

### Memo – Delmore Overland Flow Paths

To: Vineway Ltd
From: Derek Kong
Cc: James Kitchen,
Date: 28/06/2025

Subject: - Delmore Stage 1 and 2, Upper Orewa

#### 1. BACKGROUND & PURPOSE

This memo accompanies the Fast-Track approval application for the proposed road corridor works. It summarises the detailed Overland Flow Path ("**OLFP**") engineering analysis undertaken to confirm that all flood hazards are safely managed within the road reserve and that the design fully complies with:

- Auckland Transport's TDM Road Drainage Guide (v1.2, Table 3)
- Auckland Council's Healthy Waters Stormwater Code of Practice (SWCoP)

This detailed analysis is found on the following drawings –

Overland flowpath Plans - 3725-1-4600 to 4607, 3725-2AB-4600 to 4605 and 3725-2CDE-4600 to 4605

Outlet sizing typical plan and dimensions - 3725-1-4650, 372-2AB-4650 & 372-2CDE-4650

#### 2. RAINFALL & HYDROLOGICAL MODELING

Design Storm: 100-year ARI + 3.8 °C climate change allowance (32.7% increase).

TP108 Depth: 309.2 mm total for the 100-yr event.

Method: Rational method, 10 time of concentration

**Losses:** Manning's n = 0.015 (road corridor overland flow).

Bypass Assumption: All flows considered as surface bypass; no pipe capacity has been allowed for.

#### 3. DRAWING REFERENCES

**Stage 1:** Sheets 3725-1-4600 to 3725-1-4607, & 3725-1-4606

**Stage 2:** Sheets 3725-2AB-4600 to 3725-2AB-4605, 3725-2CDE-4600 to 3725-2CDE-4605, & 3725-2AB-4606, 3725-2CDE-4606

#### **4. RESULTS SUMMARY**

Location	n Peak D×V (m/s²	) TDM Category (Table 3, Refer Figure 1 below)	Compliance Status	Notes
Stage 1	≤ 0.4	"Obvious Danger"	Complies	All pram crossings are within this category.
Stage 1	≤ 0.6	"No Obvious Danger"	Complies	No pram crossings adjacent to OLFP's where DxV is above this threshold.
Stage 2	≤ 0.4	"Obvious Danger"	Complies	Entire foot path network below pedestrian hazard limit.
Stage 2	≤ 0.6	"No Obvious Danger"	Complies	No pram crossings adjacent to OLFP's where DxV is above this threhold

Situation	Requirement				
Where floor levels of adjacent buildings are above road level	Total flow contained within road reserve.  Freeboard from peak flow level to habitable floors in accord with Buildin Code and unitary plan.				
Where floor levels of adjacent buildings are less than 350mm above the top of the kerb, and the fall on the footpath towards the kerb is	Greater than 100mm: Water depth must be limited to 50mm above top o kerb.  Less than 100mm: Water depth must be limited to top of kerb in conjunction with a footpath profile that prevents flow from the roadway entering onto the adjacent property.  In these cases, compliance with Building Code and unitary plan may require separate approvals.				
Where no kerb is provided	Above depths must be measured from the channel lip level plus 100mm.				
Pedestrian safety <sup>1</sup>	No obvious danger: $d_g \times V_{ave} \le 0.6 \text{m}^3/\text{s}$ . Obvious danger: $d_g \times V_{ave} \le 0.4 \text{m}^3/\text{s}$ .				
Vehicle safety	Maximum height of energy line 300mm above roadway surface for areas subject to transverse flow. The exception is specific floodway design and additional vehicle warning and protection, where $_{\rm co}$ x $_{\rm yout}$ s $0.3{\rm m}^2/{\rm s}.$ On-street parking is not to be permitted where overland flow exceeds $0.3{\rm m}^2/{\rm s}.$				
1 Obvious danger is interpreted as a marked crossings and corners of inte	reas where pedestrians are directed to, or most likely to cross water paths, e.g. rsections.				
d <sub>o</sub> = flow depth in the channel adjace	ent to the kerb, i.e. at the invert (m).				

Figure 1 - Table 3 from Auckland Transports Road Drainage chapter, Transport Design Manual

**Containment:** All OLFPs remain within the top-of-kerb extent under the 1 % AEP storm.

**Pedestrian Safety:** All depths  $\times$  velocities are below 0.4 m<sup>2</sup>/s at pram crossing locations. (Table 3 threshold for "low hazard").

**Vehicle Safety:** Where D×V is between 0.4 and 0.6  $\text{m}^2/\text{s}$ , depths remain  $\leq$  0.25 m and are classified as "No Obvious Danger." No pram crossings are located within these zones, so pedestrians are not directed to these areas to direct them away from the danger.

**Erosion Protection:** Discharge points to adjacent streams are detailed with rip-rap aprons and grade-control weirs to prevent scour, down to the stream edge. They will follow the natural contour as far as practical.

Drawing 3275-1-4650 and 3725-2-4650 have typical OLFP channel dimensions and rip rap sizes. Note this is preliminary and final design will be confirmed during Engineering Plan Approval stage. This is consistent with the standard approach taken in respect of this matter.

