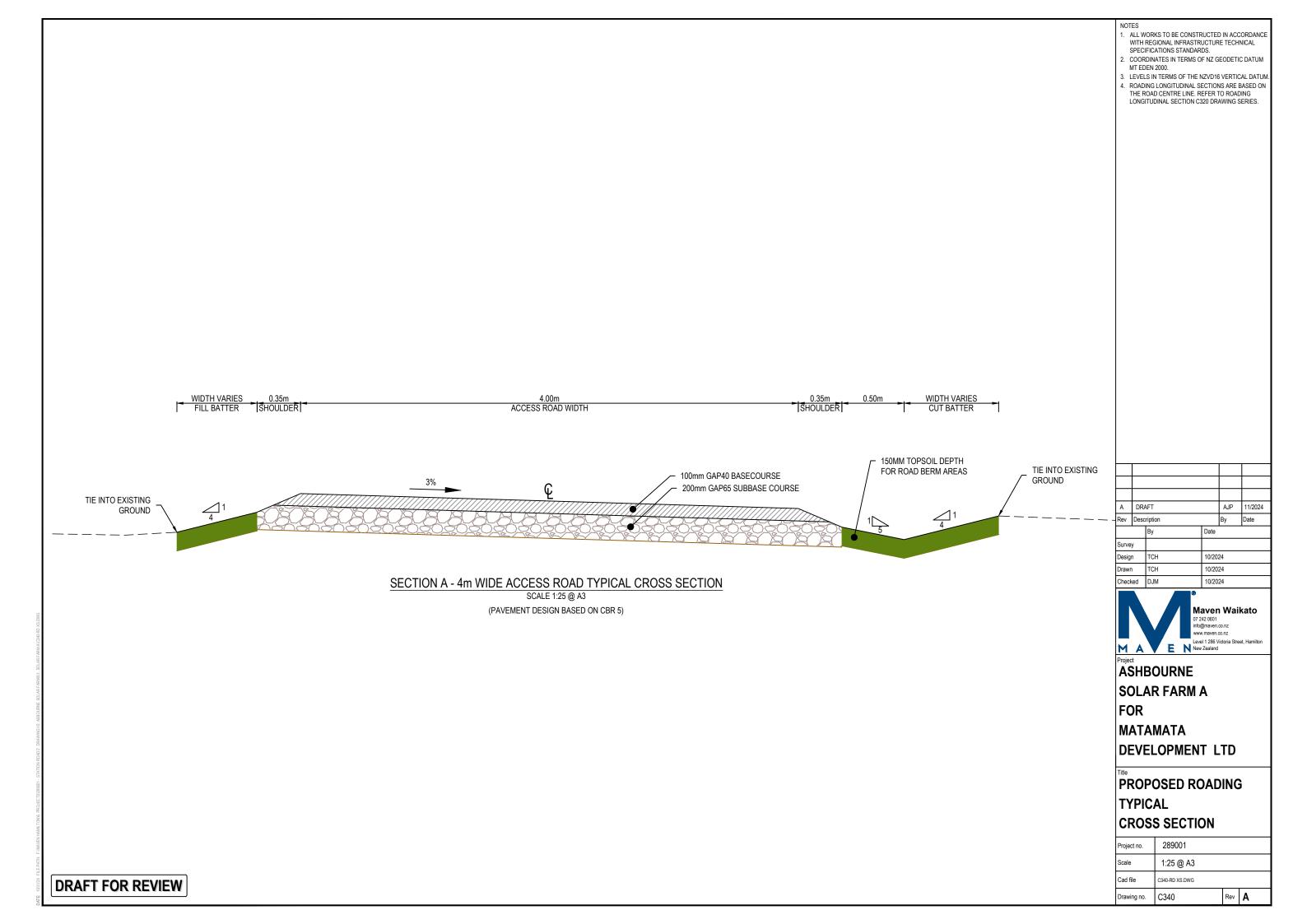




ASHBOURNE SOLAR FARM A FOR MATAMATA DEVELOPMENTS

PROPOSED **ROADING LONGSECTIONS**

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-7	Rev	Α



NOTES: Valuate Creating Carriageway 1.0 GENERAL 1:12 1.1 All works shall be constructed in accordance with the following standards and terms as applicable to site specific conditions. 1.2 No work shall be undertaken within the road reserve until Council has approved a Light = 4m traffic management plan. Large = 12m 1.3 The Contractor shall be responsible for the cost of repairs to any underground Utility Service damaged during construction. Any damage shall be rectified to the satisfaction of the Litility Owner. **ENTRANCE LONGSECTION** 2.0 LOCATION 2.1 Each entrance shall be located to provide a clear sight distance in both directions in 4.5m Driveway **Gate or Cattlestop** accordance with _____ the Development Manual. (Gate shall swing inwards to property) 2.2 Separation distances shall be as indicated in Figure 3.2 of the Development Manual 3.0 CULVERT 3.1 If an entrance crosses a Regional Council Board Drain or major watercourse, the contractor shall obtain certified waterway approval from WAIKATO REGIONAL COUNCIL. 3.2 If the entrance crosses a watertable or drain, a 300mm diameter minimum, Reinforced Concrete Rubber Ring Joint (R.C.R.R.J) Class X pipe shall be installed, unless otherwise approved by Council's Roading/Consent Engineer. 3.3 Any unsuitable bedding material including vegetation, topsoil and peat shall be removed and replaced in accordance with the pipe manufacturers specifications. 3.4 All cultverts shall be laid straight at a constant grade and a minimum of 2.0m from the edge of seal or metal. Socket ends shall always be uphill and the pipe shall be extended to a sufficient length, to ensure the resulting batter is not steeper than 1:3. 2200 12 See 4.1 A minimum subgrade CBR of 5 is required before placement of sub-base material. If this CBR cannot be achieved, Council's Roading/Consent Engineer will advise how to proceed. This may involve an additional depth of pavement construction, or the installation of pecsynthetics. 4.2 Pit sand, brown rock or similar material shall be placed, trimmed and compacted to provide 100mm depth of subbase, if required. The subbase shall be placed from the edge of the carriageway to the gate or cattlestop. 5.0 BASECOURSE **Property Boundary** 5.1 Clean good quality WHAP 40 basecourse metal shall be placed, frimmed and compacted to provide 150mm depth of basecourse from the carriageway to the gate Intilative Fonce line 5.2 The basecourse material snall be trimmed to provide a crown at the centre of the entrance to ensure adequate surface drainage. The crossfall shall be 5% from the The entranceway shall be sealed in accordance with the Development Manual. Sealing shall be a 180/200 bitumen two cost grade 3 & 5 chip seal. Edge of agai/meth 17.25m

Large Vehicle Entrance for Rural Road

> 20 ha

PLAN

NOT TO SCALE



Large Vehicle Entrance -Rural Zone

MPDC DG307c : October 2011

A DRAFT AJP 10/2024

Rev Description By Date

By Date

Survey

Design

Drawn TCH 10/2024

Checked DJM 10/2024

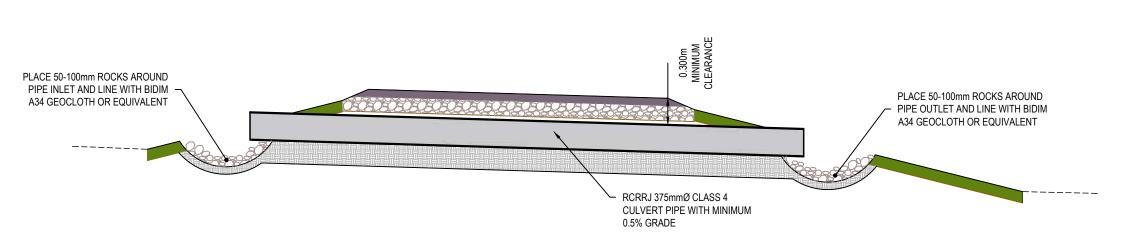


Project
ASHBOURNE
SOLAR FARM A
FOR
MATAMATA
DEVELOPMENT

PROPOSED
ROADING STANDARD
DETAILS

Project no.	289001		
Scale	N.T.S		
Cad file	C380-RD DET.DWG		
Drawing no.	C380	Rev	Α

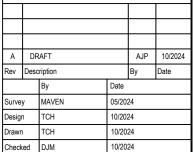
RESOURCE CONSENT



STORMWATER CULVERT TYPICAL DETAIL

NOTES

- ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 2018 AND SUBSEQUENT UPDATES.
- 2. ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2093 AND SUBSEQUENT UPDATES.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000. LEVELS IN TERMS OF NZVD16 VERTICAL DATUM 1946.
- EXISTING SERVICES ARE FROM BAUDIG AND THEY
 ARE INDICATIVE ONLY. CONTRACTOR TO LOCATE
 THE EXISTING SERVICES PRIOR TO CONSTRUCTION.
- 5. REFER TO THE ROADING C300 SERIES DRAWING FOR THE PROPOSED DRAINAGE CULVERT LOCATIONS.