#### 5. COMPLIANCE WITH AT TDM & SWCoP

**Depth × Velocity Criteria:** All pedestrian and vehicle 'hazard zones' are classified as such where they meet or exceed the TDM thresholds ( $0.4 \text{ m}^2/\text{s}$  and  $0.6 \text{ m}^2/\text{s}$ ). The 'hazard zones' within the development are located away from pram crossings, vehicle parking areas or dwellings.

Freeboard & Flow Widths: Overland flow easements and all dwelling freeboard allowances exceed SWCoP minimums ( $\geq$  300 mm freeboard above 1% AEP level;  $\geq$  1 m flow path width).

**Stormwater Conveyance:** Surface bypass design aligns with SWCoP Section 6.5.3, ensuring swale slopes  $\leq 1$ : 20 and grass-lined channels are provided where required.

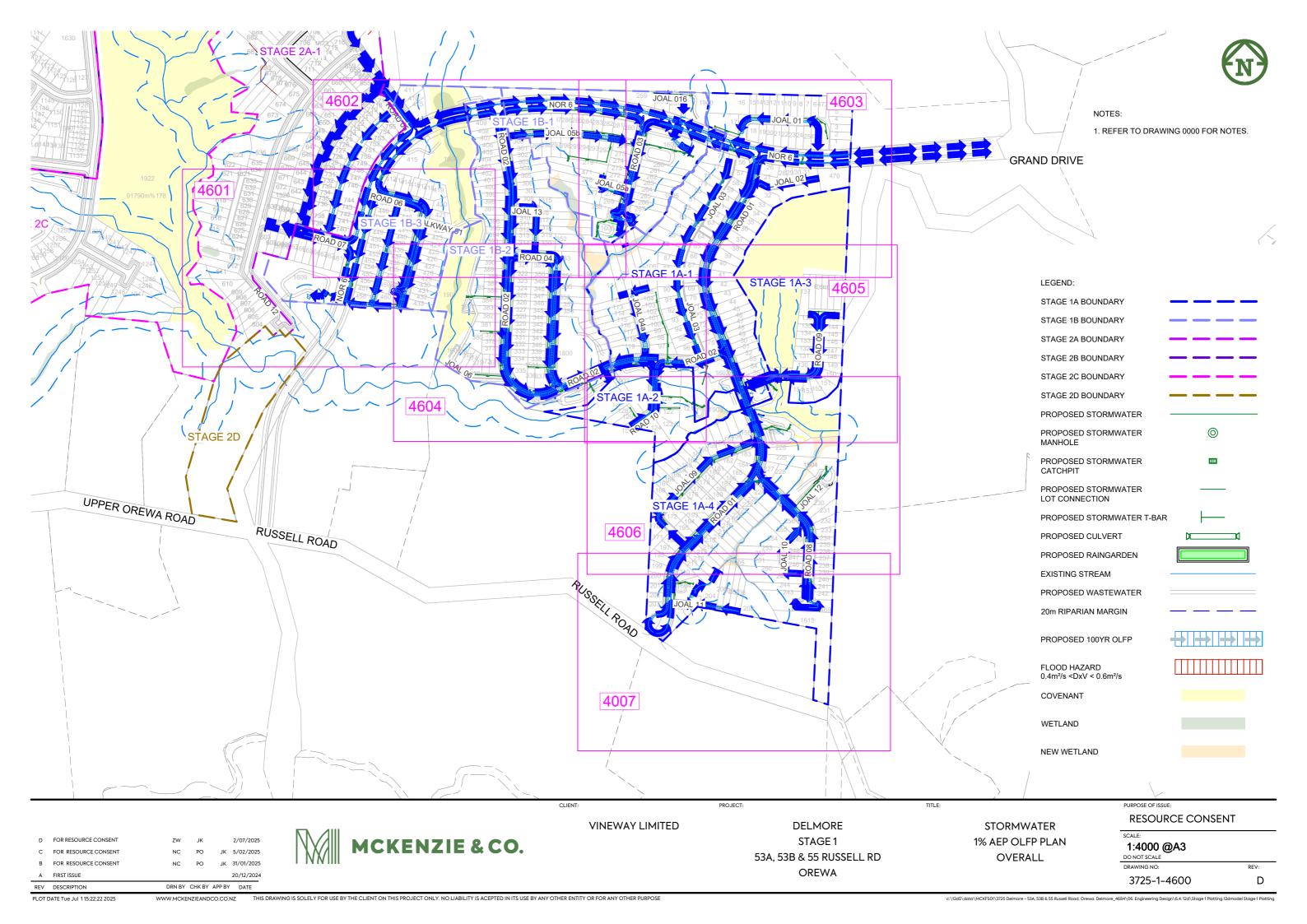
**Maintenance Access:** All flow paths include vehicle access points and inspection chambers per SWCoP Guidelines.

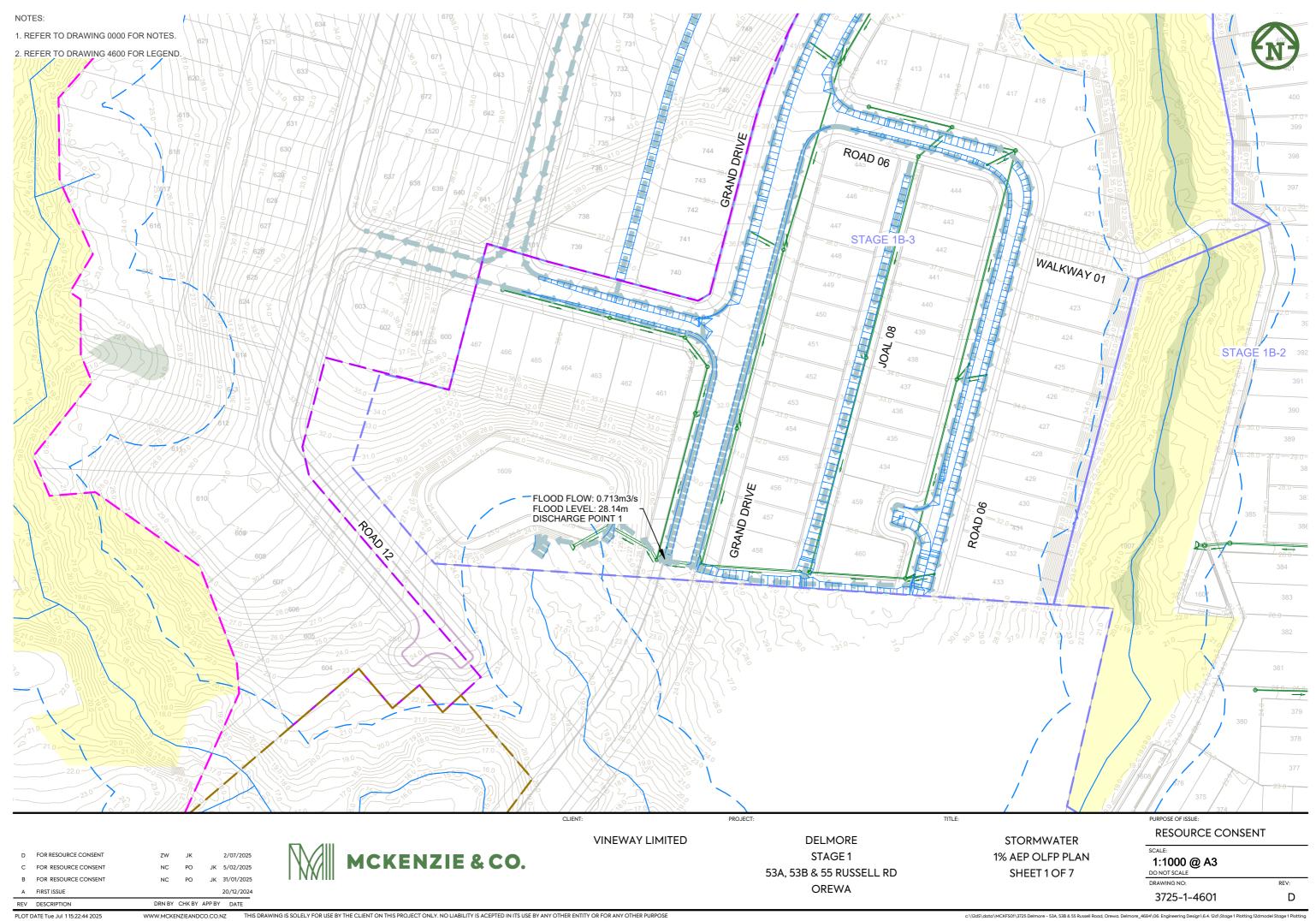
#### 6. CONCLUSIONS

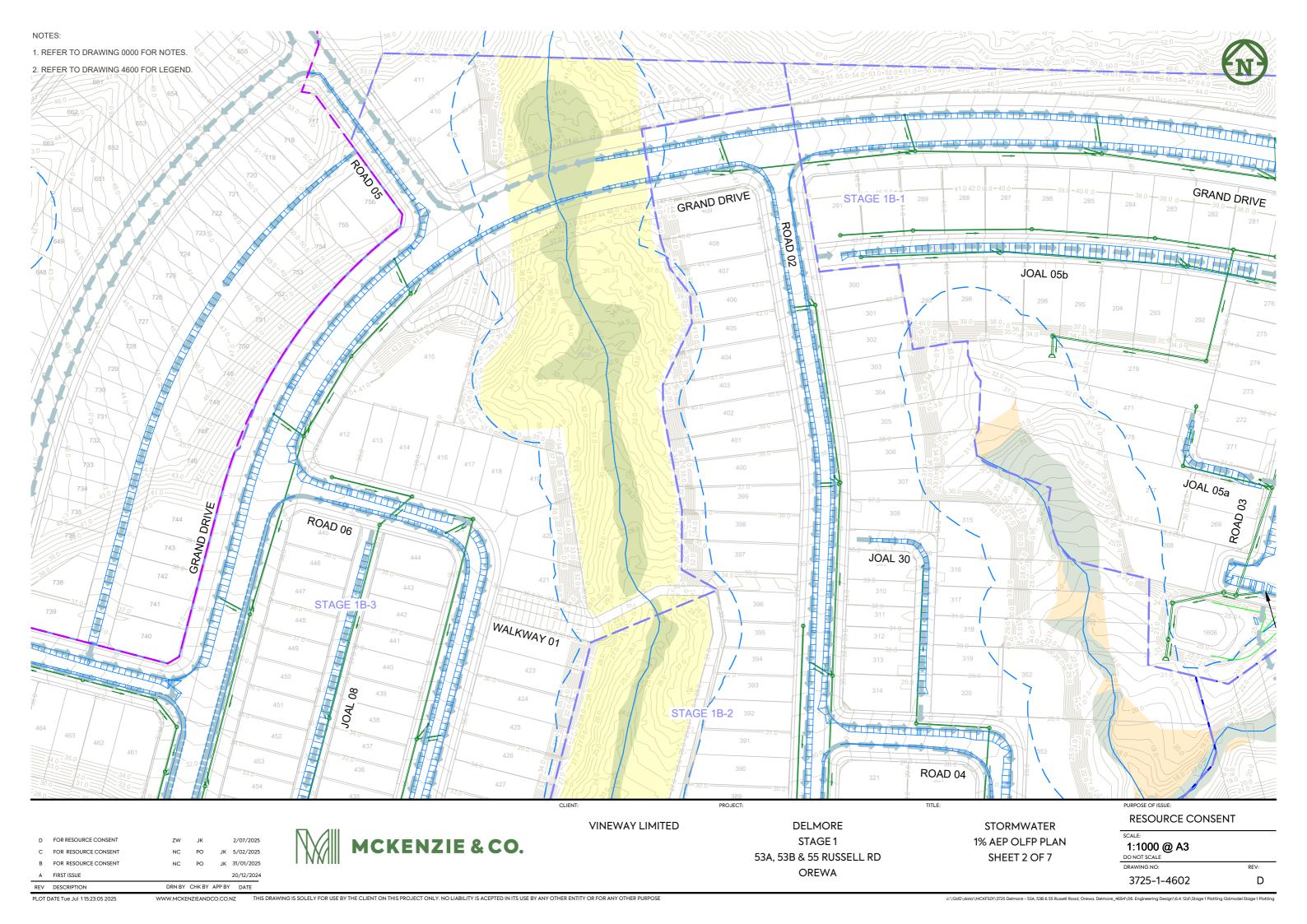
The OLFP design contains all floodwater within the road corridor or directs minor, controlled spillovers to protected discharge points. The relatively small catchments, and moderate steepness of the roads limits the extents of the OLFP's to ensure that they do not pose a hazard risk to people, infrastructure or property