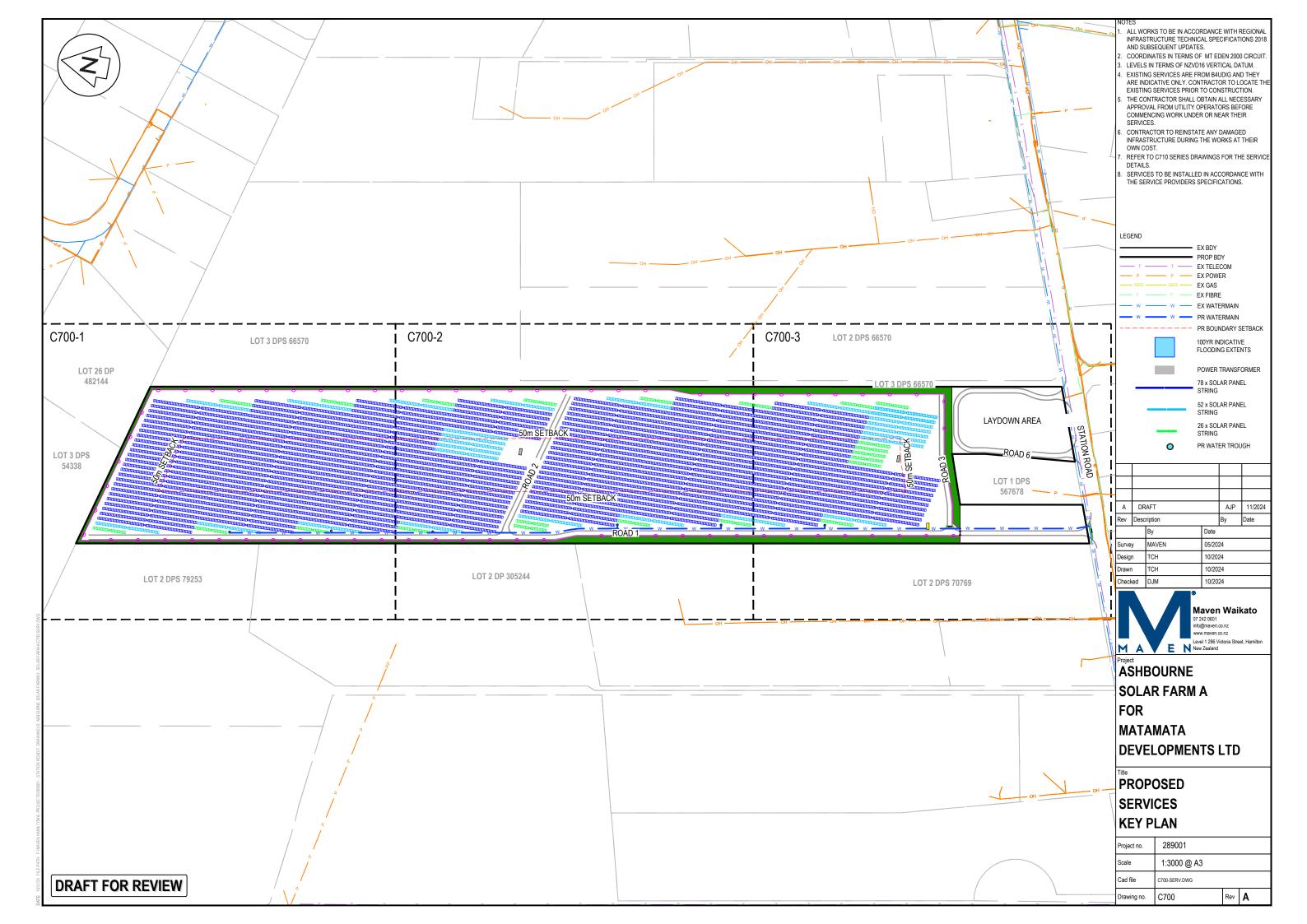
Project

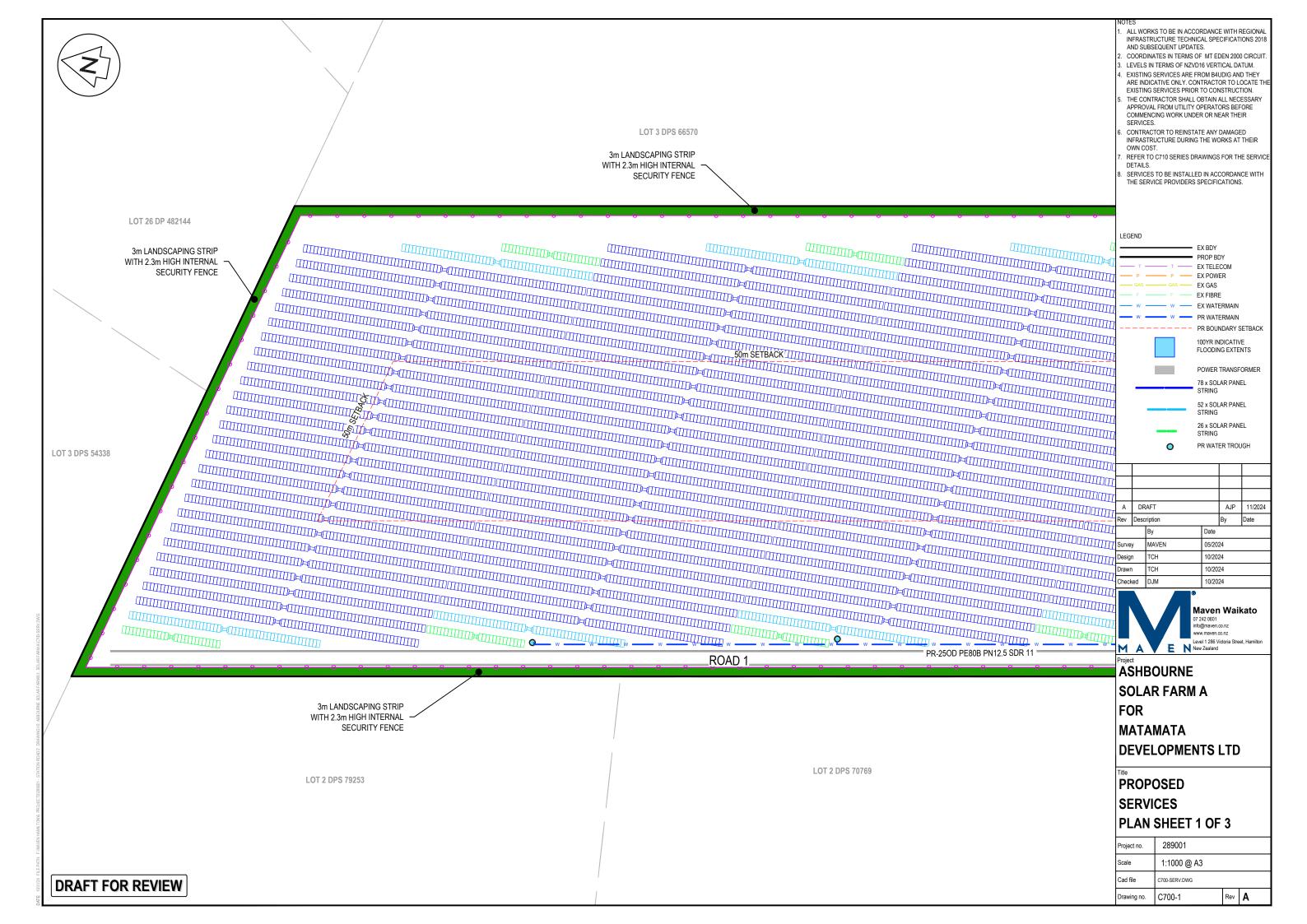
ASHBOURNE
SOLAR FARM A
FOR
MATAMATA
DEVELOPMENTS LTD

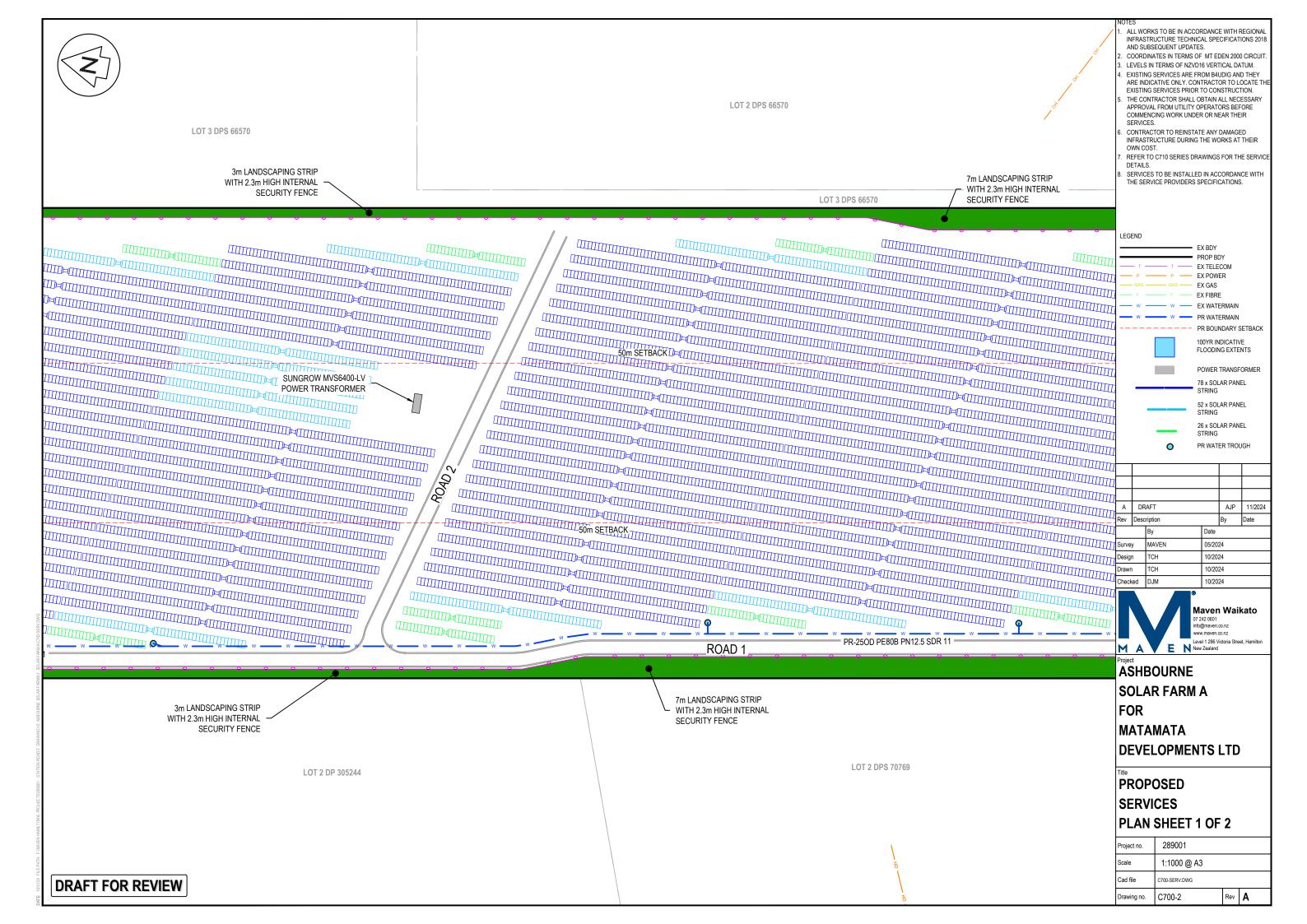
Title

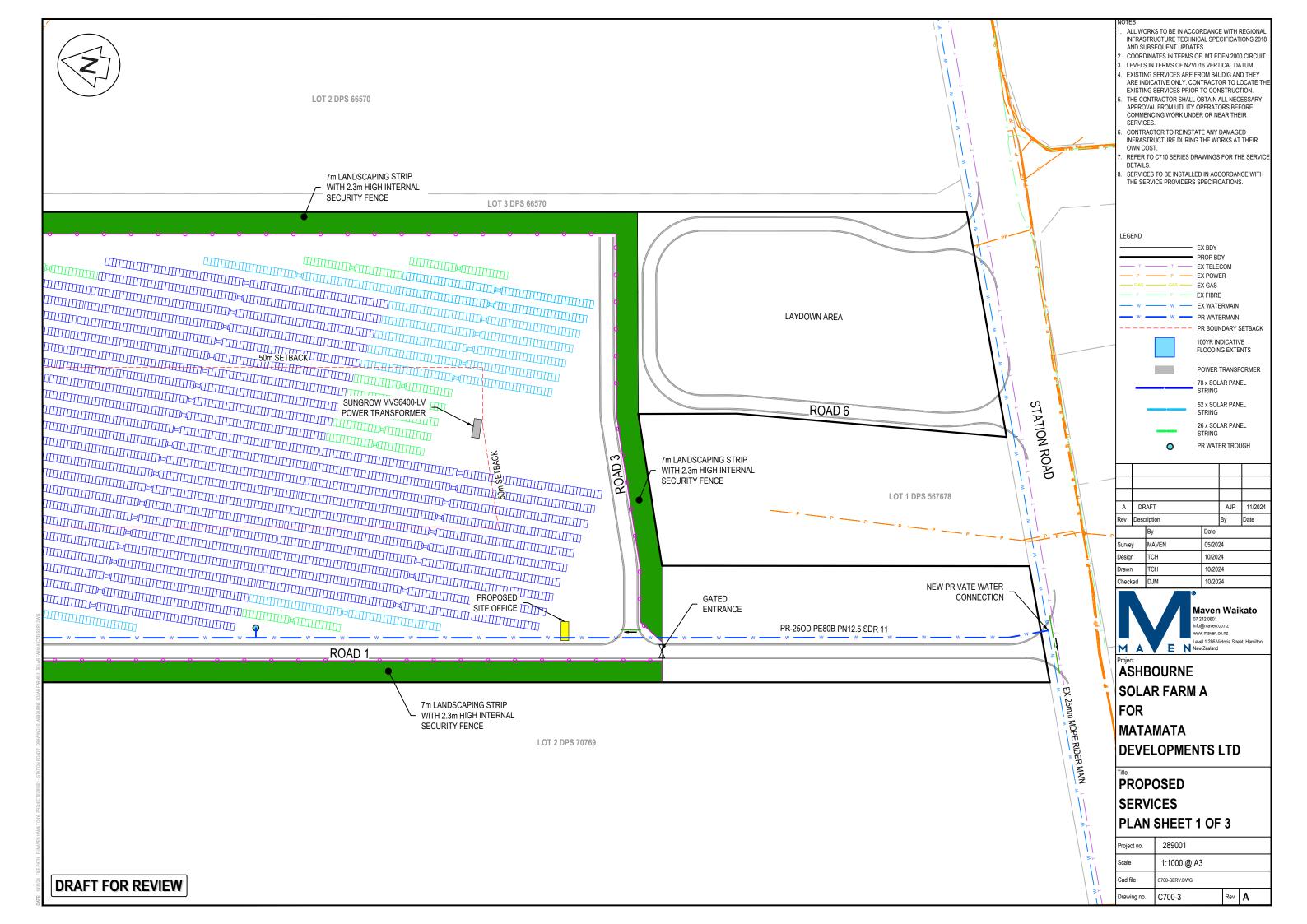
PROPOSED STORMWATER DETAILS PLAN

Project no.	289001		
Scale	N.T.S		
Cad file	C480-SW DET.DWG		
Drawing no.	C420	Rev	Α









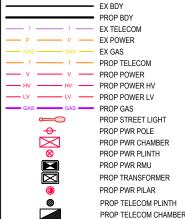
DRAFT FOR REVIEW

NOTE

- ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 2018 AND SUBSEQUENT UPDATES.
- 2. COORDINATES IN TERMS OF MT EDEN 2000 CIRCUIT
- LEVELS IN TERMS OF NZVD16 VERTICAL DATUM.
 EXISTING SERVICES ARE FROM B4UDIG AND THEY
 ARE INDICATIVE ONLY. CONTRACTOR TO LOCATE TH
- EXISTING SERVICES PRIOR TO CONSTRUCTION.

 5. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY APPROVAL FROM UTILITY OPERATORS BEFORE COMMENCING WORK UNDER OR NEAR THEIR SERVICES.
- S. CONTRACTOR TO REINSTATE ANY DAMAGED INFRASTRUCTURE DURING THE WORKS AT THEIR OWN COST.
- REFER TO C710 SERIES DRAWINGS FOR THE SERVICE DETAILS.
- 8. SERVICES TO BE INSTALLED IN ACCORDANCE WITH THE SERVICE PROVIDERS SPECIFICATIONS.

LEGEND



Α	DR	AFT			AJP	10/2024
Rev	Desc	ription	ption		Ву	Date
		Ву		Date		
Surve	у	MAVEN 05/2024				
Desig	n	TCH		10/2024		
Drawr	1	TCH		10/2024		
Check	ed	DJM		10/2024		



roject

ASHBOURE SOLAR FARM A FOR MATAMATA DEVELOPMENTS LTD

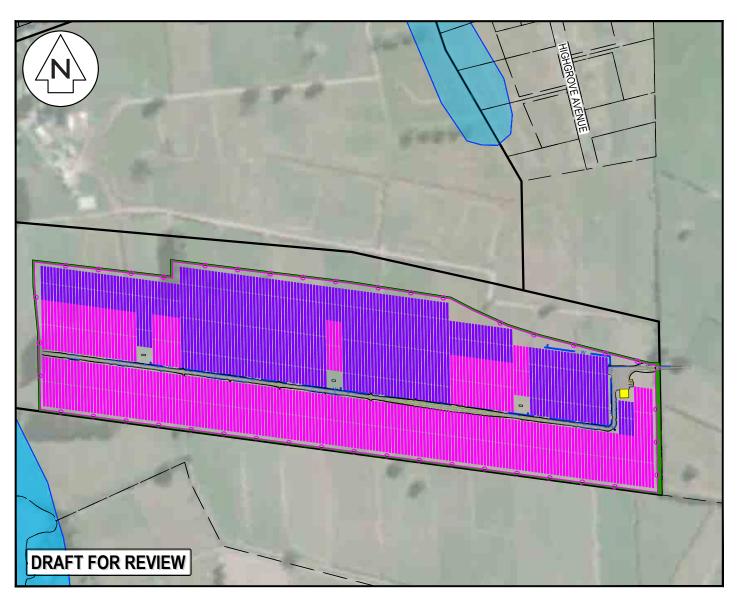
Title

PROPOSED SERVICES DETAILS

Project no.	289001		
Scale	1:500 @ A3		
Cad file	C710-SERV DET.DWG		
Drawing no.	710	Rev	Α



ASHBOURNE SOLAR FARM B FOR MATAMATA DEVELOPMENTS LTD

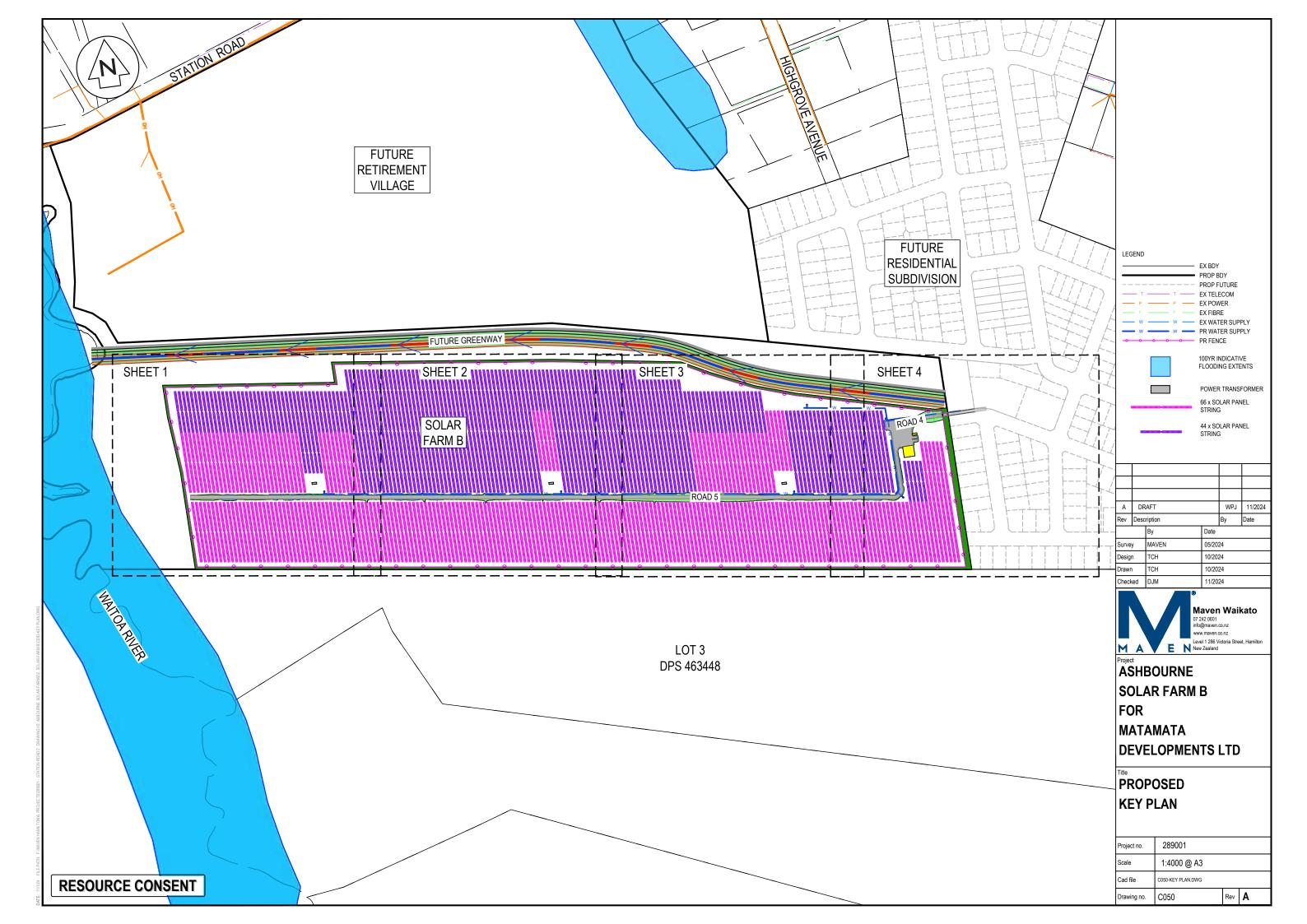


SHT.NO.	SHEET TITLE	REVISION
0111.110.	OHEET THEE	TEVIOIOIV
C000	COVER SHEET & INDEX	A
C050	KEY PLAN	A
C090	EXISTING FEATURES AND REMOVAL PLAN	А
C200	EXISTING CONTOURS	A
C210	PROPOSED CONTOURS	A
C220	CUT-FILL PLAN	A
C230	SEDIMENT & EROSION CONTROL PLAN	A
C240	SEDIMENT & EROSION CONTROL DETAILS	А
C300	ROADING LAYOUT PLAN	A
C320	ROADING LONGSECTIONS	A
C340	ROADING TYPICAL SECTIONS	A
C380	ROADING STANDARD DETAILS	A
C480	STORM WATER DETAILS	A
C700	SERVICES LAYOUT PLAN	A
C710	SERVICE DETAILS	А