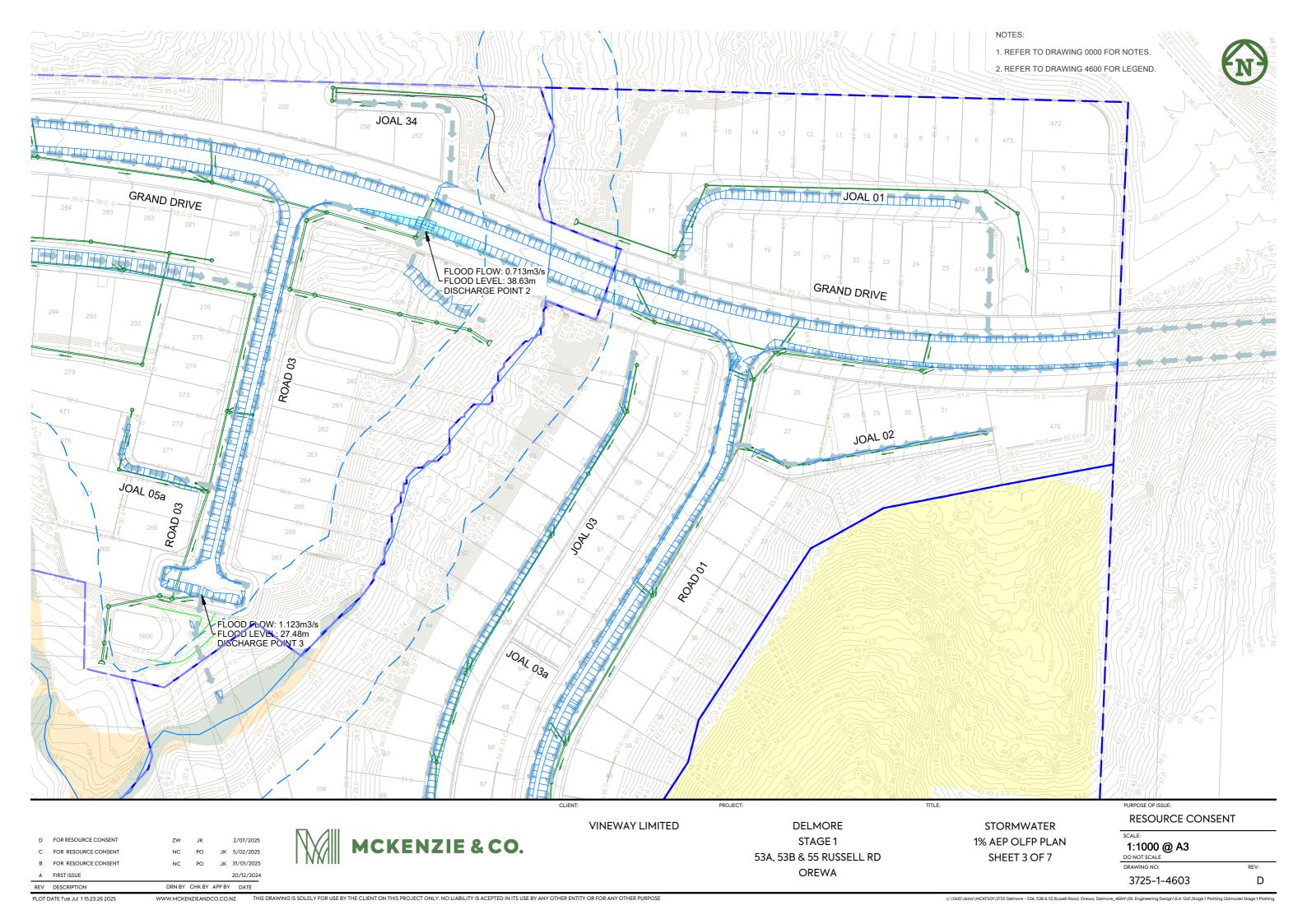
The design satisfies all requirements of Auckland Transport's TDM Road Drainage Guide and Healthy Waters' SWCoP. As such, all flood hazards will be safely managed within the road reserve in accordance with all relevant standards. OLFPs and flooding are therefore not considered to have an adverse impact.

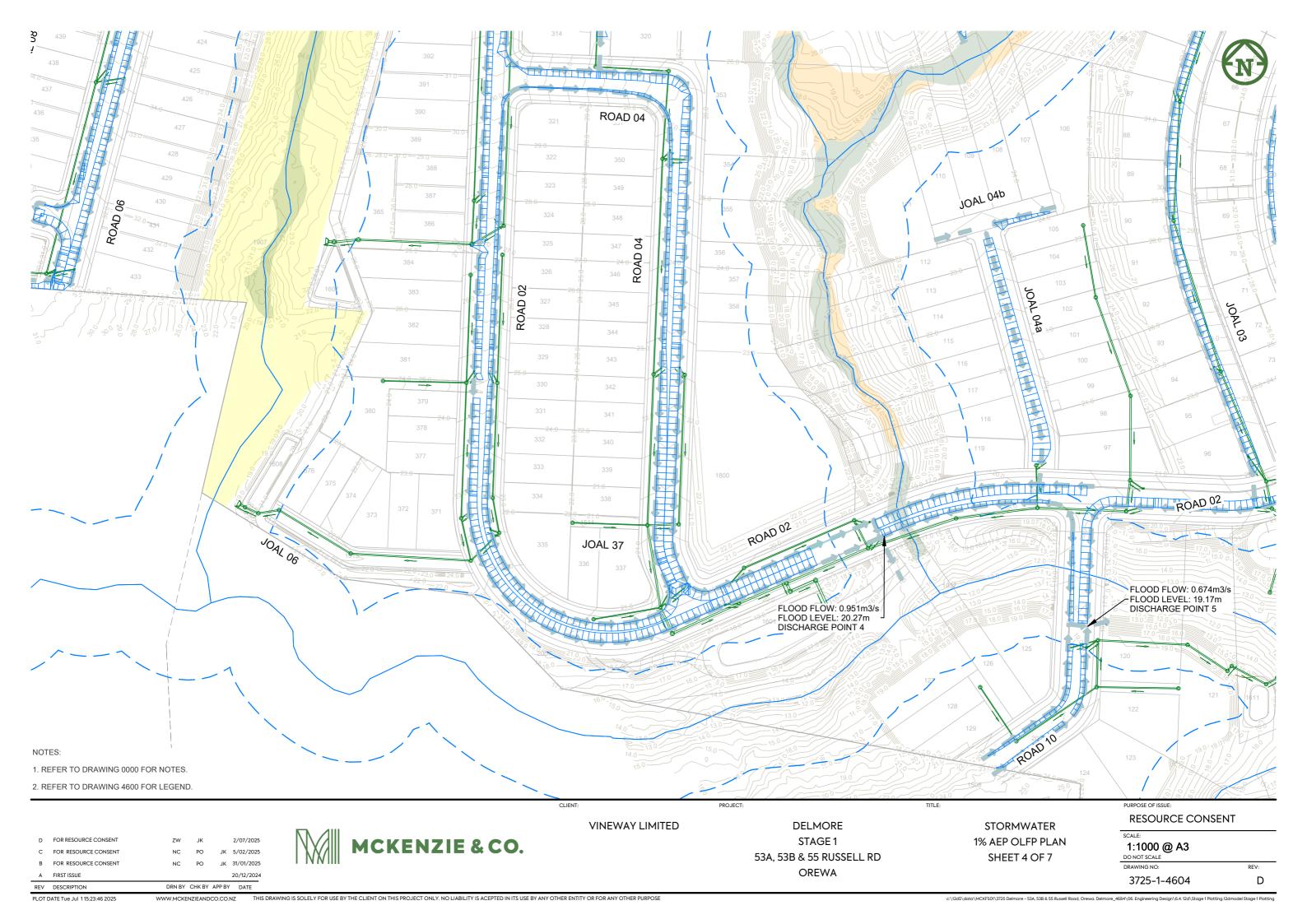
# Appendix A – Drawings

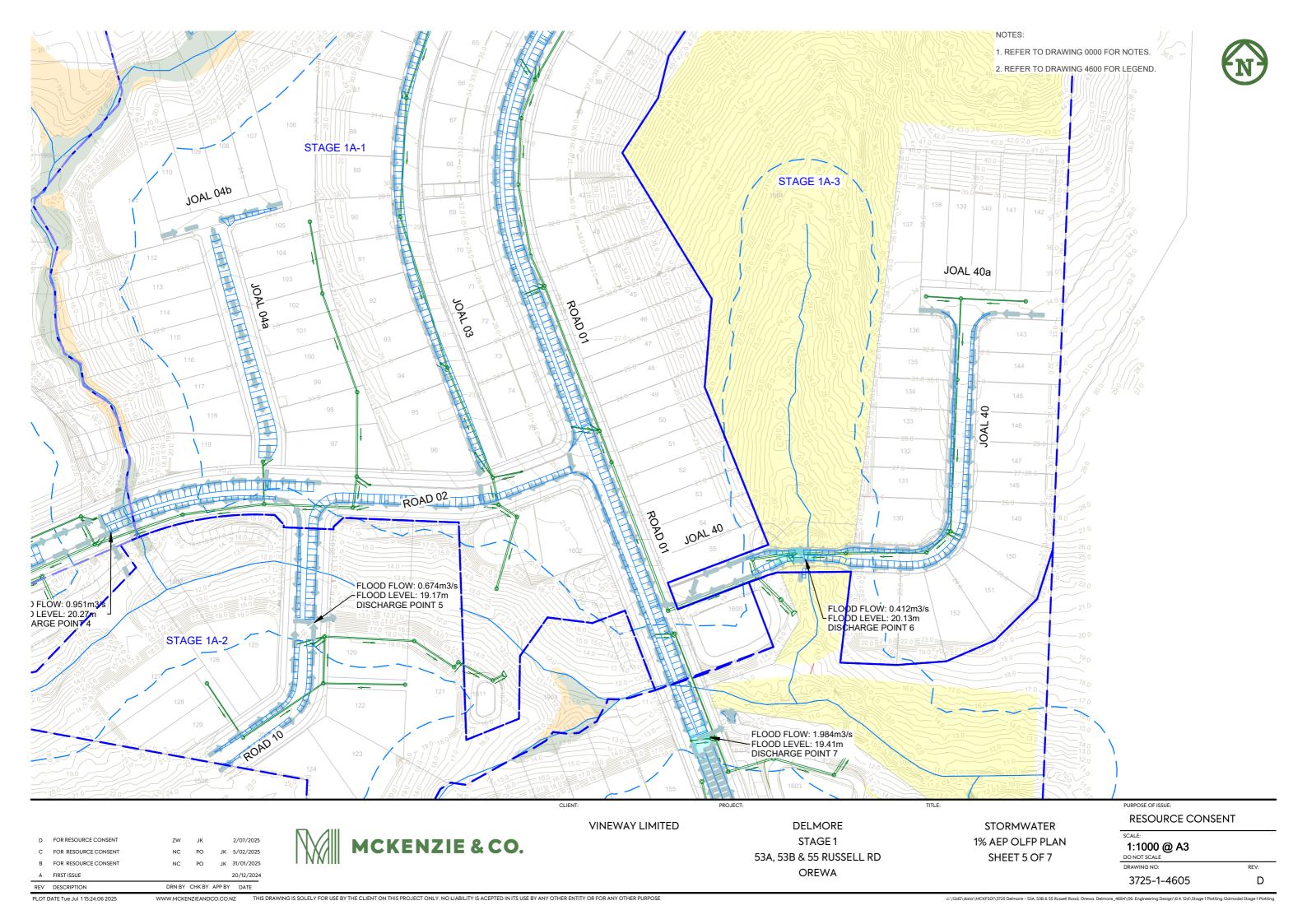


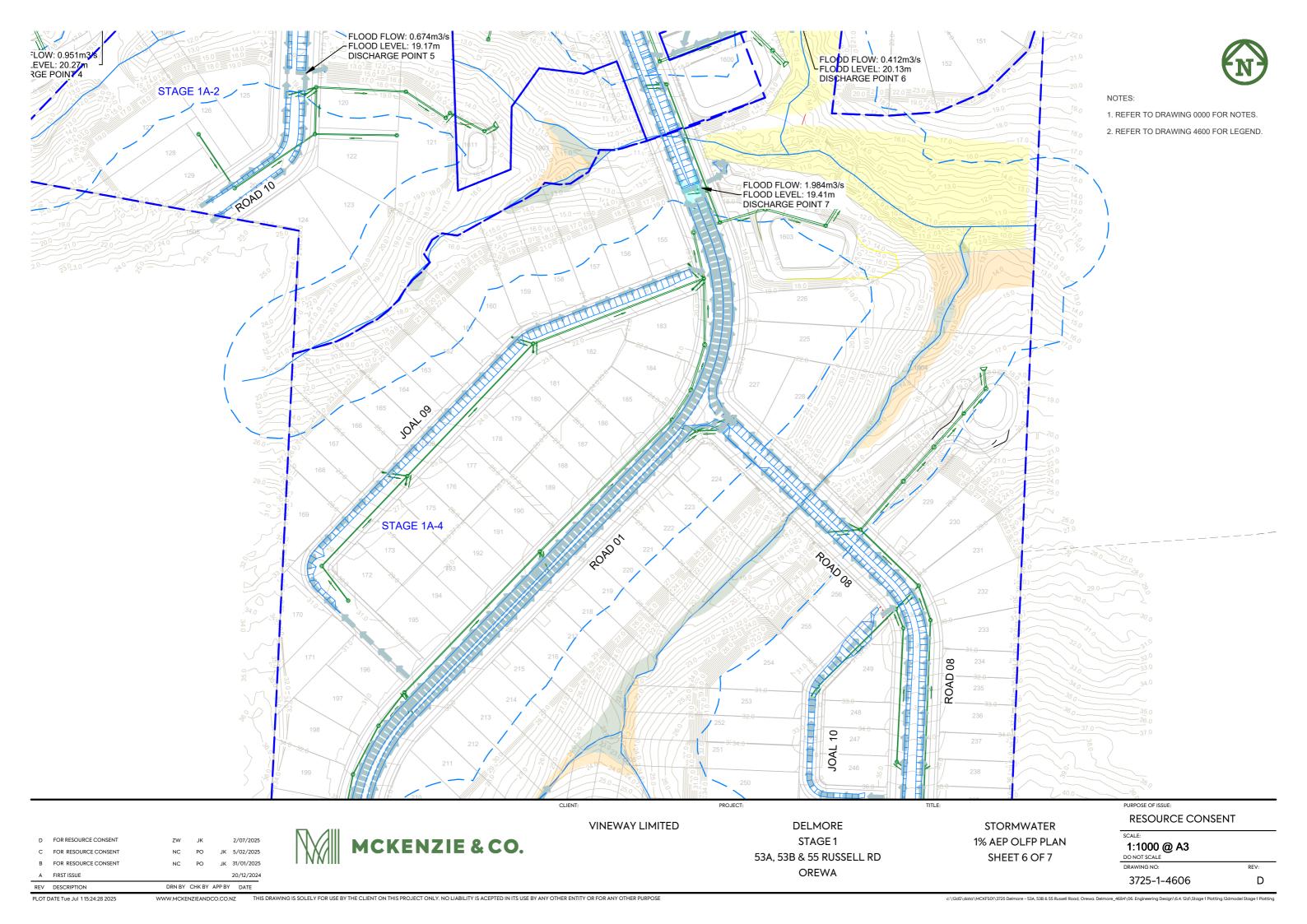


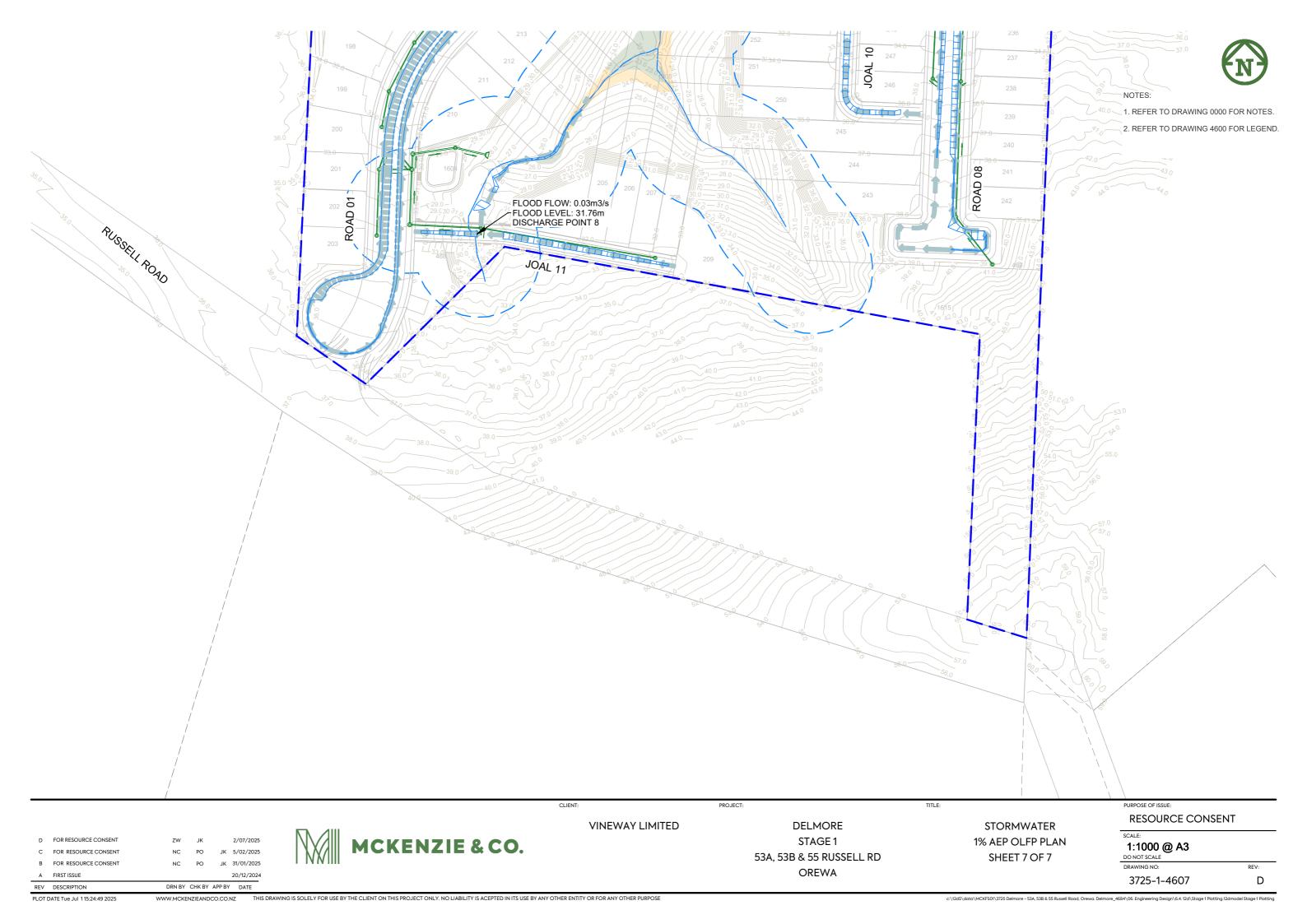


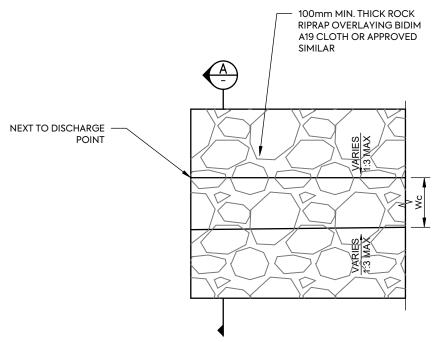




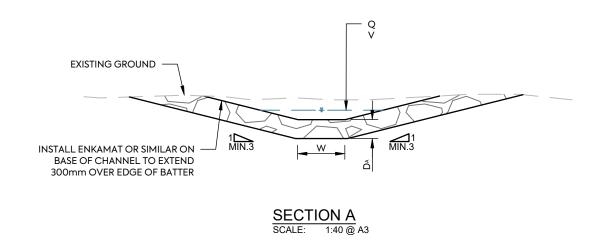