LOCALITY PLAN

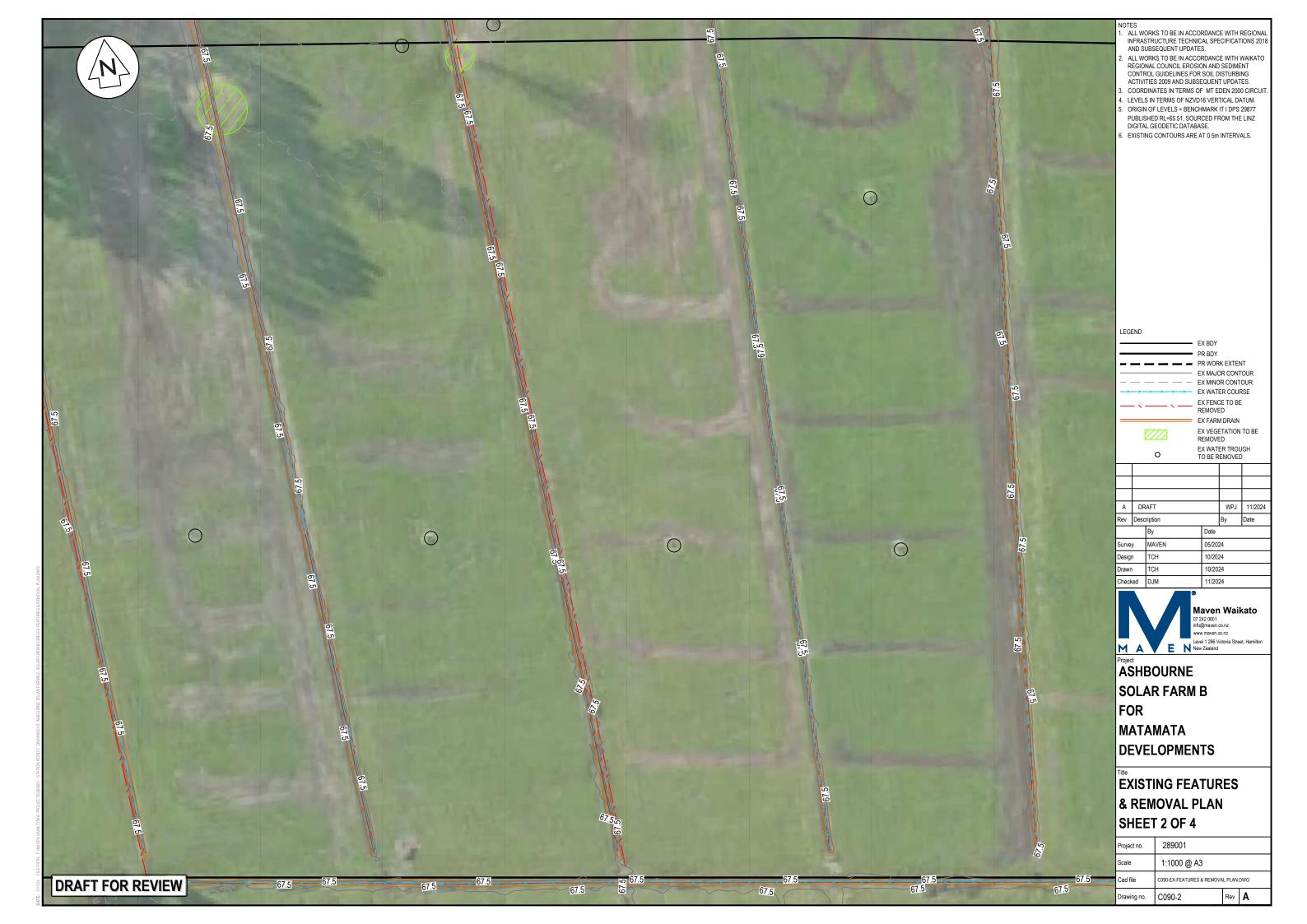
PROJECT NUMBER: 289001

ISSUED DATE: NOVEMBER 2024 ISSUE FOR: RESOURCE CONSENT







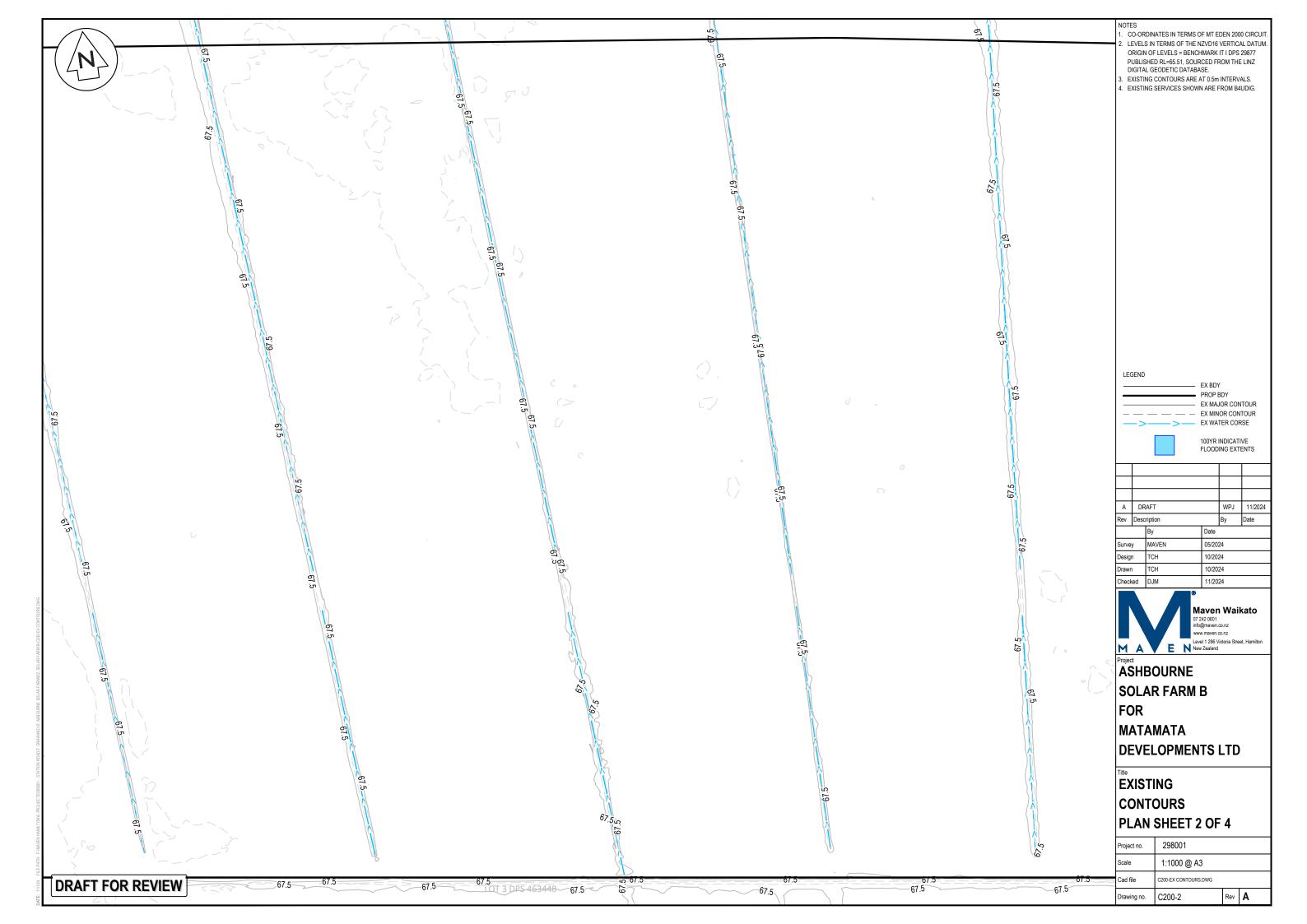


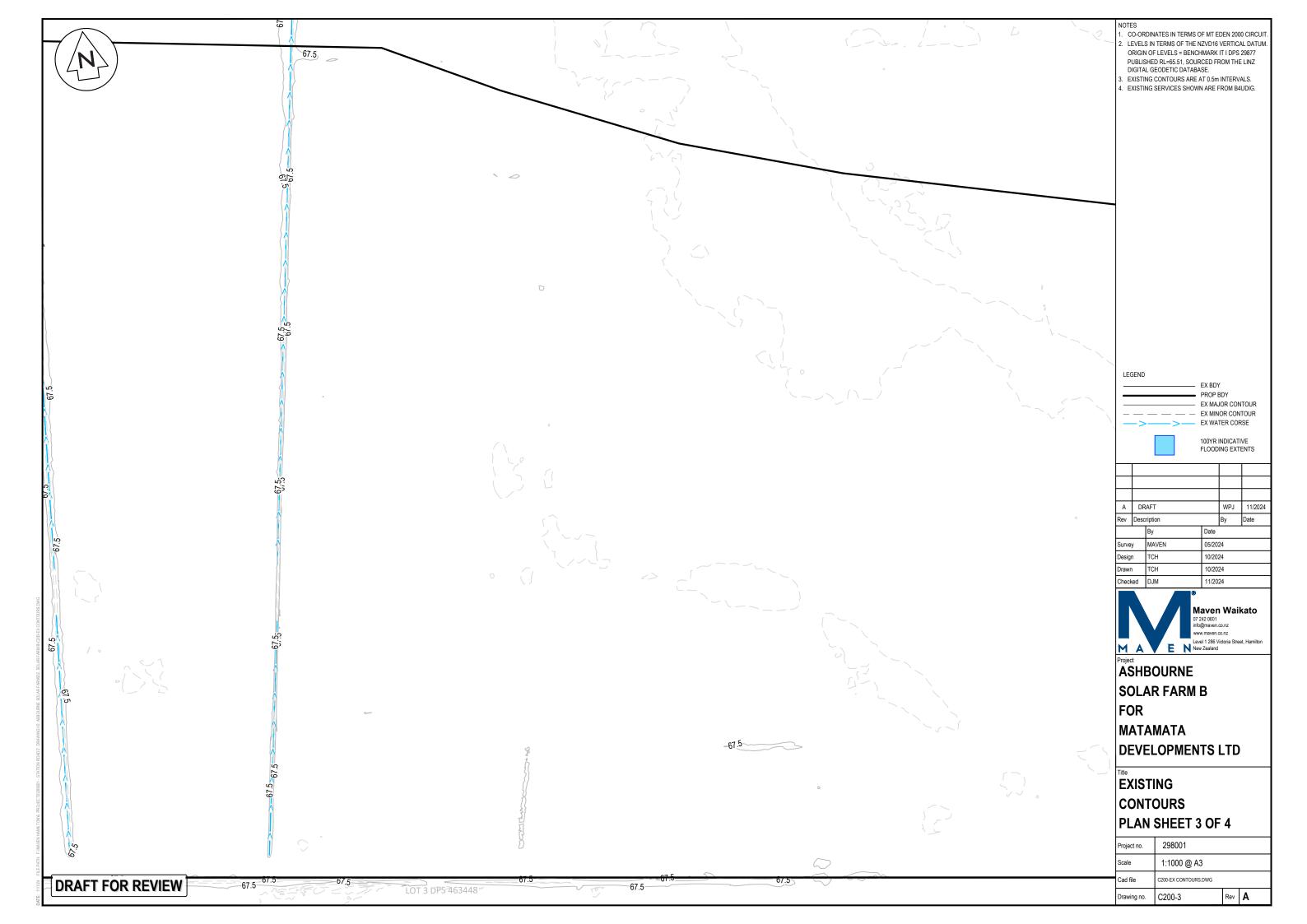




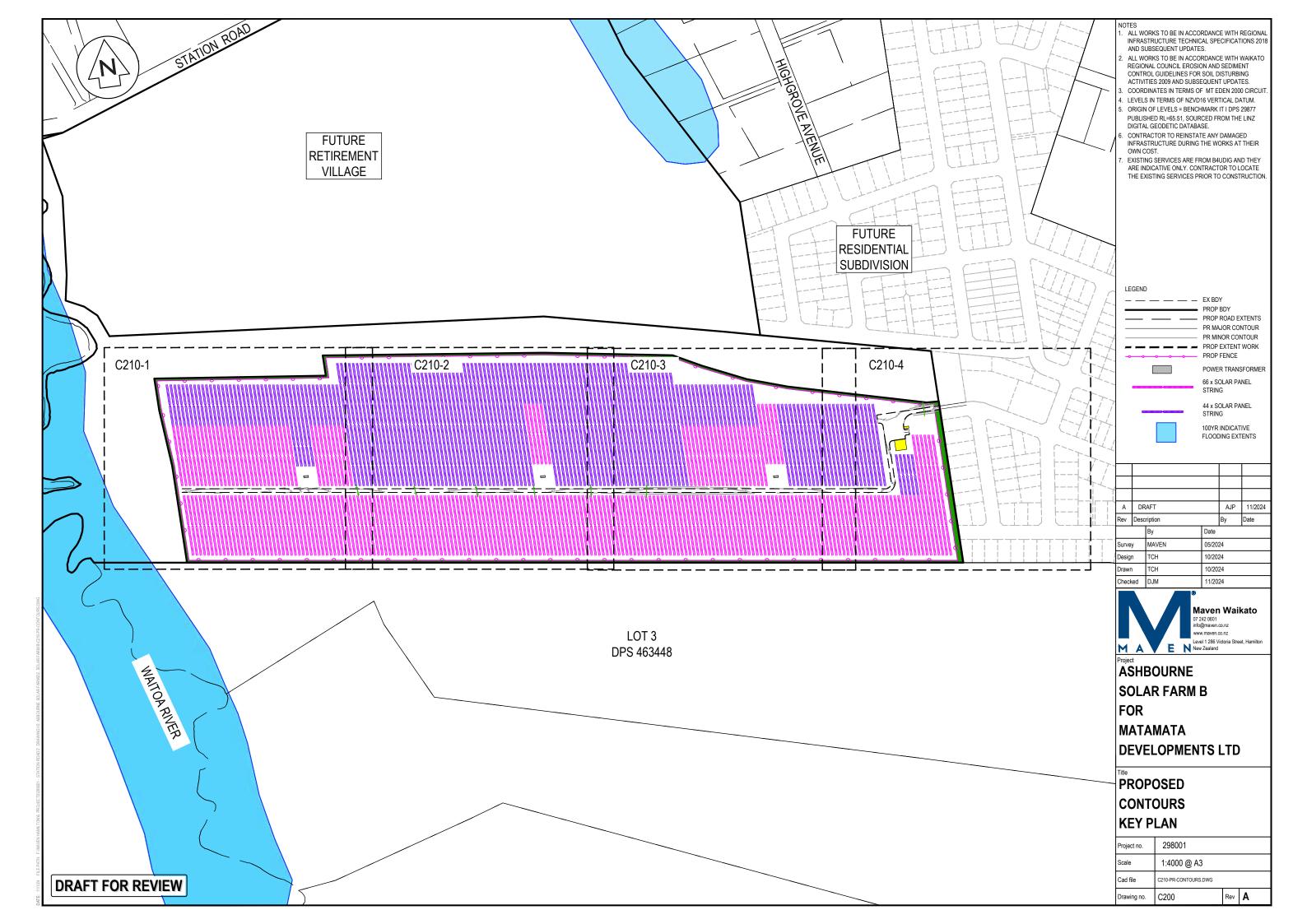


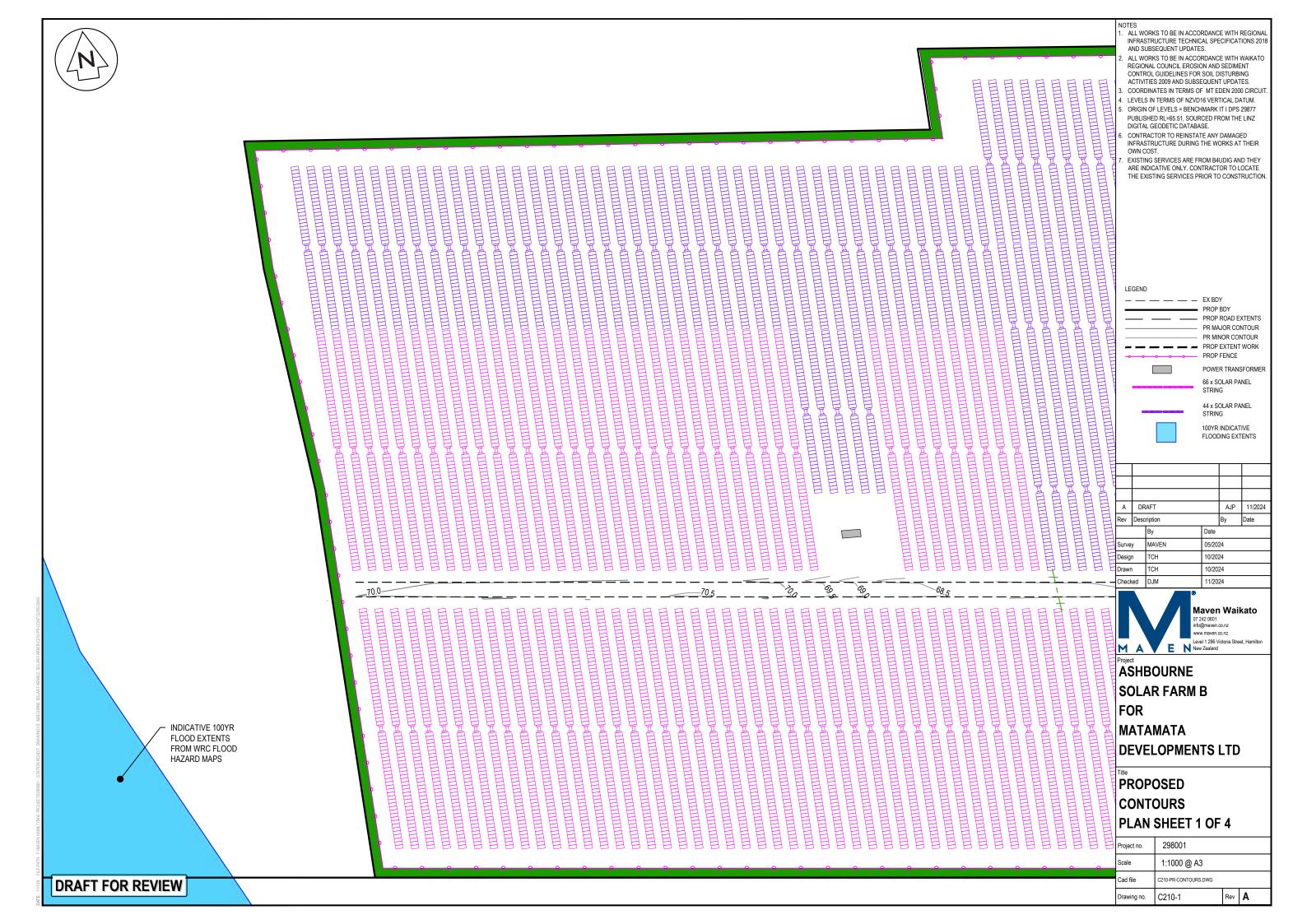


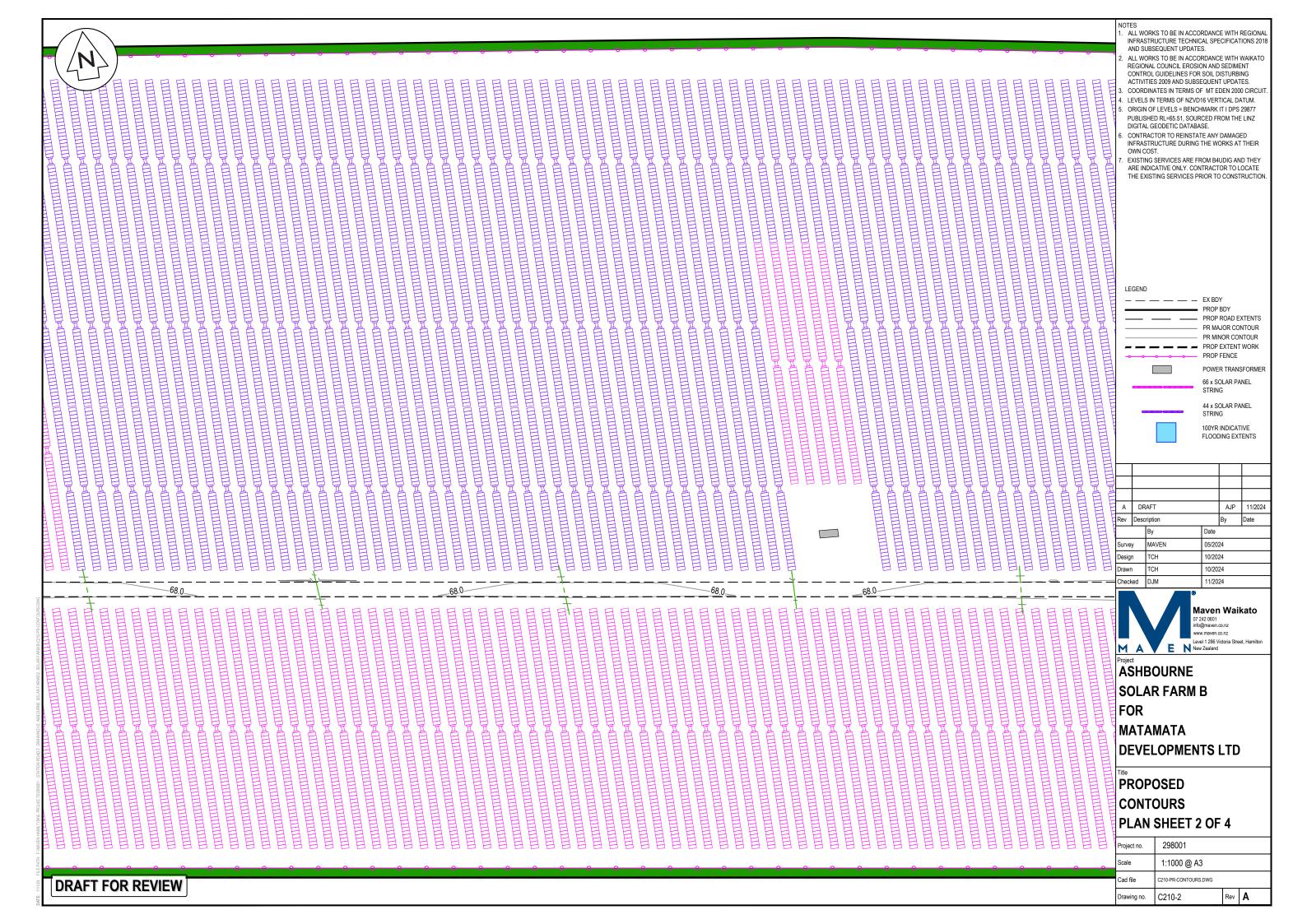


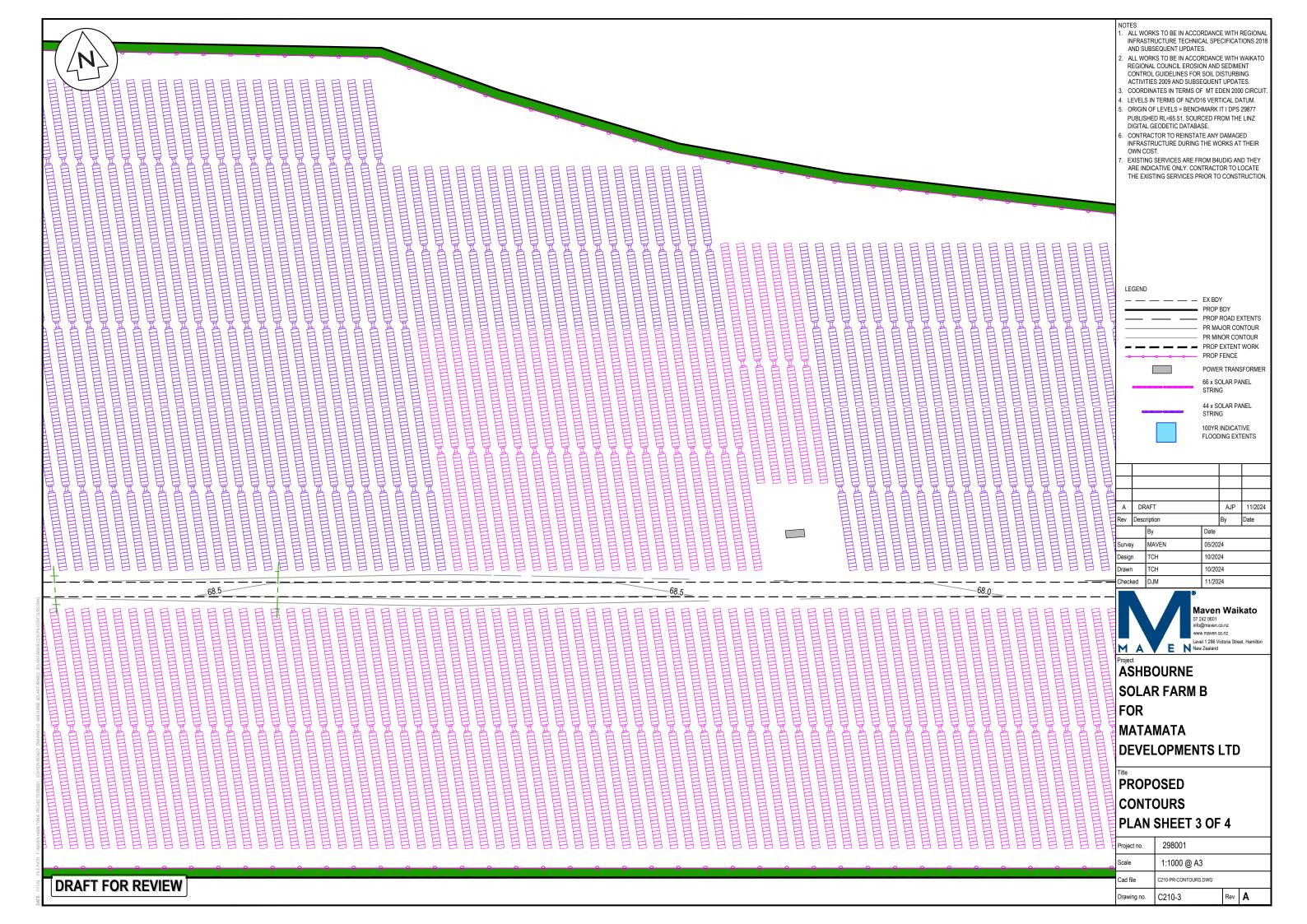




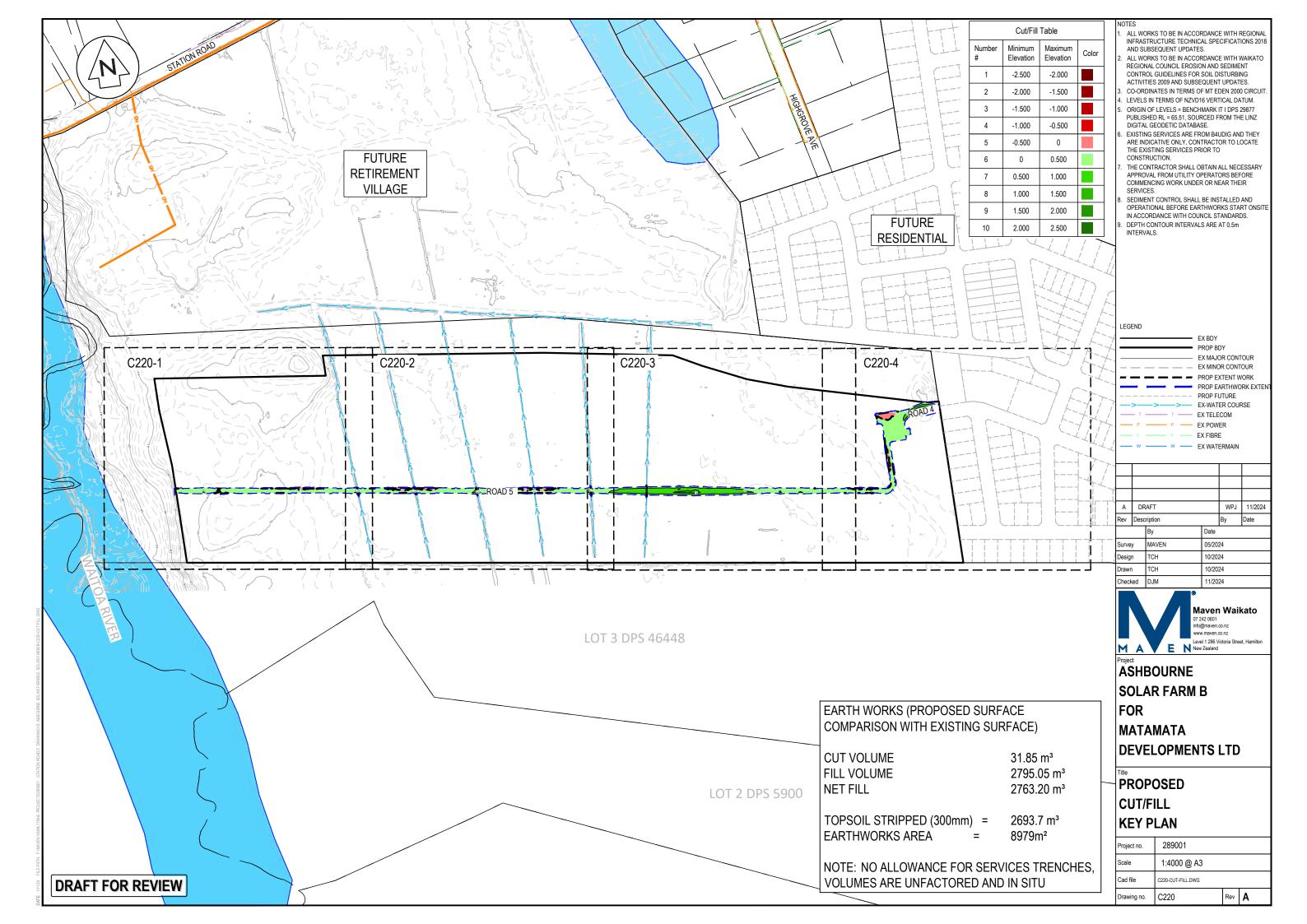


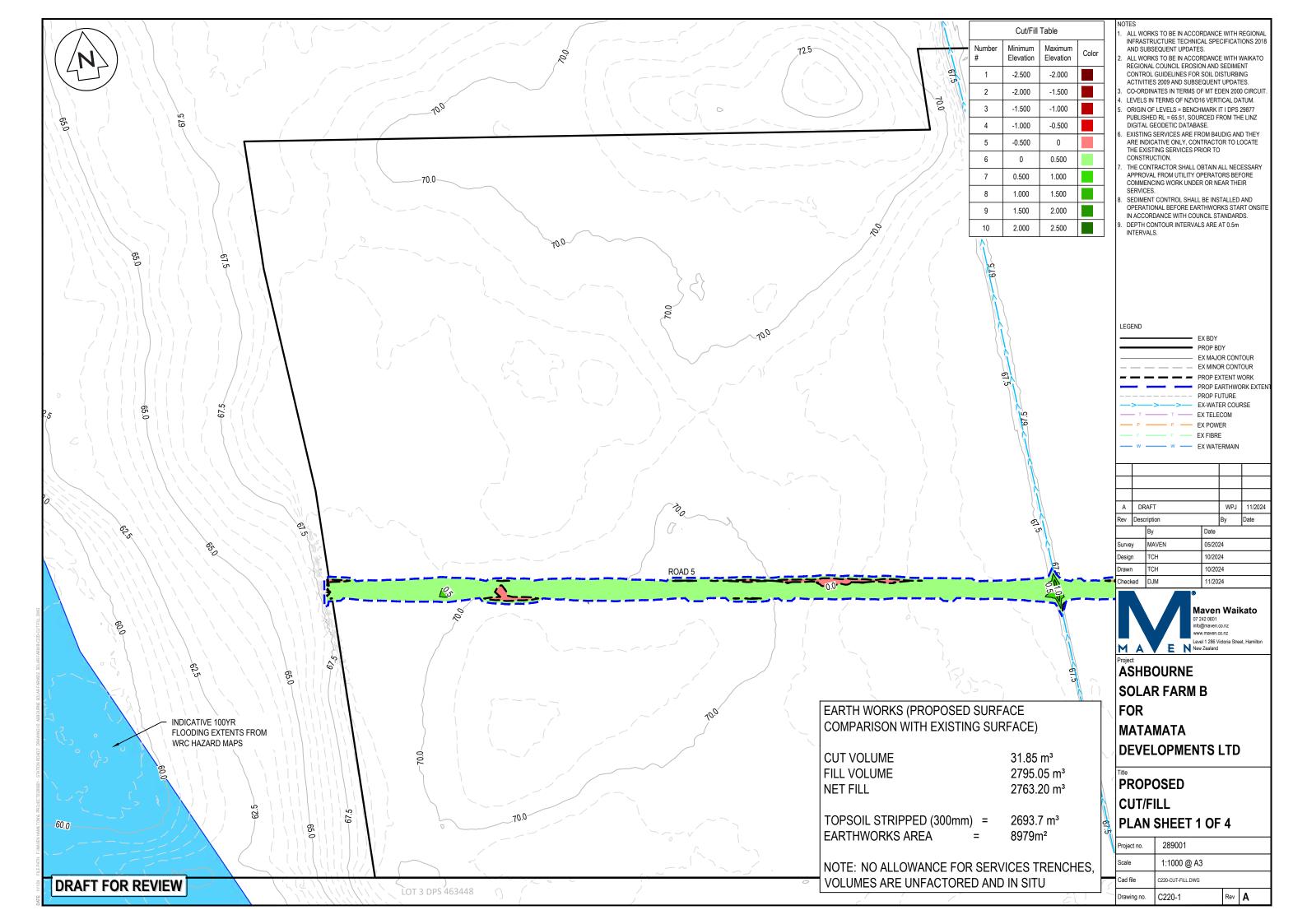


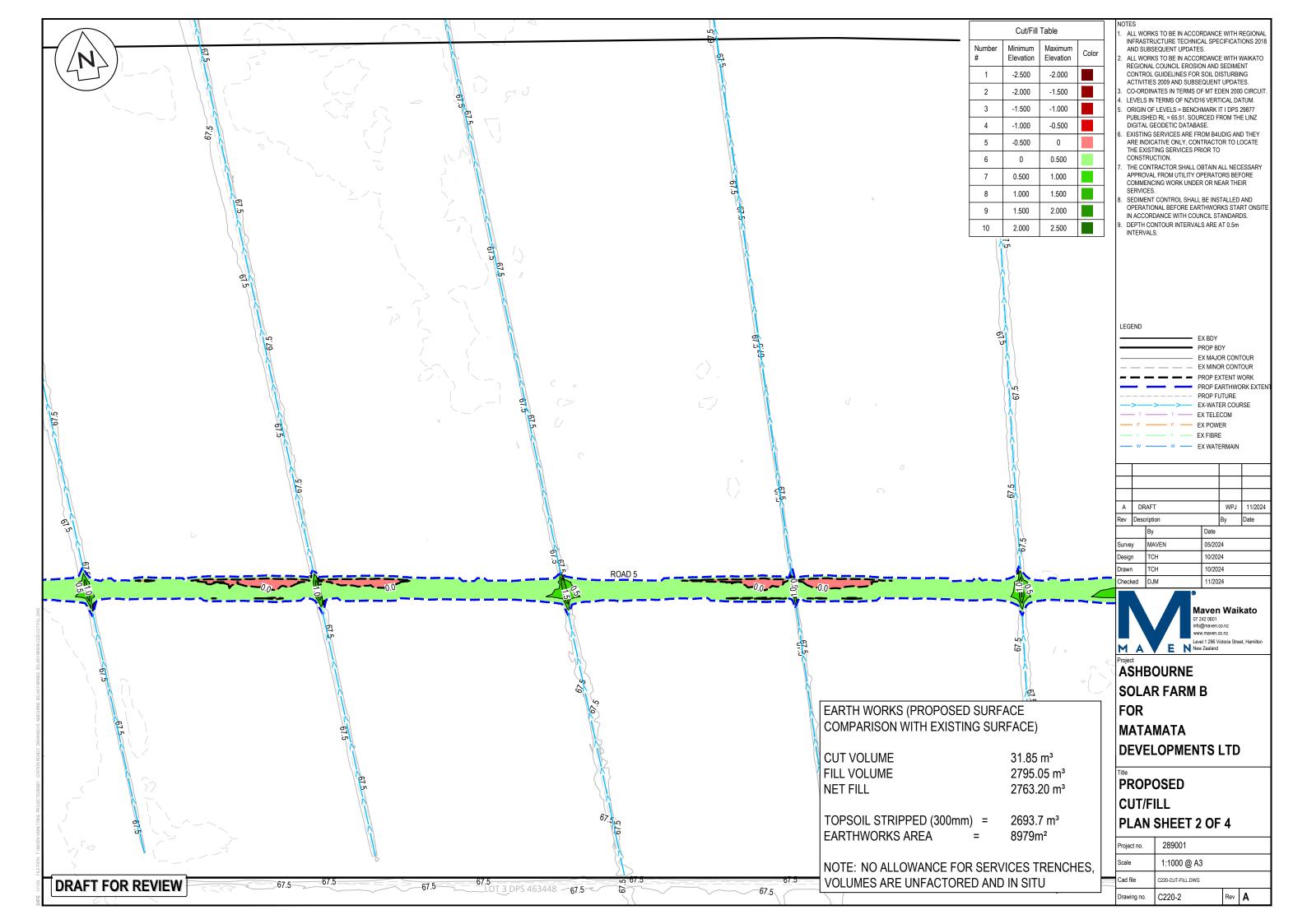


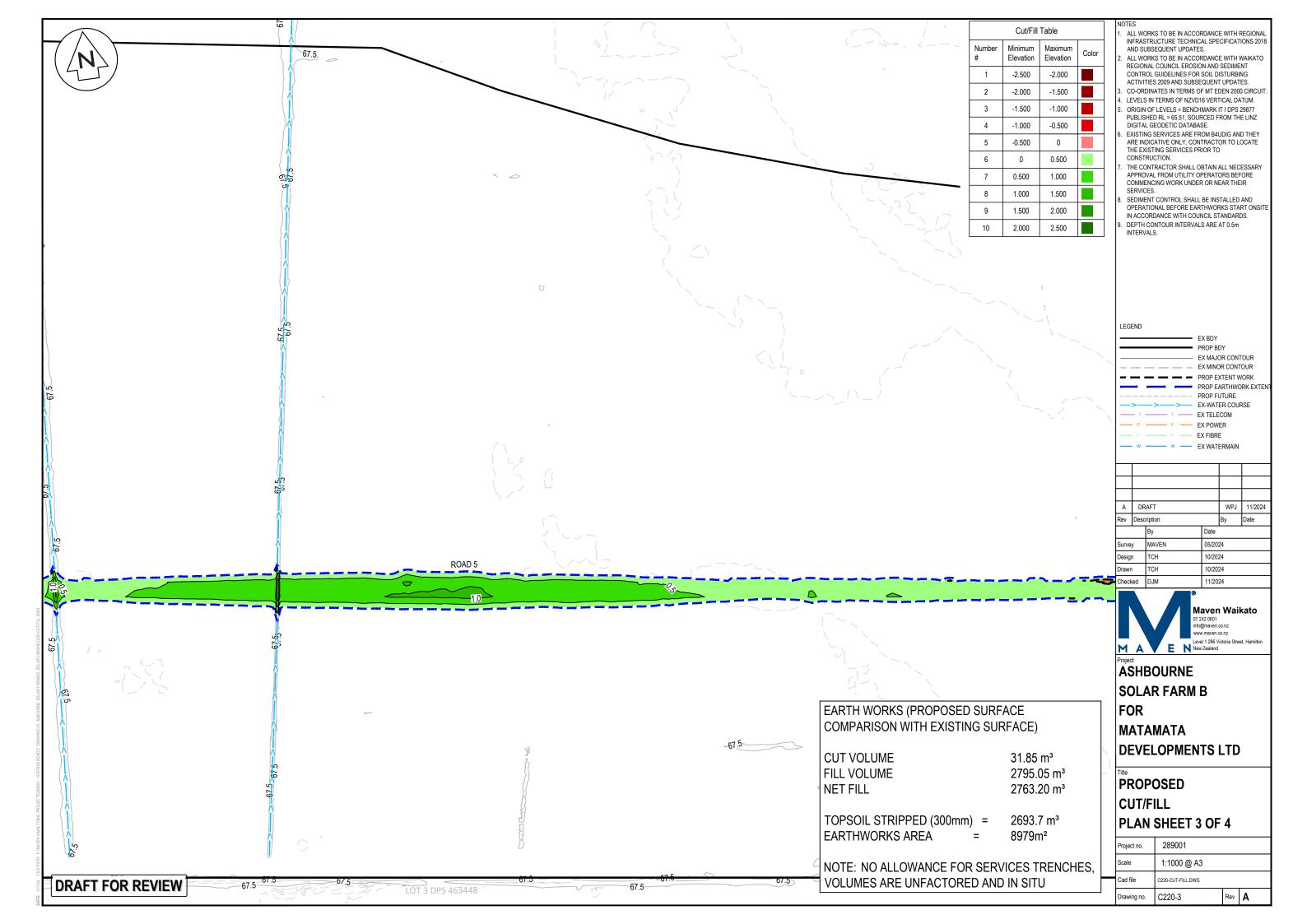


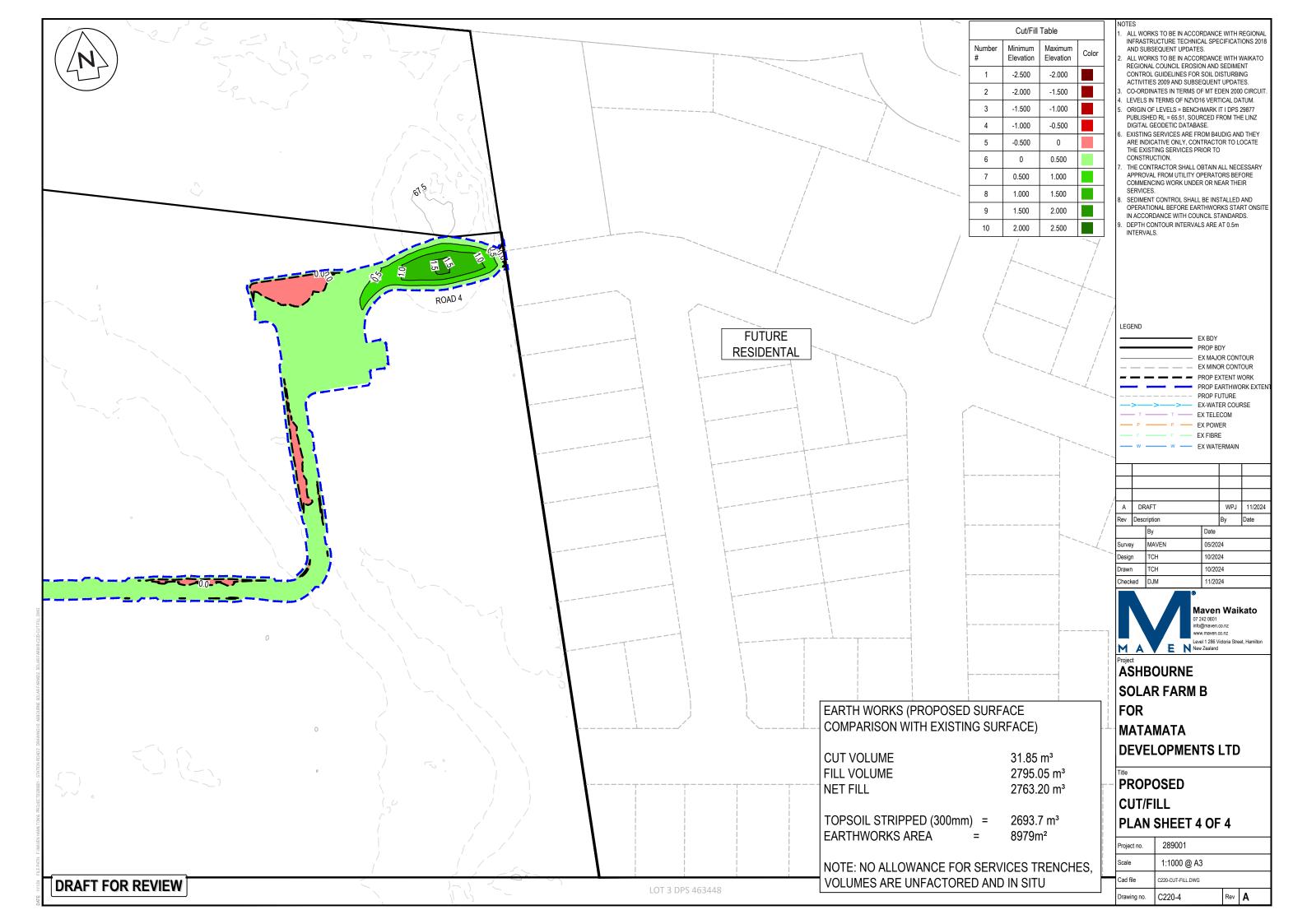


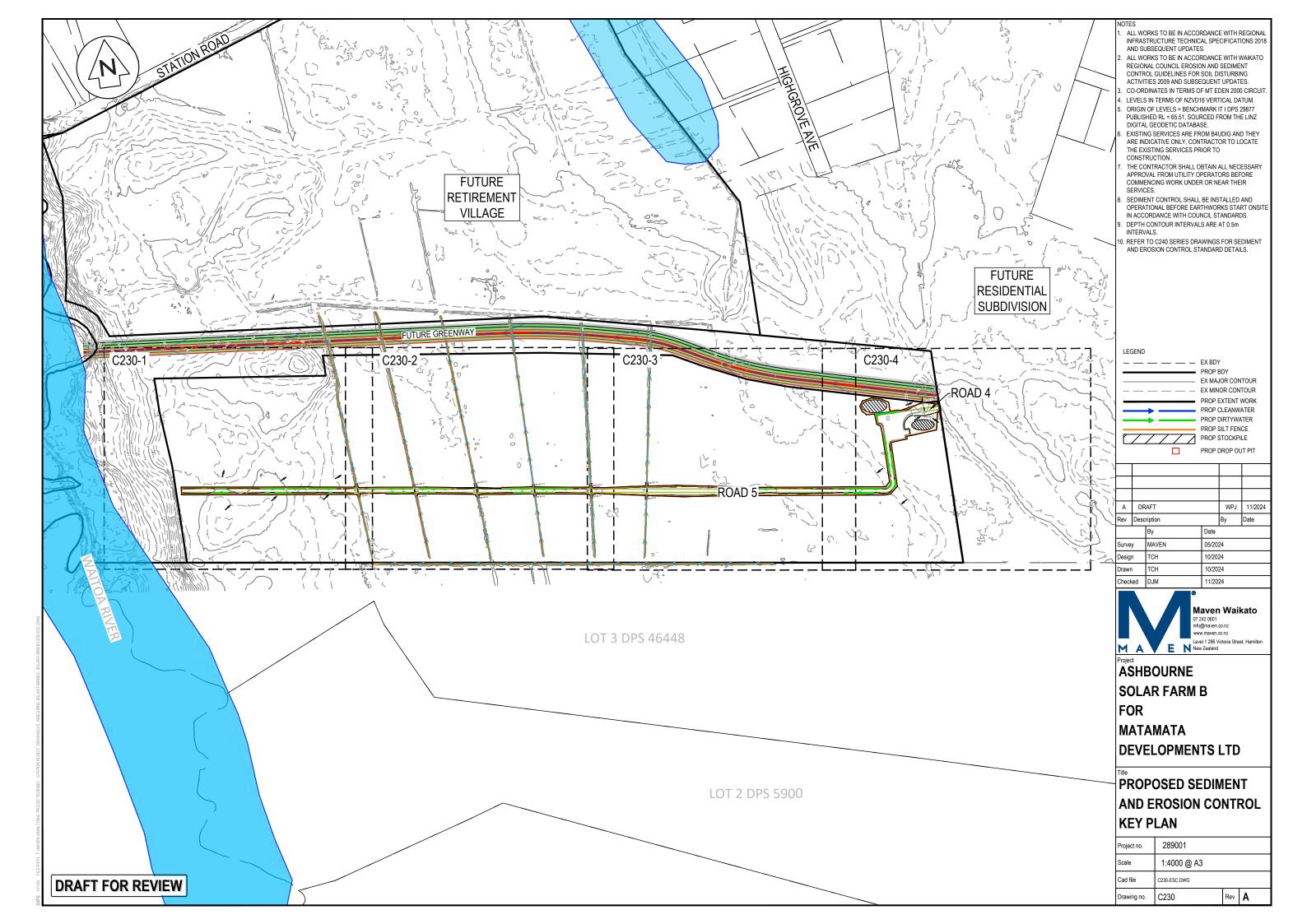


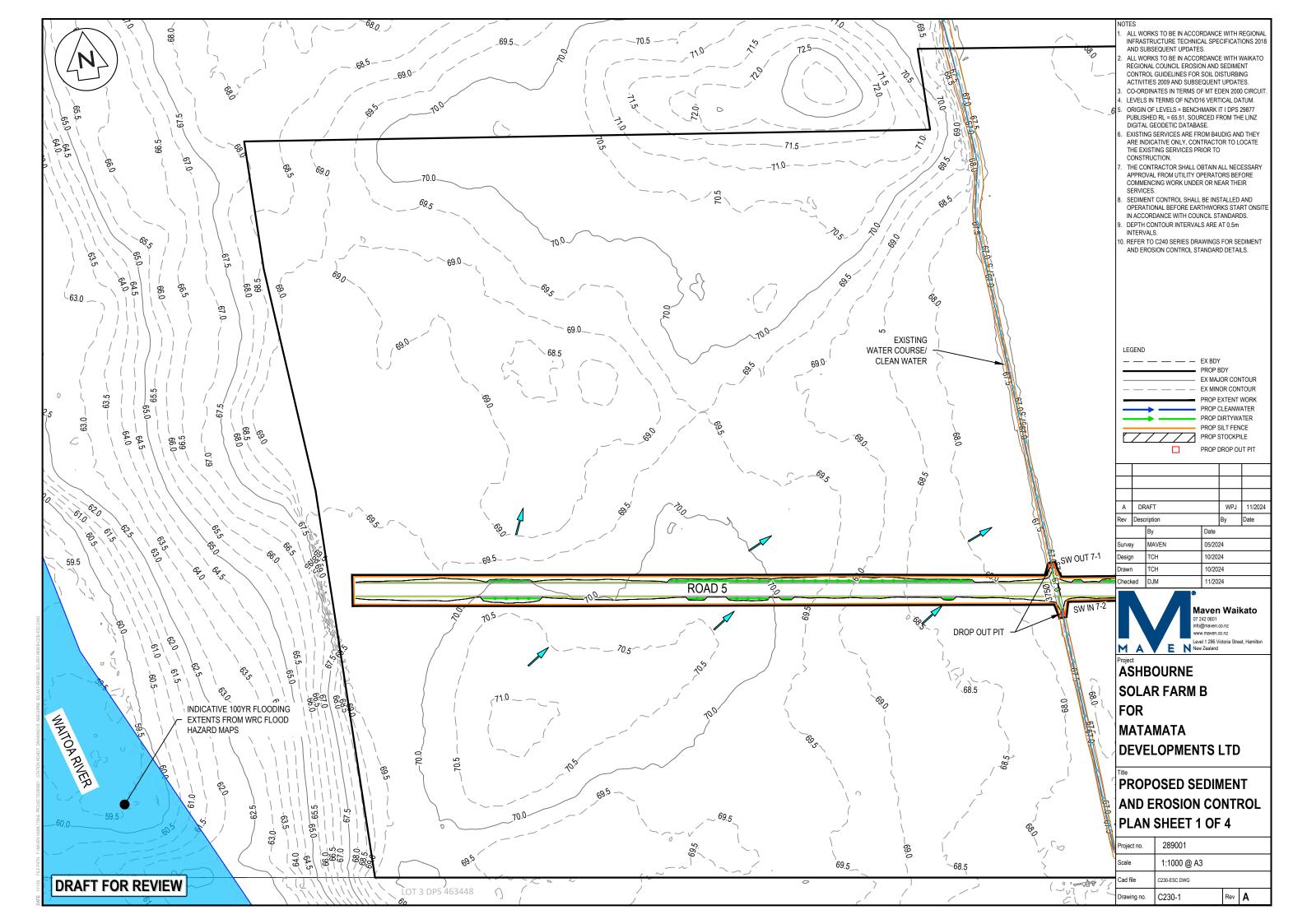


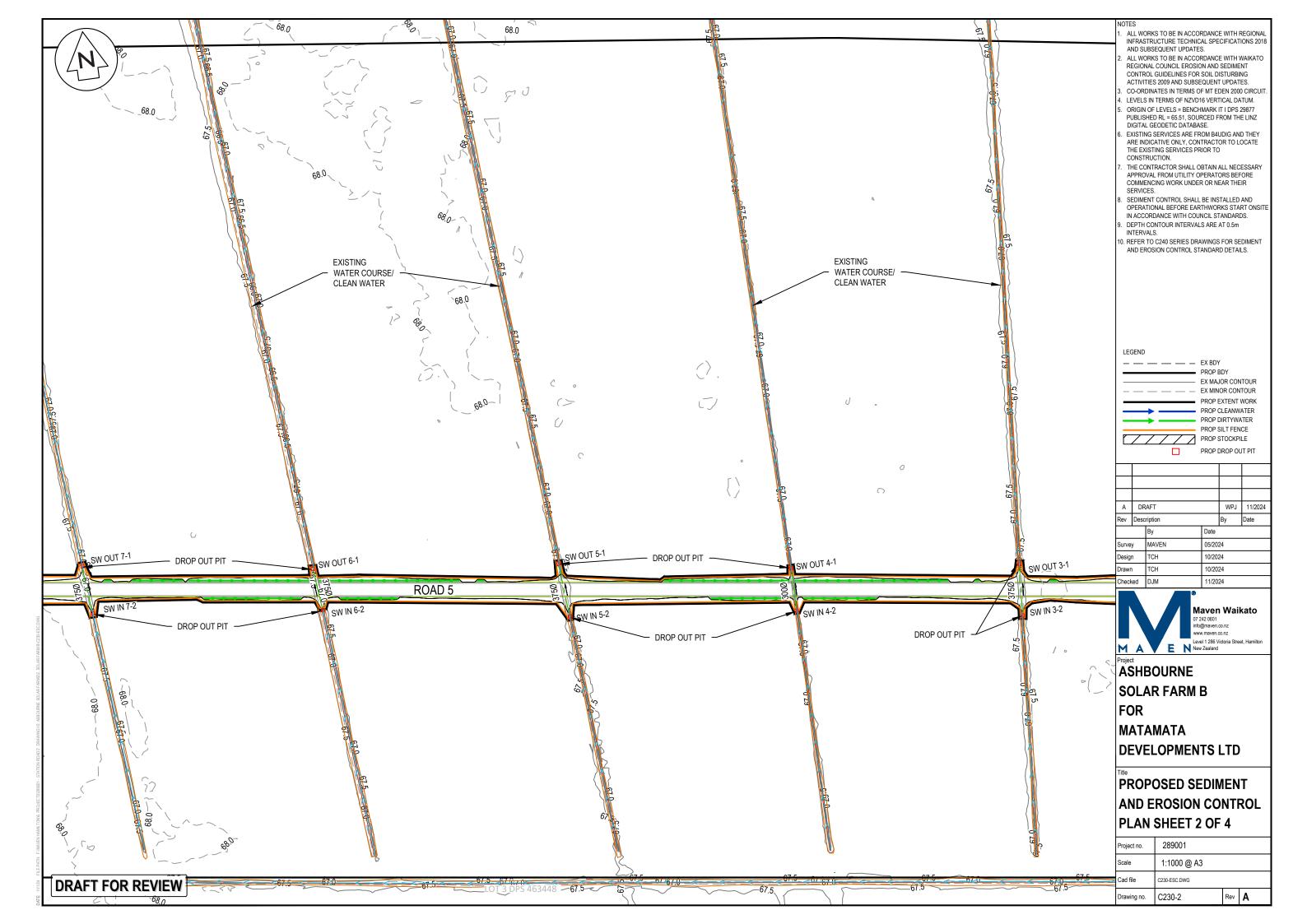


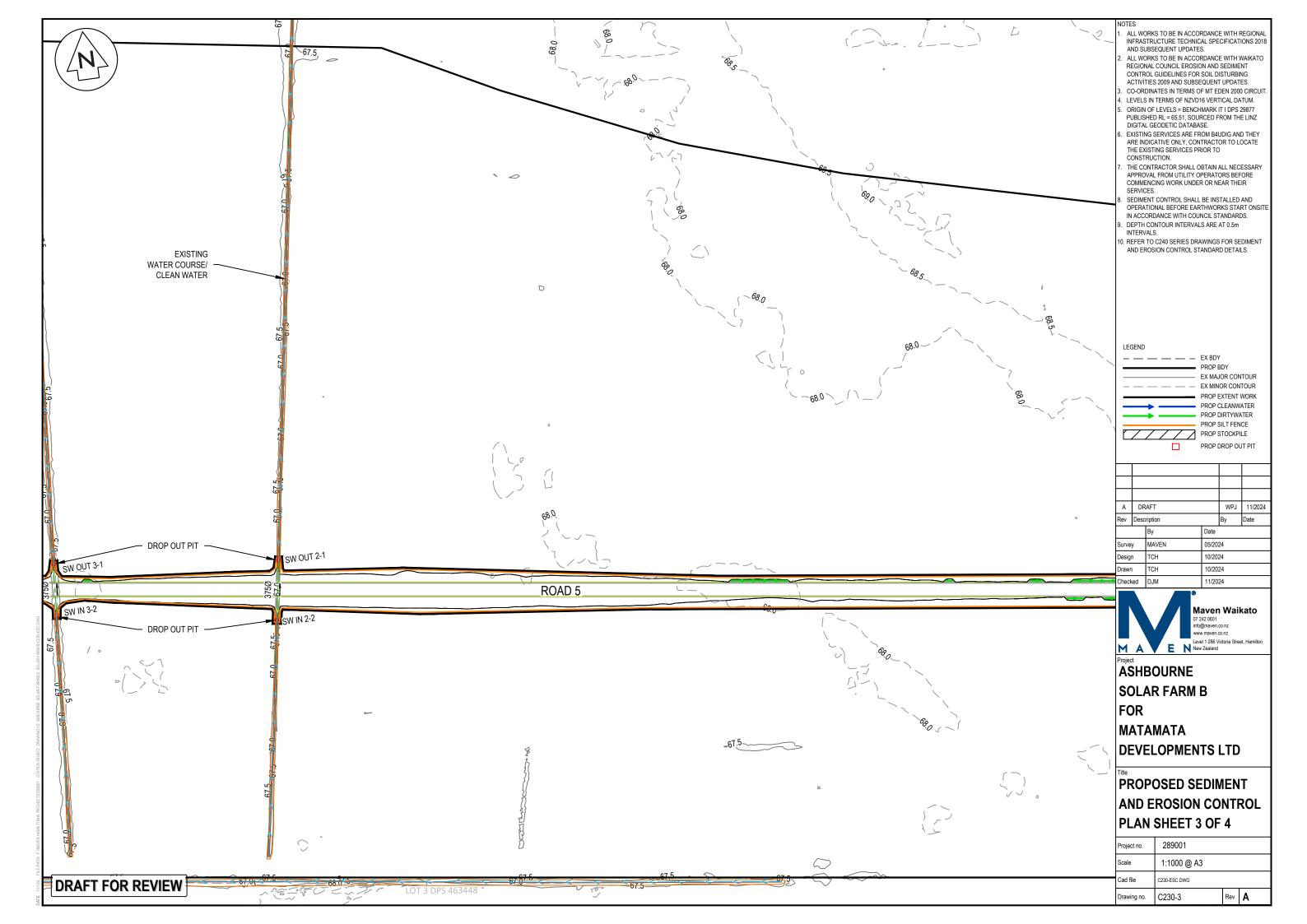


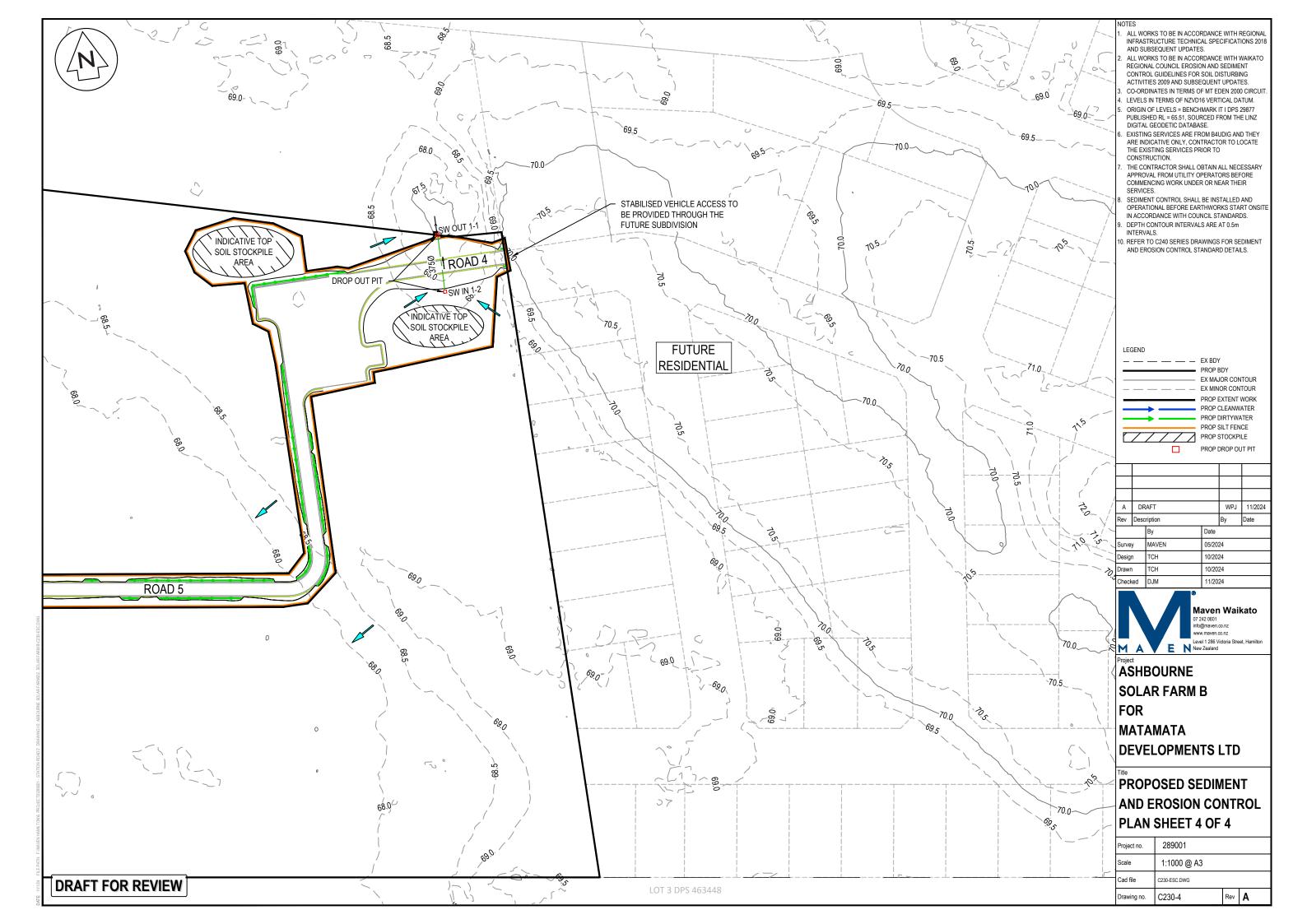


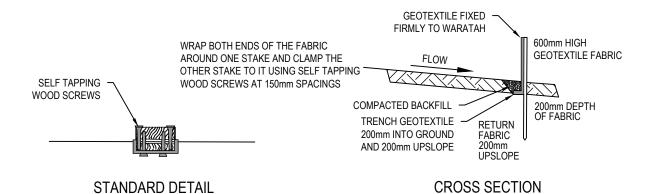






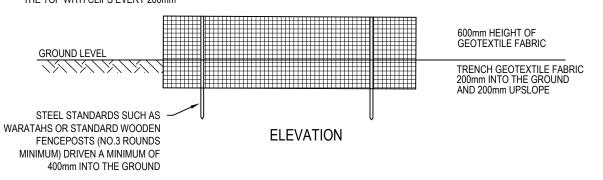


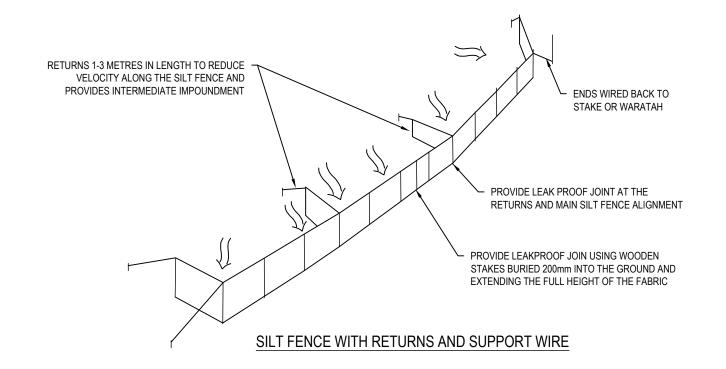




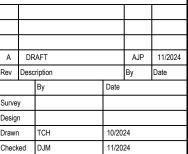
FABRIC JOIN

POST SPACING CAN BE INCREASED FROM 2 METRES TO 4 METRES IF SUPPPORTED BY A 2.5mm DIAMETER HIGH TENSILE WIRE ALONG THE TOP WITH CLIPS EVERY 200mm





- . ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 201 AND SUBSEQUENT UPDATES. . ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO
- REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2009 AND SUBSEQUENT UPDATES.
- 3. COORDINATES IN TERMS OF MT EDEN 2000 CIRCUI
- . LEVELS IN TERMS OF NZVD16 VERTICAL DATUM. . ORIGIN OF LEVELS = BENCHMARK IT I DPS 29877
- PUBLISHED RL=65.51, SOURCED FROM THE LINZ DIGITAL GEODETIC DATABASE.
- REFER TO C230 SERIES DRAWINGS FOR SEDIMENT AND EROSION CONTROL LAYOUT DRAWINGS.



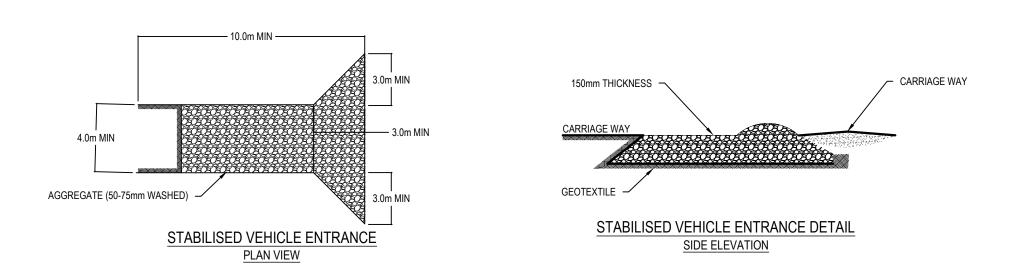


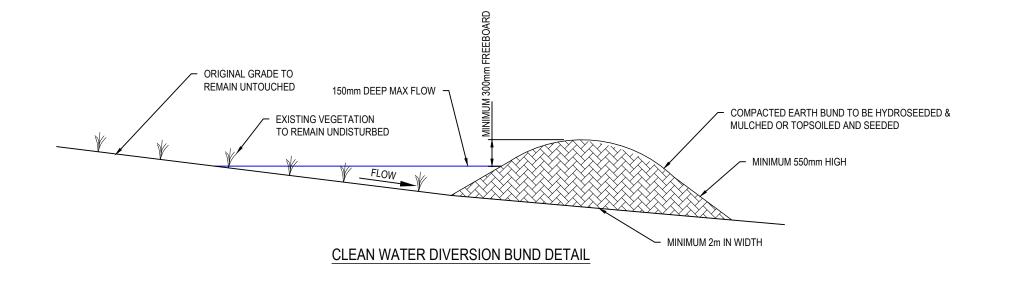
ASHBOURNE SOLAR FARM B FOR **MATAMATA DEVELOPMENTS LTD**

SEDIMENT EROSION **CONTROL DETAILS** SHEET 1 OF 2

Project no.	289001			
Scale	N.T.S			
Cad file	C240-ESC DET.DWG			
Drawing no.	C240	Rev	Α	

DRAFT FOR REVIEW





- NOTES 1. ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 201 AND SUBSEQUENT UPDATES.
- 2. ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2009 AND SUBSEQUENT UPDATES.
- 3. COORDINATES IN TERMS OF MT EDEN 2000 CIRCUI
- . LEVELS IN TERMS OF NZVD16 VERTICAL DATUM. . ORIGIN OF LEVELS = BENCHMARK IT I DPS 29877 PUBLISHED RL=65.51, SOURCED FROM THE LINZ
- DIGITAL GEODETIC DATABASE. REFER TO C230 SERIES DRAWINGS FOR SEDIMENT AND EROSION CONTROL LAYOUT DRAWINGS.

Α	DRAFT			AJP	11/2024	
Rev	Desc	cription			Ву	Date
	By Date		Date			
Surve	y					
Design						
Drawn		TCH		10/2024		
Checked DJM 11/2024						

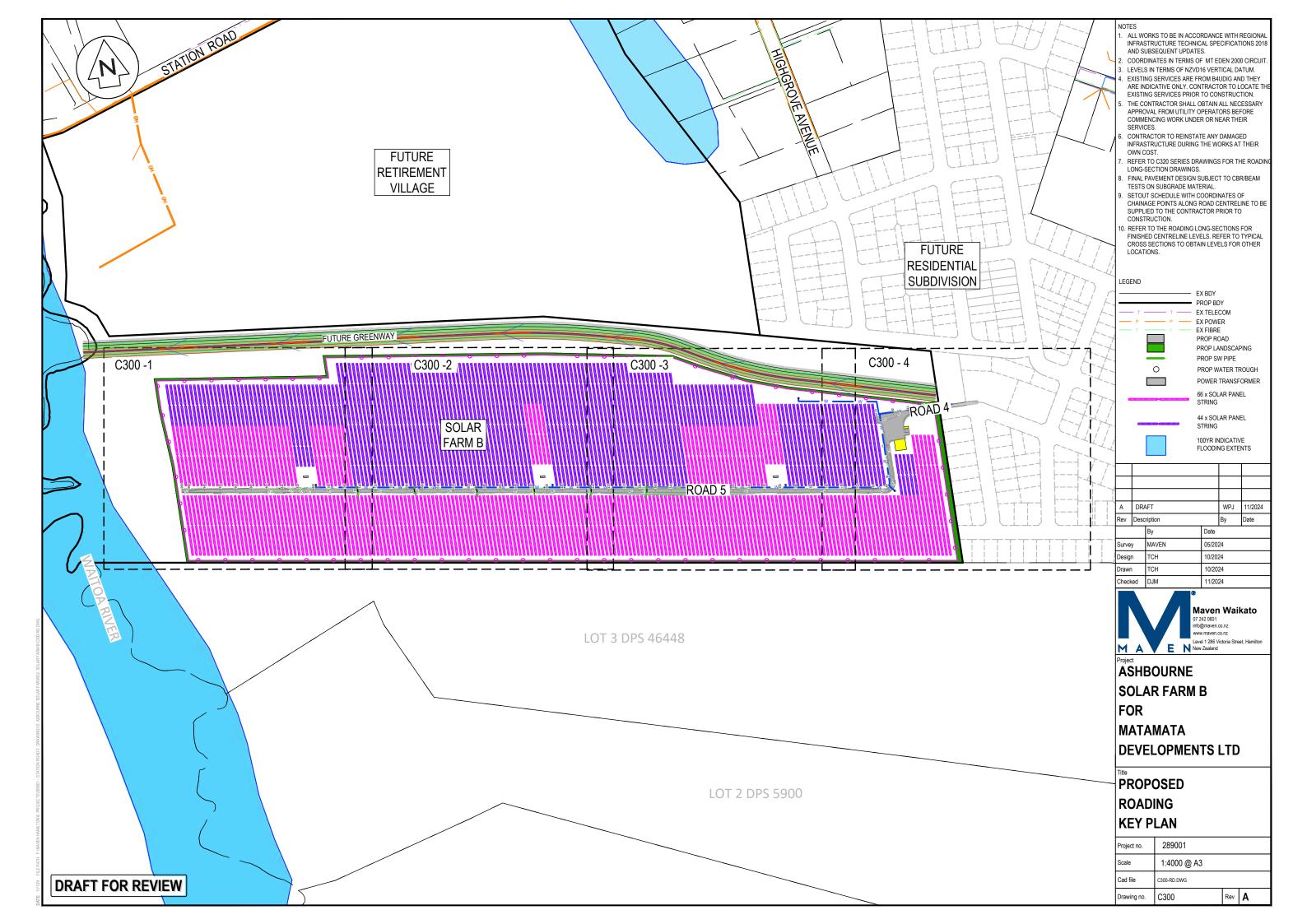


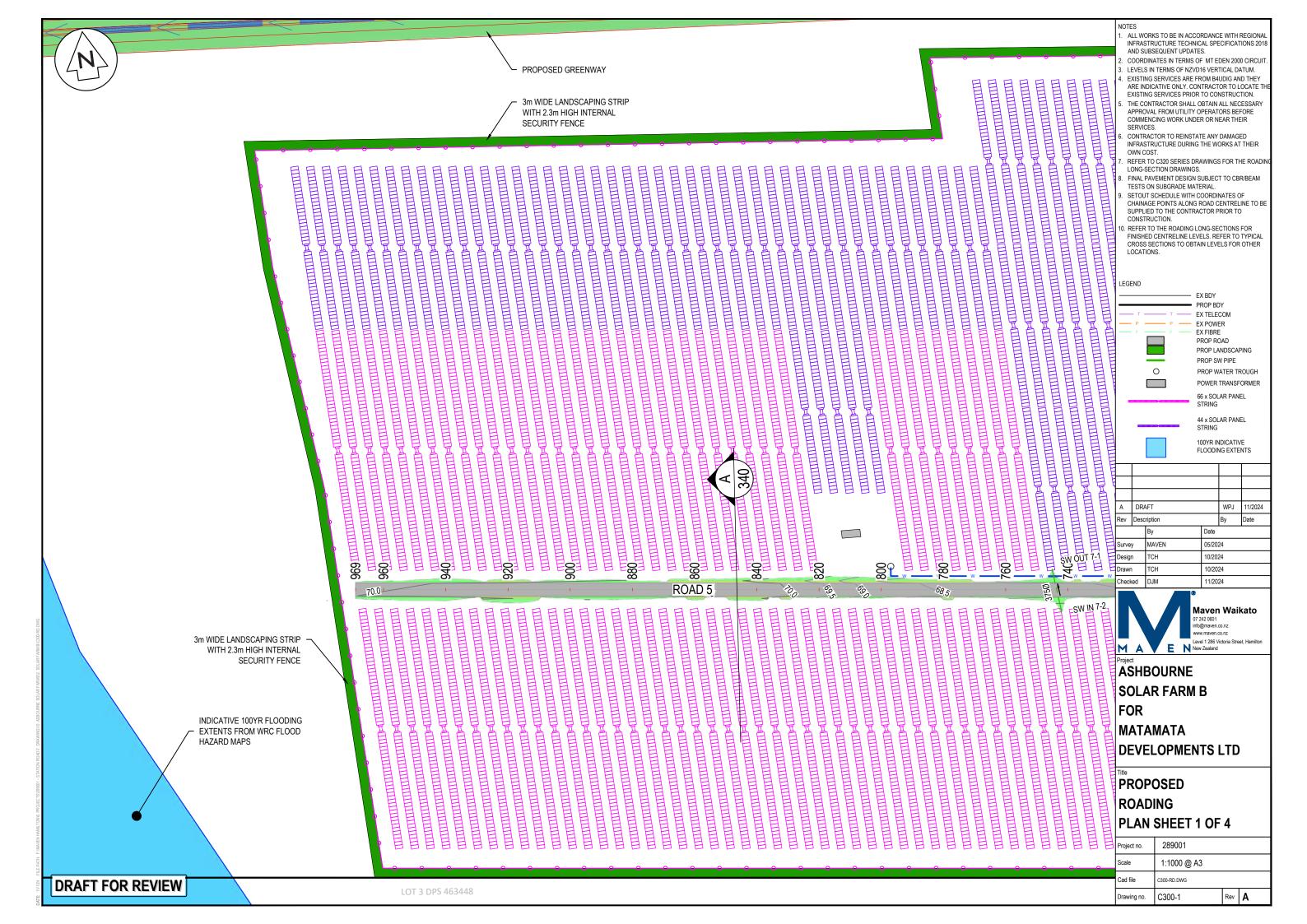
ASHBOURNE **SOLAR FARM B** FOR MATAMATA **DEVELOPMENTS LTD**

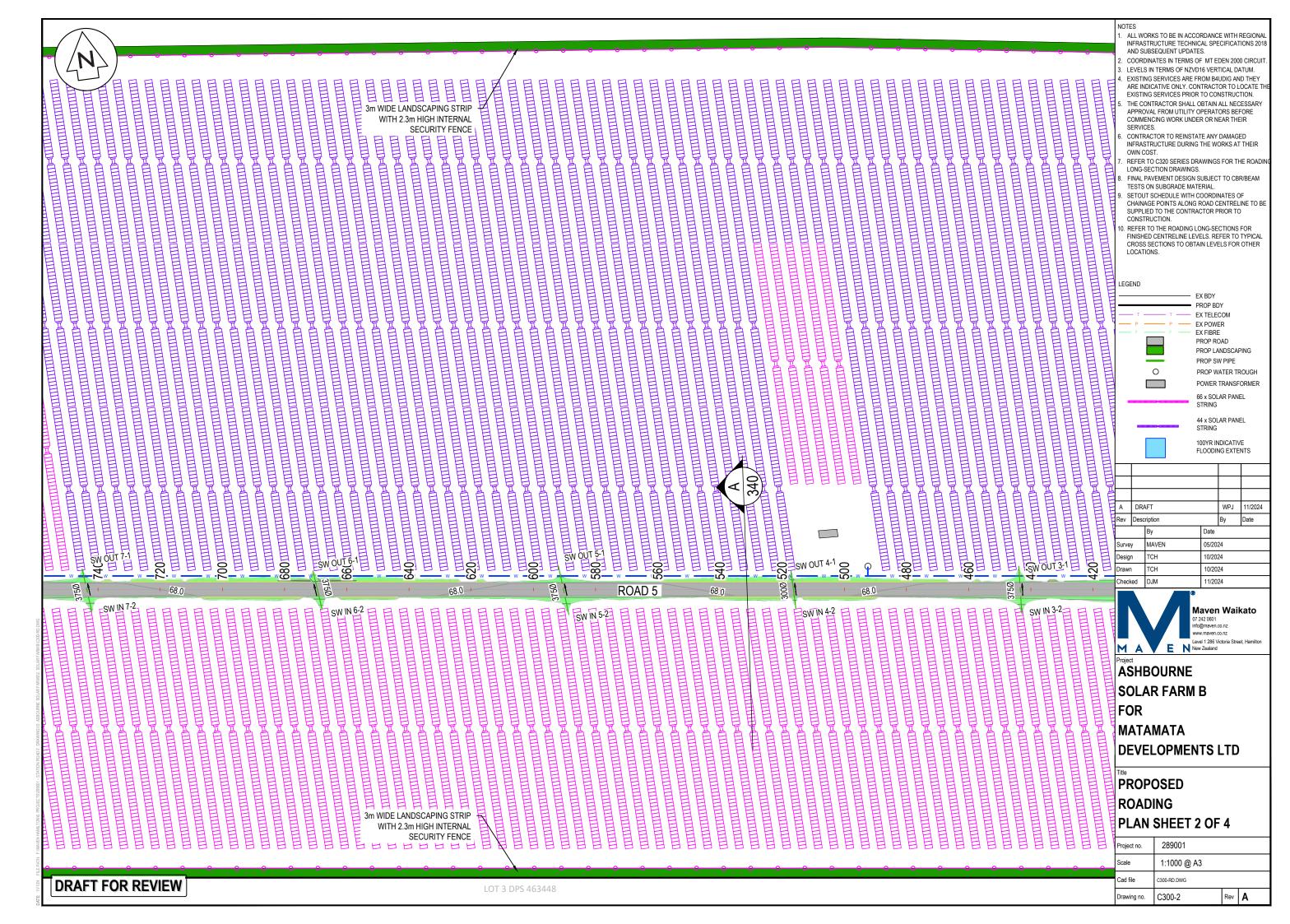
SEDIMENT EROSION CONTROL DETAILS SHEET 2 OF 2

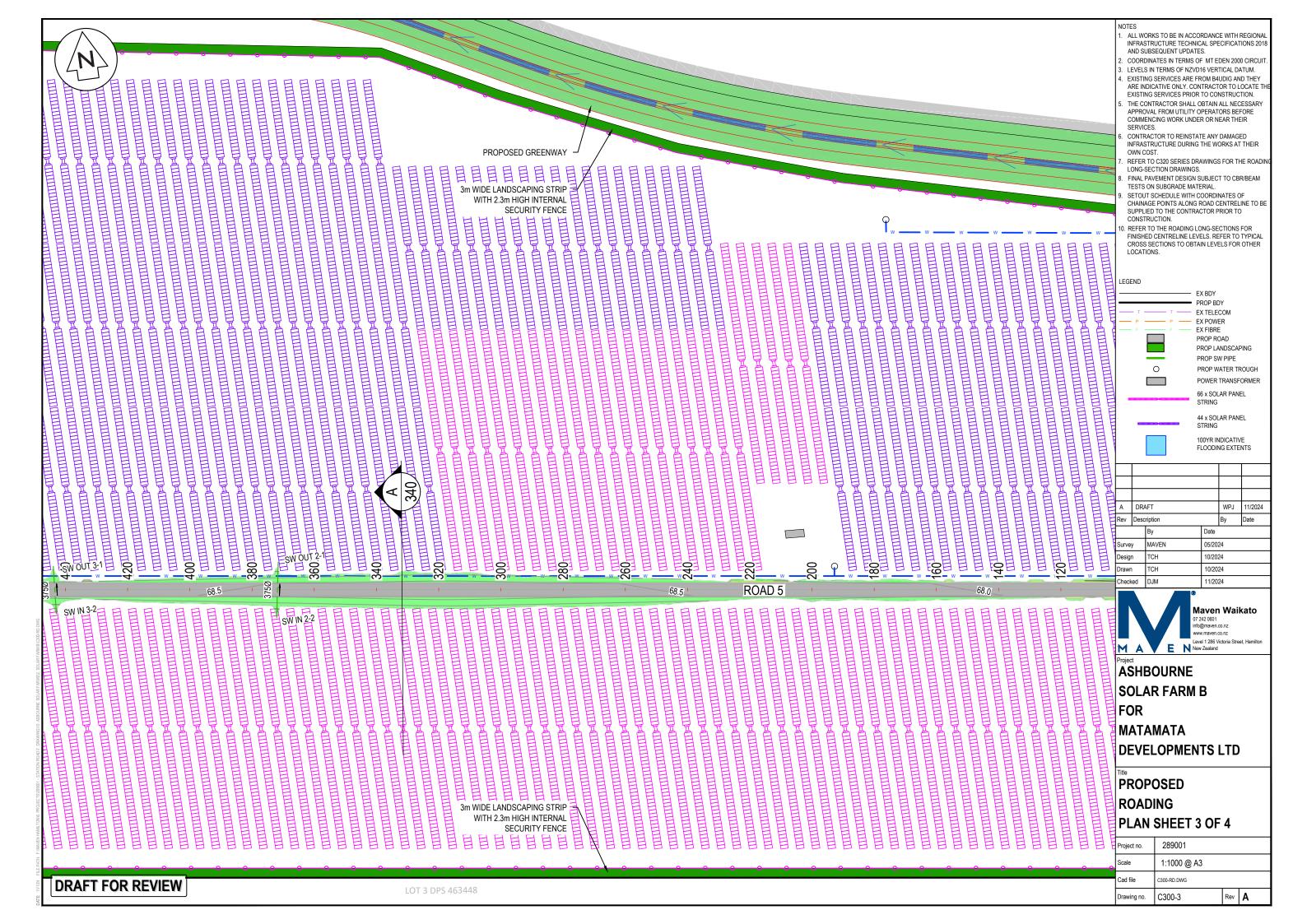
Project no.	289001			
Scale	N.T.S			
Cad file	C240-ESC DET.DWG			
Drawing no.	C240-1	Rev	Α	

DRAFT FOR REVIEW











	Ē		1			
DATUM RL = 65.0						
HORIZONTAL GEOMETRY						
VERTICAL GEOMETRY		-(0.7	75%		
EXISTING LEVELS	92.69	67.82	5.00	68.97	68.91	68.89
DESIGN LEVELS	69.53	69.38	03.50	80.69	00 00 00 00	68.92
CUT/FILL	-0.23	9.59	6.0	0.11	0.02	0.03
CHAINAGE	0.00	20.00	10.00	60.00	80.00	81.33

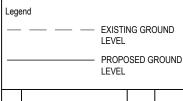
ROAD 4 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

SAG $\overline{\Delta}$ DATUM RL = 65.0 R=10.00 HORIZONTAL GEOMETRY L=17.17 -0.57% -3% 0.5% _VC=10m VC=20m **VERTICAL GEOMETRY** K=4.11 K=5.71 68.54 69.79 EXISTING LEVELS 68.87 68.76 68.63 68.59 68.00 67.61 67.64 67.68 68.92 68.83 67.73 67.70 67.75 67.85 68.79 68.77 68.65 68.23 68.09 DESIGN LEVELS CUT/FILL 0.17 120.00 100.00 41.06 69.53 79.53 CHAINAGE

> **ROAD 5 LONGITUDINAL SECTION** SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE



Α	DRA	DRAFT		WPJ	11/2024	
Rev	Desc	escription			Ву	Date
	By Date		Date			
Surve	у	MAVEN		05/2024		
Desig	n	TCH		10/2024		
Drawr	1	TCH		10/2024		
Check	red	DJM		11/2024		·

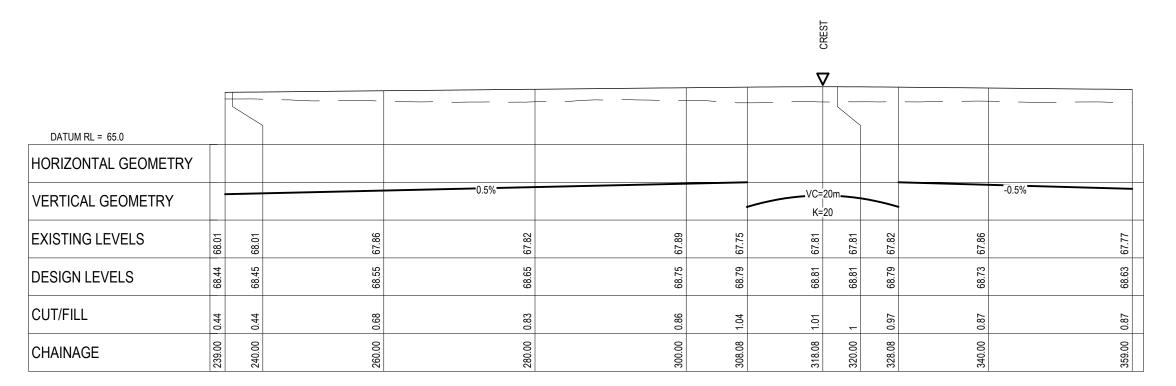


ASHBOURNE **SOLAR FARM B** FOR MATAMATA **DEVELOPMENTS**

PROPOSED **ROADING LONGSECTIONS**

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320	Rev	Α

	F	_						
DATUM RL = 65.0								
HORIZONTAL GEOMETRY								
VERTICAL GEOMETRY					0.59	6		
EXISTING LEVELS	29.79	89.79	02.70	67.72	67.73	77.79	68.18	68.00
DESIGN LEVELS	67.85	67.85	67.95	68.05	68.15	68.25	68.35	68.45
CUT/FILL	0.17	0.17	0.25	0.33	0.41	0.48	0.17	0.44
CHAINAGE	119.50	120.00	140.00	160.00	180.00	200.00	220.00	239.50



ROAD 5 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- . COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE

Legend	
	EXISTING GROUND LEVEL
	PROPOSED GROUND LEVEL

Α	DRA	AFT		WPJ	11/2024	
Rev	Desc	ription		Ву	Date	
		Ву	Date			
Surve	у	MAVEN 0		05/2024		
Desig	n	TCH	10/2024			
Drawr	1	TCH	10/20	10/2024		
Check	ked	DJM	11/20	11/2024		

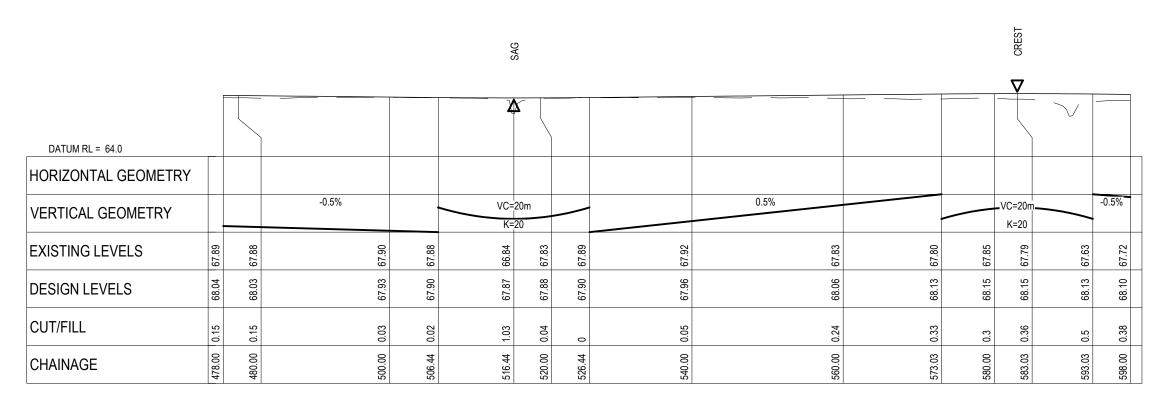


ASHBOURNE **SOLAR FARM B** FOR MATAMATA **DEVELOPMENTS**

PROPOSED **ROADING** LONGSECTIONS

Project no.	289001								
Scale	AS SHOWN								
Cad file	C320-RD LS.DWG								
Drawing no.	C320-1	Rev	Α						

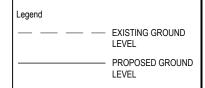
	Г							
DATUM RL = 64.0								
HORIZONTAL GEOMETRY								
VERTICAL GEOMETRY					-0.5%)) 		
VERTICAL GLOWLTRT								
EXISTING LEVELS	67.77	67.77	67.78	67.79	67.89	67.84	67.88	67.89
DESIGN LEVELS	68.64	68.63	68.53	68.43	68.33	68.23	68.13	68.04
CUT/FILL	0.87	0.86	0.75	0.64	0.44	0.39	0.25	0.15
CHAINAGE	358.50	360.00	380.00	400.00	420.00	440.00	460.00	478.50



ROAD 5 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE
 WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE



Α	DRA	AFT		WPJ	11/2024
Rev	Desc	ription		Ву	Date
		Ву	Date		
Surve	у	MAVEN 05/		05/2024	
Design		TCH	10/2024		
Drawn	1	TCH	10/2024		
Check	ed	DJM	11/2024		

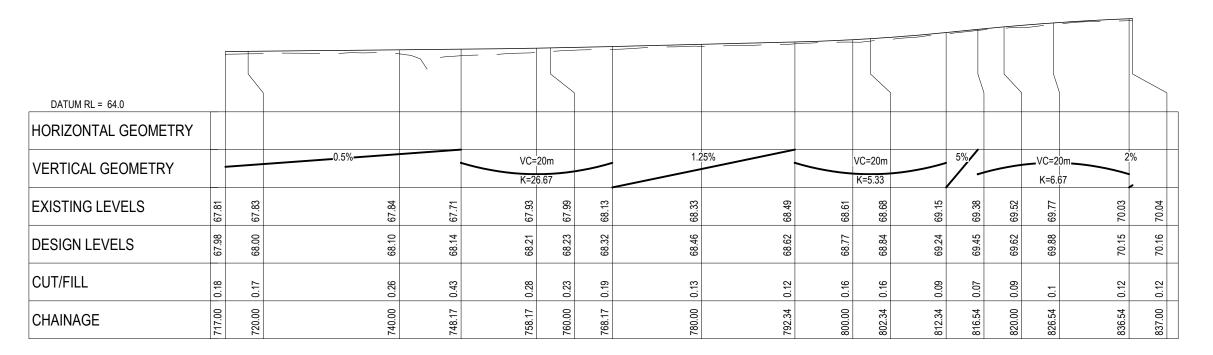


ASHBOURNE **SOLAR FARM B** FOR **MATAMATA DEVELOPMENTS**

PROPOSED **ROADING** LONGSECTIONS

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-2	Rev	Α

	F	_									
							_	•			
DATUM RL = 64.0											
HORIZONTAL GEOMETRY											
VERTICAL GEOMETRY				-0.5%			VC=			0.5%	
EXISTING LEVELS	67.71	67.76	67.75	67.74	92 76	5	67.75	67.77	67.77	67.88	67.81
DESIGN LEVELS	68.11	68.09	67.99	68.79	08 29	8	67.77	67.80	67.80	06.79	67.99
CUT/FILL	0.4	0.34	0.25	0.15	0 04	5	0.04	0.03	0.03	0.02	0.18
CHAINAGE	597.50	00.009	620.00	640.00	659 58		660.00	679.58	00:089	700.00	717.50



ROAD 5 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE

Legend	
	EXISTING GROUND LEVEL
	PROPOSED GROUND LEVEL

Α	DRA	AFT		WPJ	11/2024
Rev	Desc	ription		Ву	Date
		Ву	Date		
Surve	у	MAVEN	05/2024		
Desig	ın	TCH	10/20	24	
Draw	n	TCH	10/20	24	
Chec	ked	DJM	11/20	24	

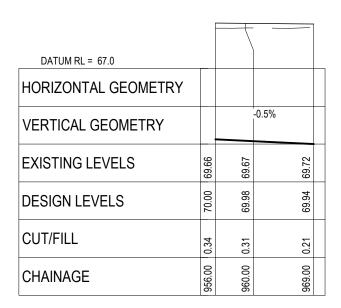


ASHBOURNE **SOLAR FARM B** FOR MATAMATA **DEVELOPMENTS**

PROPOSED **ROADING LONGSECTIONS**

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-3	Rev	Α

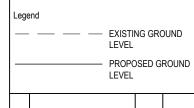
						O							
						∇							
	Ī	T											
DATUM RL = 67.0													
HORIZONTAL GEOMETRY													
VERTICAL GEOMETRY	VC=2	20m 2%			_VC=20m.						-0.5%		
	K=6	.67			K=8								
EXISTING LEVELS	70.03	70.03	70.13	70.23	70.32	70.38	70.38	70.32	70.07	69.93	70.16	69.58	99.69
DESIGN LEVELS	70.15	70.15	70.22	70.31	70.45	70.47	70.47	70.46	70.38	70.28	70.18	70.08	70.00
CUT/FILL	0.12	0.12	0.1	0.08	0.12	0.09	0.09	0.14	0.31	0.35	0.00	0.5	0.34
CHAINAGE	836.50	836.54	840.00	844.42	854.42	860.00	860.42	864.42	880.00	00:006	920.00	940.00	956.50



ROAD 5 LONGITUDINAL SECTION SCALE HORI 1:500 VERT 1:250

DRAFT FOR REVIEW

- . ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS STANDARDS.
- . COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000.
- LEVELS IN TERMS OF THE NZVD16 VERTICAL DATUM. ROADING LONGITUDINAL SECTIONS ARE BASED ON THE ROAD CENTRE LINE.
- REFER TO ROADING DRAWING C340 DRAWING FOR THE ROADING TYPICAL CROSS SECTION.
 REFER TO C300 SERIES DRAWINGS FOR THE



Α	DRA	AFT		WPJ	11/2024	
Rev	Description		Ву	Date		
		Ву	Date	,		
Surve	у	MAVEN	05/2024			
Desig	n	TCH	10/2024			
Drawn		TCH	10/2024			
Checked		DJM	11/2024			



ASHBOURNE **SOLAR FARM B** FOR MATAMATA DEVELOPMENTS

PROPOSED **ROADING** LONGSECTIONS

Project no.	289001		
Scale	AS SHOWN		
Cad file	C320-RD LS.DWG		
Drawing no.	C320-4	Rev	Α

NOTES: Valuate Cressing Carriageway 1.0 GENERAL 1:12 1.1 All works shall be constructed in accordance with the following standards and terms as applicable to site specific conditions. 1.2 No work shall be undertaken within the road reserve until Council has approved a Light = 4m traffic management plan. Large = 12m 1.3 The Contractor shall be responsible for the cost of repairs to any underground Utility Service damaged during construction. Any damage shall be rectified to the satisfaction of the Litility Owner. **ENTRANCE LONGSECTION** 2.0 LOCATION 2.1 Each entrance shall be located to provide a clear sight distance in both directions in 4.5m Driveway **Gate or Cattlestop** (Gate shall swing inwards to property) 2.2 Separation distances shall be as indicated in Figure 3.2 of the Development Manual 3.0 CULVERT 3.1 If an entrance crosses a Regional Council Board Drain or major watercourse, the contractor shall obtain certified waterway approval from WAIKATO REGIONAL COUNCIL. 3.2 If the entrance crosses a watertable or drain, a 300mm diameter minimum, Reinforced Concrete Rubber Ring Joint (R.C.R.R.J) Class X pipe shall be installed, unless otherwise approved by Council's Roading/Consent Engineer. 3.3 Any unsuitable bedding material including vegetation, topsoil and peat shall be removed and replaced in accordance with the pipe manufacturers specifications. 3.4 All cultiverts shall be laid straight at a constant grade and a minimum of 2.0m from the edge of seal or metal. Socket ends shall always be uphill and the pipe shall be extended to a sufficient length, to ensure the resulting batter is not steeper than 1:3. 2200 12 See 4.1 A minimum subgrade CBR of 5 is required before placement of sub-base material. If this CBR cannot be achieved, Council's Roading/Consent Engineer will advise how to proceed. This may involve an additional depth of pavement construction, or the installation of pecsynthetics. 4.2 Pit sand, brown rock or similar material shall be placed, trimmed and compacted to provide 100mm depth of subbase, if required. The subbase shall be placed from the edge of the carriageway to the gate or cattlestop. 5.0 BASECOURSE **Property Boundary** 5.1 Clean good quality WHAP 40 basecourse metal shall be placed, frimmed and compacted to provide 150mm depth of basecourse from the carriageway to the gate Intilative Fonce line 5.2 The basecourse material snall be trimmed to provide a crown at the centre of the entrance to ensure adequate surface drainage. The crossfall shall be 5% from the The entranceway shall be sealed in accordance with the Development Manual. Sealing shall be a 180/200 bitumen two cost grade 3 & 5 chip seal. Edge of agai/meth 17.25m

Large Vehicle Entrance for Rural Road

> 20 ha

PLAN

NOT TO SCALE



Large Vehicle Entrance -Rural Zone

MPDC DG307c : October 2011

A DRAFT AJP 11/2024

Rev Description By Date

By Date

Survey

Design

Drawn TCH 11/2024

Checked DJM 11/2024



Project
ASHBOURNE
SOLAR FARM B
FOR
MATAMATA
DEVELOPMENT

PROPOSED
ROADING STANDARD
DETAILS

Project no.	289001		
Scale	N.T.S		
Cad file	C380-RD DET.DWG		
Drawing no.	C380	Rev	Α

NOTES: Valuate Cressing Carriageway 1.0 GENERAL 1:12 1.1 All works shall be constructed in accordance with the following standards and terms as applicable to site specific conditions. 1.2 No work shall be undertaken within the road reserve until Council has approved a Light = 4m traffic management plan. Large = 12m 1.3 The Contractor shall be responsible for the cost of repairs to any underground Utility Service damaged during construction. Any damage shall be rectified to the satisfaction of the Litility Owner. **ENTRANCE LONGSECTION** 2.0 LOCATION 2.1 Each entrance shall be located to provide a clear sight distance in both directions in 4.5m Driveway **Gate or Cattlestop** (Gate shall swing inwards to property) 2.2 Separation distances shall be as indicated in Figure 3.2 of the Development Manual 3.0 CULVERT 3.1 If an entrance crosses a Regional Council Board Drain or major watercourse, the contractor shall obtain certified waterway approval from WAIKATO REGIONAL COUNCIL. 3.2 If the entrance crosses a watertable or drain, a 300mm diameter minimum, Reinforced Concrete Rubber Ring Joint (R.C.R.R.J) Class X pipe shall be installed, unless otherwise approved by Council's Roading/Consent Engineer. 3.3 Any unsuitable bedding material including vegetation, topsoil and peat shall be removed and replaced in accordance with the pipe manufacturers specifications. 3.4 All cultiverts shall be laid straight at a constant grade and a minimum of 2.0m from the edge of seal or metal. Socket ends shall always be uphill and the pipe shall be extended to a sufficient length, to ensure the resulting batter is not steeper than 1:3. 2200 12 See 4.1 A minimum subgrade CBR of 5 is required before placement of sub-base material. If this CBR cannot be achieved, Council's Roading/Consent Engineer will advise how to proceed. This may involve an additional depth of pavement construction, or the installation of pecsynthetics. 4.2 Pit sand, brown rock or similar material shall be placed, trimmed and compacted to provide 100mm depth of subbase, if required. The subbase shall be placed from the edge of the carriageway to the gate or cattlestop. 5.0 BASECOURSE **Property Boundary** 5.1 Clean good quality WHAP 40 basecourse metal shall be placed, frimmed and compacted to provide 150mm depth of basecourse from the carriageway to the gate Intilative Fonce line 5.2 The basecourse material snall be trimmed to provide a crown at the centre of the entrance to ensure adequate surface drainage. The crossfall shall be 5% from the The entranceway shall be sealed in accordance with the Development Manual. Sealing shall be a 180/200 bitumen two cost grade 3 & 5 chip seal. Edge of agai/meth 17.25m

Large Vehicle Entrance for Rural Road

> 20 ha

PLAN

NOT TO SCALE



Large Vehicle Entrance -Rural Zone

MPDC DG307c : October 2011

A DRAFT AJP 11/2024

Rev Description By Date

By Date

Survey

Design

Drawn TCH 11/2024

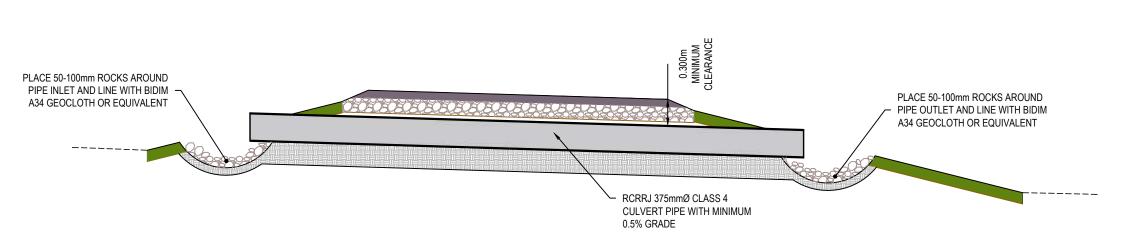
Checked DJM 11/2024



Project
ASHBOURNE
SOLAR FARM B
FOR
MATAMATA
DEVELOPMENT

PROPOSED
ROADING STANDARD
DETAILS

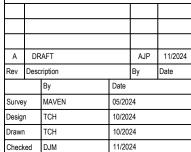
Project no.	289001		
Scale	N.T.S		
Cad file	C380-RD DET.DWG		
Drawing no.	C380	Rev	Α



STORMWATER CULVERT TYPICAL DETAIL

NOTES

- ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 2018 AND SUBSEQUENT UPDATES.
- ALL WORKS TO BE IN ACCORDANCE WITH WAIKATO REGIONAL COUNCIL EROSION AND SEDIMENT CONTROL GUIDELINES FOR SOIL DISTURBING ACTIVITIES 2009 AND SUBSEQUENT UPDATES.
- COORDINATES IN TERMS OF NZ GEODETIC DATUM MT EDEN 2000. LEVELS IN TERMS OF NZVD16 VERTICAL DATUM 1946.
- EXISTING SERVICES ARE FROM B4UDIG AND THEY ARE INDICATIVE ONLY. CONTRACTOR TO LOCATE THE EXISTING SERVICES PRIOR TO CONSTRUCTION.
- 5. REFER TO THE ROADING C300 SERIES DRAWING FOR THE PROPOSED DRAINAGE CULVERT LOCATIONS.



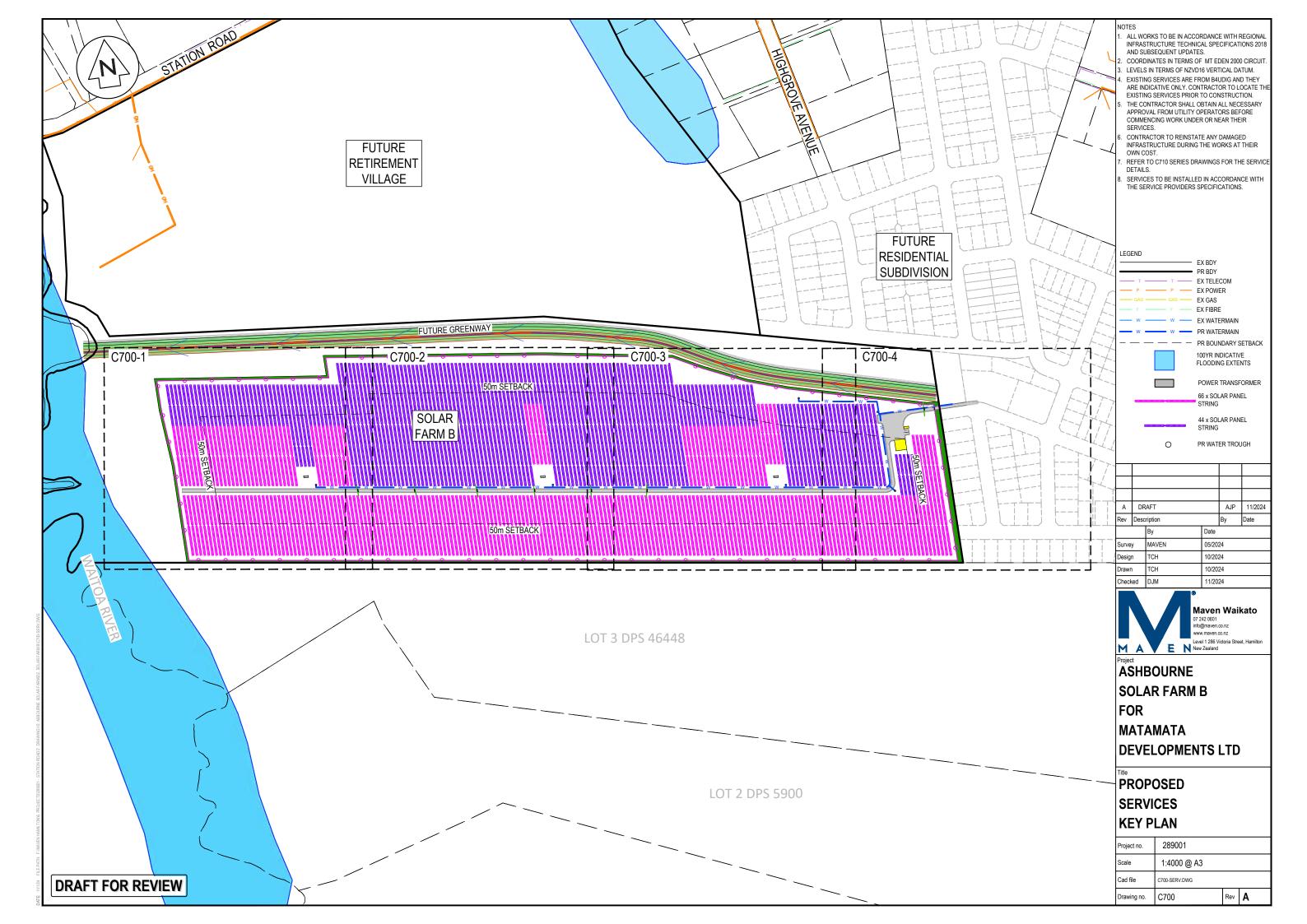


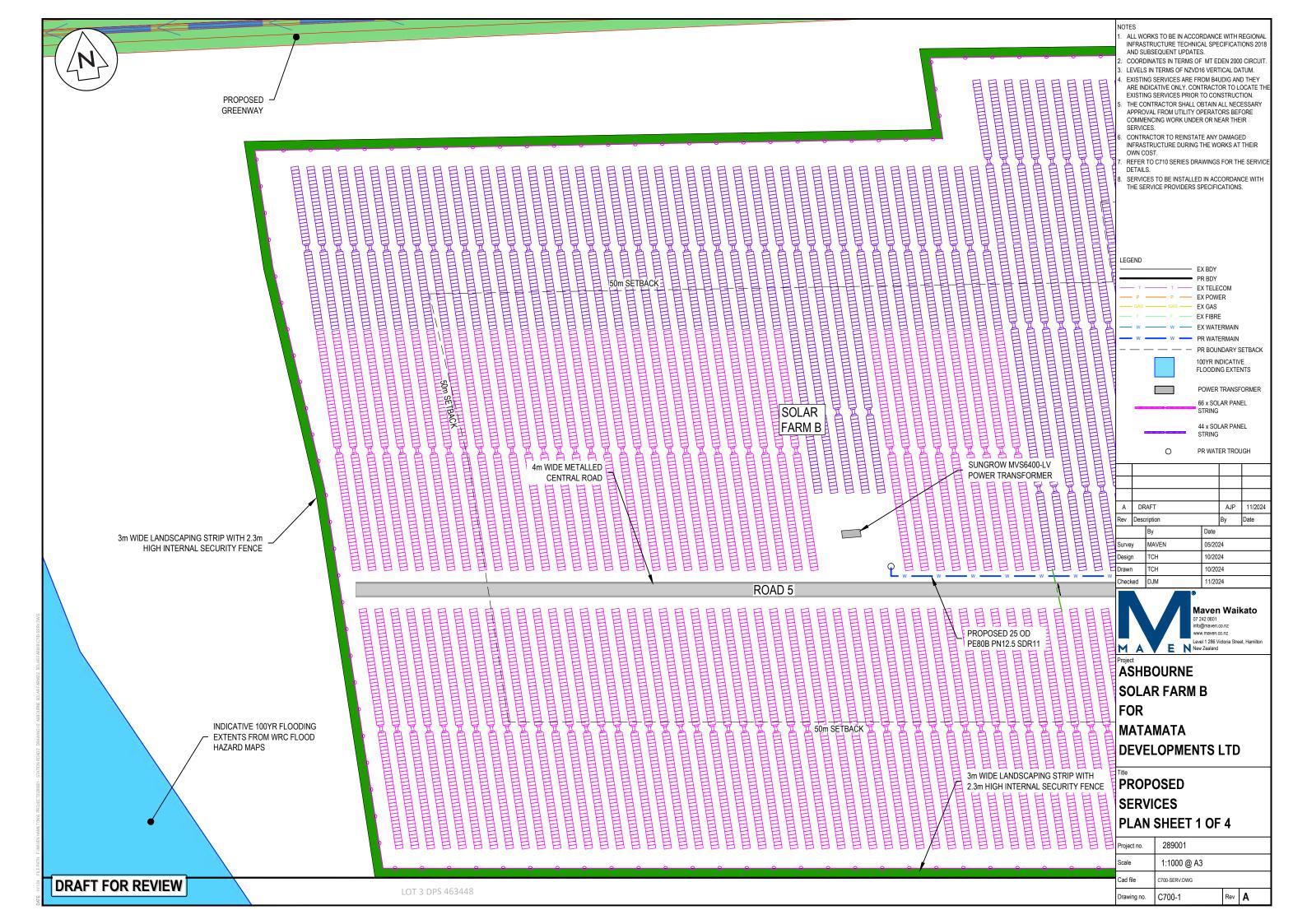
ASHBOURNE SOLAR FARM B FOR MATAMATA DEVELOPMENTS LTD

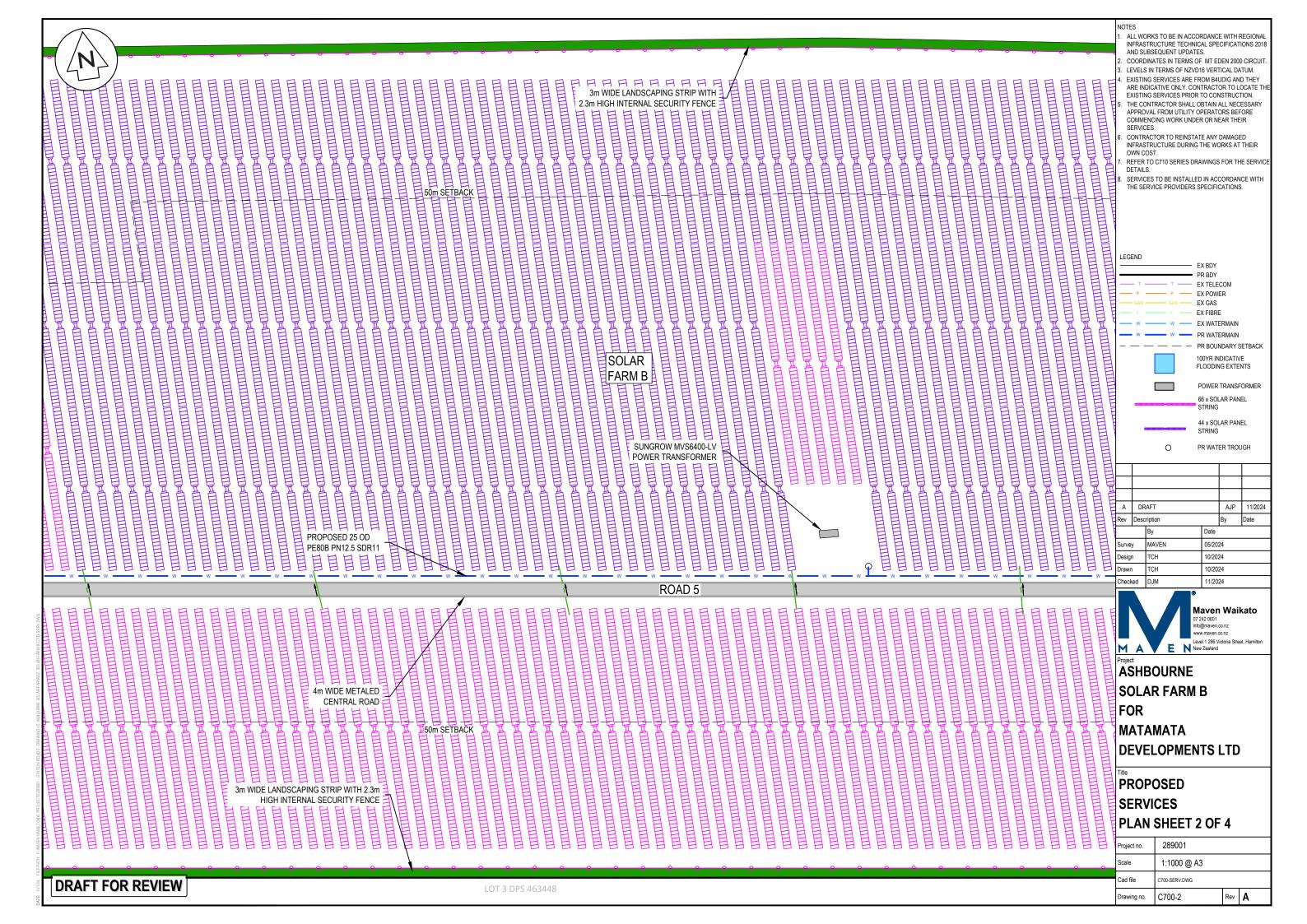
Title

PROPOSED STORMWATER DETAILS PLAN

Project no.	289001			
Scale	N.T.S			
Cad file	C480-SW DET.DWG			
Drawing no.	C420	Rev	Α	









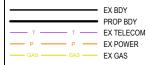




NOTES

- ALL WORKS TO BE IN ACCORDANCE WITH REGIONAL INFRASTRUCTURE TECHNICAL SPECIFICATIONS 201 AND SUBSEQUENT UPDATES.
- COORDINATES IN TERMS OF MT EDEN 2000 CIRCUIT
 LEVELS IN TERMS OF NZVD16 VERTICAL DATUM.
- EXISTING SERVICES ARE FROM B4UDIG AND THEY
 ARE INDICATIVE ONLY. CONTRACTOR TO LOCATE
 THE EXISTING SERVICES PRIOR TO CONSTRUCTION
- THE CONTRACTOR SHALL OBTAIN ALL NECESSARY APPROVAL FROM UTILITY OPERATORS BEFORE COMMENCING WORK UNDER OR NEAR THEIR SERVICES.
- 6. CONTRACTOR TO REINSTATE ANY DAMAGED INFRASTRUCTURE DURING THE WORKS AT THEIR OWN COST.
- 7. REFER TO C710 SERIES DRAWINGS FOR THE SERVICE DETAILS.
- 8. SERVICES TO BE INSTALLED IN ACCORDANCE WITH THE SERVICE PROVIDERS SPECIFICATIONS.





	Α	DR	AFT		WPJ	11/2024
	Rev	Desc	ription		Ву	Date
			Ву	Date		
	Survey		MAVEN	05/2024		
	Design Drawn		TH	11/2024		
			TH	11/2024		



Project

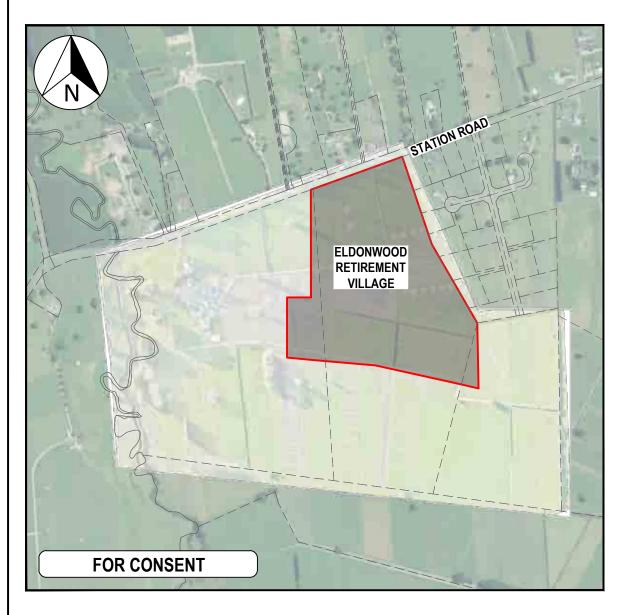
ASHBOURNE SOLAR FARM B FOR MATAMATA DEVELOPMENTS LTD

Title

PROPOSED SERVICES DETAIL PLAN

Project no.	289001				
Scale	AS SHOWN				
Cad file	C710-SERV DET.DWG				
Drawing no.	710	Rev	Α		





LOCALITY PLAN SCALE 1:10000 @A3

ASHBOURNE RETIREMENT VILLAGE, STATION ROAD, MATAMATA FOR UNITY DEVELOPMENT LTD

SHT.NO.	SHEET TITLE	REVISION				
C0000	Cover Sheet	В				
C0001	Index	А				
C0500	Pre Development Contour Plan	В				
C1100	Proposed Site Overview Plan	A				
	Earthworks Design Plans					
	C2200- C2400	В				
Roading Design Plans						
	C3000 - C3301	В				
Stormwater Design Plans						
	C4000 - C4907	В				
	Wastewater Design Plans					
	C5000A - C5804	В				
Water Supply Design Plans						
	C6000 - C6802 B					
	Combined Services Design Plans					
	C7000 - C7003	A				

PROJECT NUMBER: J00606

ISSUED DATE: 04/2025



DRAWING INDEX

SHT.NO.	SHEET TITLE	REVISION			
GENERAL					
C0000	Cover sheet	В			
C0001	Drawing Index	А			
C0500	Pre Development Contour Plan	В			
C1100	Proposed Site Overview Plan	А			
	EARTHWORKS				
C2200	Proposed Overview Contour Plan	В			
C2201	Proposed Contour Plan (1 of 4)	В			
C2202	Proposed Contour Plan (2 of 4)	В			
C2203	Proposed Contour Plan (3 of 4)	В			
C2204	Proposed Contour Plan (4 of 4)	В			
C2300	Proposed Erosion & Sediment Control Plan	В			
C2350	Proposed ESC Standard Details Plan (1 of 4)	В			
C2351	Proposed ESC Standard Details Plan (2 of 4)	В			
C2352	Proposed ESC Standard Details Plan (3 of 4)	В			
C2353	Proposed ESC Standard Details Plan (4 of 4)	В			
C2400	Proposed Cut / Fill Plan	В			
	ROADING				
C3000	Proposed Roading Layout Overview Plan	В			
C3001	Proposed Roading Layout Plan (1 of 3)	В			
C3002	Proposed Roading Layout Plan (2 of 3)	В			
C3003	Proposed Roading Layout Plan (3 of 3)	В			
C3004	Proposed Roading Long Section Plan (1 of 7)	В			
C3005	Proposed Roading Long Section Plan (2 of 7)	В			
C3006	Proposed Roading Long Section Plan (3 of 7)	В			
C3007	Proposed Roading Long Section Plan (4 of 7)	В			
C3008	Proposed Roading Long Section Plan (5 of 7)	В			
C3009	Proposed Roading Long Section Plan (6 of 7)	В			
C3010	Proposed Roading Long Section Plan (7 of 7)	В			
C3300	Proposed Roading Typical Cross Section (1 of 2)	В			
C3301	Proposed Roading Typical Cross Section (2 of 2)	В			

SHT.NO.	SHEET TITLE	REVISION
	STORMWATER	
C4000	Proposed Stormwater Overview Plan	В
C4001	Proposed Stormwater Layout Plan (1 of 4)	В
C4002	Proposed Stormwater Layout Plan (2 of 4)	В
C4003	Proposed Stormwater Layout Plan (3 of 4)	В
C4004	Proposed Stormwater Layout Plan (4 of 4)	В
C4050	Existing Stormwater Catchment Plan	В
C4051	Proposed 10 year Stormwater Catchment Plan	В
C4052	Proposed 100 year Stormwater Catchment Plan	В
C4600	Proposed Stormwater Pond 1 Cross Section Plan	В
C4601	Proposed Stormwater Pond 2 Cross Section Plan	В
C4650	Proposed Stormwater Swales Layout Plan	В
C4651	Proposed Stormwater Swales Cross Sections	В
C4700	Proposed Soakage Trench Cross Section Plan	В
C4701	Proposed Stormwater Pond Detail Plan	В
C4702	Proposed Stormwater Raingarden Detail Plan	В
C4703	Proposed Soakage Device Cross Section Plan	В
C4800	Proposed Stormwater Standard Detail	В
C4900	Proposed Overland Flow Path Layout Plan	A
C4901	Proposed Overland Flow Cross Section (1 of 3)	A
C4902	Proposed Overland Flow Cross Section (2 of 3)	A
C4903	Proposed Overland Flow Cross Section (3 of 3)	A
	WASTEWATER	
C5000A	Proposed Wastewater Overall Layout Plan Option A	В
C5001	Proposed Wastewater Plan (1 of 3)	В
C5002	Proposed Wastewater Plan (2 of 3)	В
C5003	Proposed Wastewater Plan (3 of 3)	В
C5200	Proposed WW Long Section Plan (1 of 9)	В
C5201	Proposed WW Long Section Plan (2 of 9)	В
C5202	Proposed WW Long Section Plan (3 of 9)	В
C5203	Proposed WW Long Section Plan (4 of 9)	В
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SHT.NO.	SHEET TITLE	REVISION				
C5204	Proposed WW Long Section Plan (5 of 9)	В				
C5205	Proposed WW Long Section Plan (6 of 9)	В				
C5206	Proposed WW Long Section Plan (7 of 9)	В				
C5207	Proposed WW Long Section Plan (8 of 9)	В				
C5208	Proposed WW Long Section Plan (9 of 9)	В				
C5800	Proposed WW Standard Detail Plan (1 of 5)	В				
C5801	Proposed WW Standard Detail Plan (2 of 5)	В				
C5802	Proposed WW Standard Detail Plan (3 of 5)	В				
C5803	Proposed WW Standard Detail Plan (4 of 5)	В				
C5804	Proposed WW Standard Detail Plan (5 of 5)	В				
WATER SUPPLY						
C6000	Proposed Water Supply Overview Plan	В				
C6001	Proposed Water Supply Plan (1 of 4)	В				
C6002	Proposed Water Supply Plan (2 of 4)	В				
C6003	Proposed Water Supply Plan (3 of 4)	В				
C6004	Proposed Water Supply Plan (4 of 4)	В				
C6500	Water Supply Connection Details	В				
C6800	Water Supply Standard Details (1 of 2)	В				
C6801	Water Supply Standard Details (2 of 2)	В				
C6802	Water Supply Anchor Block Details	В				
	COMBINED SERVICES					
C7000	Proposed Combined Services Overview Plan	А				
C7001	Proposed Combined Services Plan (1 of 3)	А				
C7002	Proposed Combined Services Plan (2 of 3)	A				
C7003	Proposed Combined Services Plan (3 of 3)	A				

PROJECT NUMBER: J00606

ISSUED DATE: 04/2025