TYPICAL DISCHARGE CHANNEL DETAIL - PLAN SCALE: 1:40 @ A3



## **DISCHARGE POINT DIMENSIONS**

DISCHARGE	FLOOD FLOW (m3/s)	FLOOD VELOCITY (m/s)	CHANNEL BED WIDTH Wb	GRADE (SLOPE) '%'	CHANNEL SIDE SLOPE	RIPRAP DIAMETER 'd^s' (m)	RIPRAP THICKNESS 'D^A' (m)	
DISCHARGE POINT 1	0.713	2.99	0.4	12	1 IN 3	0.20	0.40	
DISCHARGE POINT 2	1.013	4.784	0.5	34	1 IN 3	0.43	0.86	
DISCHARGE POINT 3	1.123	4.256	0.5	23	1 IN 3	0.36	0.72	
DISCHARGE POINT 4	0.951	OVER TOP AT CULVERT 5						
DISCHARGE POINT 5	0.674	OVER TOP AT CULVERT 4						
DISCHARGE POINT 6	OVER TOP AT CULVERT 2							
DISCHARGE POINT 7	OVER TOP AT CULVERT 3							
DISCHARGE POINT 8	0.03	OVER TOP AT CULVERT 12						

Refer to Engineering Calculations (TR2013/018)

VINEWAY LIMITED

DELMORE STAGE 1 53A, 53B & 55 RUSSELL ROAD

STORMWATER DISCHARGE CHANNEL **DETAIL PLAN** 

FOR RESOURCE CONSENT AS SHOWN DO NOT SCALE DRAWING NO: 3725-1-4608

OREWA

PROJECT:

