

To

Healthy Waters

From

Woods

Jasmin Moll – 3 Waters Engineer

Reviewed

Bidara Pathirage - Senior Associate Engineer - 3 Waters

W-REF: P24-646 Drury Centre – Stage 2 14 July 2025

Memorandum - Overland flow path assessment

Introduction

An overland flow path assessment has been undertaken by Woods on behalf of Kiwi Property to support the Fast-Track application for Stage 2 of the Drury Centre Precinct development and in response to a Section 67 request from Healthy Waters.

The Stage 2 development site covers approximately 24.27ha and will include the development of commercial, retail, accommodation, and community activities, along with associated car parking.

The memo assesses the adequacy of the proposed road design to convey overland flows during a 100-year ARI storm event, incorporating a 3.8°C climate change allowance. The assessment has been carried out in accordance with Chapter 4 of the Auckland Council Code of Practice for Land Development and Subdivision (Version 4, July 2025), and considers vehicular and pedestrian safety requirements as outlined in the Auckland Transport Technical Design Manual (AT TDM).

2. Methodology

2.1. Peak Flow calculation

Woods have undertaken an overland flow path assessment for the proposed roads within Stage 2 using TP108 methodology. The purpose of the assessment is to evaluate the capacity of the roads to safely convey overland flows during a 100-year ARI storm event, incorporating a 3.8°C climate change uplift.

The key design parameters and processes used in the assessment are summarised below:

- Catchment delineation: Catchments were delineated using GIS tool, based on a combination of proposed design surface and existing topography. The delineated catchments, overland flow path, and cross-section locations are illustrated in Figure 1.
- Design storm event: A 100-year ARI storm event with a rainfall depth of 250mm, inclusive of 3.8°C
 climate change allowance, was adopted
- Impervious coverage assumptions:
 - o Road 90%
 - Lots 100%
 - o Open space 10%

- Areas outside Stage 2: imperviousness based on AUP zoning
- Soil Type and Curve Number: Group C soil was assumed, with a curve numbers (CN) of 74 for pervious surfaces and 98 for impervious surfaces.
- Time of concentration (Tc): Estimated using the equal area-method
- Stormwater network blockage assumption: In line with Auckland Council's Stormwater Code of Practice (SWCoP) (Version 4, July 2025), a 50% blockage was assumed for piped DN > 600, and 100% blockage for pipes with DN ≤ 600.

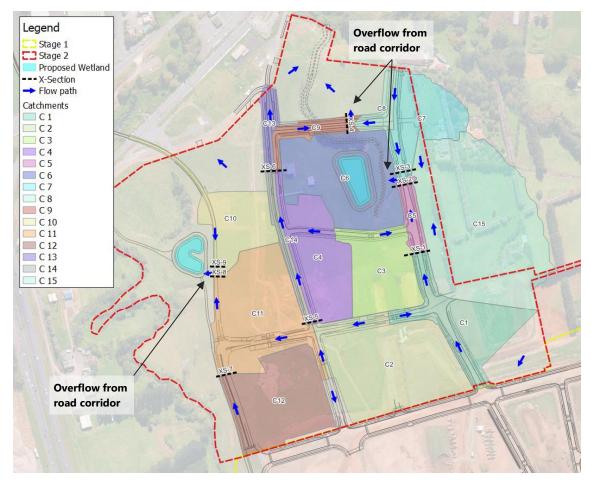


Figure 1: Catchments, overland flow path and cross-section location

2.2. Hydraulic assessment

A road capacity assessment was carried out using FHWA Hydraulic Toolbox (Version 5.3) at critical locations and speed tables along the proposed roads within Stage 2 of the Drury Centre development. The results were evaluated against the performance criteria outlined in Section 2.3 of Road Drainage in the Auckland Trasport Technical Design Manual) AT TDM), as shown in Figure 2.

TABLE 3 MAJOR EVENT - ROADWAY FLOW LIMITATIONS

Situation	Requirement
Where floor levels of adjacent buildings are above road level	Total flow contained within road reserve. Freeboard from peak flow level to habitable floors in accord with Building Code and unitary plan.
Where floor levels of adjacent buildings are less than 350mm above the top of the kerb, and the fall on the footpath towards the kerb is	Greater than 100mm: Water depth must be limited to 50mm above top of kerb. Less than 100mm: Water depth must be limited to top of kerb in conjunction with a footpath profile that prevents flow from the roadway entering onto the adjacent property. In these cases, compliance with Building Code and unitary plan may require separate approvals.
Where no kerb is provided	Above depths must be measured from the channel lip level plus 100mm.
Pedestrian safety ¹	No obvious danger: $d_g \times V_{ave} \le 0.6 \text{m}^2/\text{s}$. Obvious danger: $d_g \times V_{ave} \le 0.4 \text{m}^2/\text{s}$.
Vehicle safety	Maximum height of energy line300mm above roadway surface for areas subject to transverse flow. The exception is specific floodway design and additional vehicle warning and protection, where dg x vave ≤ 0.3m²/s. On-street parking is not to be permitted where overland flow exceeds 0.3m²/s.

¹ Obvious danger is interpreted as areas where pedestrians are directed to, or most likely to cross water paths, e.g. marked crossings and corners of intersections.

Source: Adapted from DNRW (2007a)

Figure 2: AT TDM criteria

A Mannings 'n' value of 0.02 was adopted to represent the surface roughness of the road as per SWCoP (Version 4, July 2025).

To account for the proposed pipe network, the overland flows within each catchment were adjusted by subtracting the corresponding pipe capacity, assuming a 50% blockage for piped DN > 600, as per SWCoP and described in Section 2.1 above. The proposed stormwater network layout is shown in Figure 3.

Drawings DWG No.: P24-447-01-3000-DR to P24-447-01-3004-DR and P24-447-01-3500-DR to P24-447-01-3518-DR provides details of the proposed stormwater network. Appendix C contains the drawings showing the proposed stormwater network.

 d_a = flow depth in the channel adjacent to the kerb, i.e. at the invert (m).

V_{ave} = average velocity of the flow (m/s).

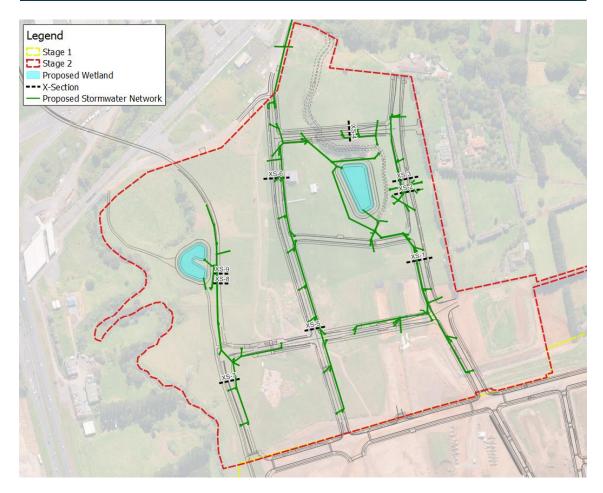


Figure 3: Proposed stormwater network

The results of the assessment are summarised in the following section.

3. Results and Discussion

For each cross section, the flow depth and average velocity were determined as well as the depth x velocity values. Table 1 summarises the results.

Table 1: OLFP assessment summary

X-Section	Catchment Area	Peak flow rate	Pipe diameter	Blockage factor	Pipe capacity with allowance for blockage	Final overland flow	Flow depth	Average velocity	Criteria (dxv)	Max flow depth x Average velocity (dxv)
	ha	m³/s	mm	%	m³/s	m³/s	m	m/s	m²/s	m²/s
XS-1	1.365	0.556	825	50%	1.075	0.000	0.000	0.000	0.3	0.000
XS-2	2.842	1.160	375	100%	0.000	1.160	0.128	1.863	0.3	0.238
XS-3	1.079	0.432	375	100%	0.000	0.432	0.147	0.637	0.3	0.094
XS-4	4.279	1.546	225	100%	0.000	1.546	0.169	1.632	0.3	0.276
XS-5	2.526	0.978	375	100%	0.000	0.978	0.157	0.896	0.3	0.141
XS-6 (Speed Table)	4.234	1.566	450	100%	0.000	1.566	0.093	1.206	0.3	0.112
XS-7	1.758	0.720	300	100%	0.000	0.720	0.188	0.634	0.3	0.119
XS-8	3.847	1.574	750	50%	0.482	1.091	0.232	1.177	0.3	0.273
XS-9	0.951	0.389	750	50%	0.650	0.000	0.000	0.000	0.3	0.000

The assessment confirms that potential overland flows originating from Stage 2 catchments are generally contained within the proposed road reserve. An exception occurs at the location of cross-section 8, where flow is not fully contained within the road reserve due to a narrowing of the road near the off-ramp to SH1. However, these flows are intended to discharge towards the Hingaia Stream and therefore not considered a problem.

The maximum allowable product of flow depth and average velocity ($d_g \times V_{ave}$) is 0.3 m²/s for parked vehicles and 0.4m²/s for pedestrian safety (threshold for obvious danger). The assessment results, summarised in Table 1, shows that all analysed cross-sections are below the minimum threshold. The proposed road design complies with the relevant criteria for vehicular and pedestrian safety outlined in AT TDM and SWCoP.

In accordance with SWCoP Section 4.3.5.6, it is expected that overland flows will remain unobstructed. Where flows exit the road corridor, appropriate measures should be implemented to minimise erosion risk and ensure safe conveyance.

Detailed runoff calculations and water level profiles for all cross sections are provided in Appendix A and Appendix B.

4. Conclusion

An overland flow path assessment has been undertaken for the proposed Stage 2 development of the Drury Centre Precinct. The assessment estimated the peak flow at critical locations along the proposed road network for a 100-year ARI storm event, incorporating a 3.8°C climate change uplift and accounting for primary stormwater network blockage in accordance with SWCoP.

Flood depth and velocities were calculated using Hydraulic Toolbox and assessed against the criteria outlined in Table 3 of the Road Drainage chapter in the AT TDM.

The results confirm that all assessed cross-section locations comply with the relevant requirements specified in the AT TDM.

Attachment A Calculations

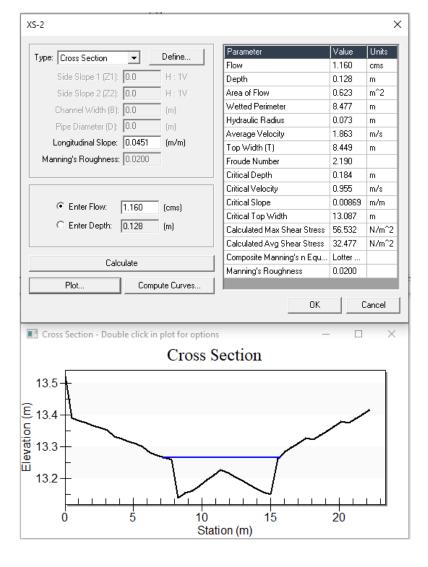
		Area	Catchment Length	Catchment Slope	Impervious Area	Pervious Area	Initial Abstraction	24hr Rainfall depth	Channelisation factor	Curve Number	Time of concentration	Lag Time	Soil storage parameter	Runoff depth	Runoff index	Column reference
Т	P108 notation	Α	L	Sc	Α	Α	IA	P ₂₄	С	CN	Тс	Lt	S	Q ₂₄	c*	from TP108 table
	Units	ha	km	m/m	%	%	mm	mm			Hrs	Hrs	mm	mm		table
XS-1	IMP	1.303	0.22	0.011	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
X3-1	PERV	0.062	0.22	0.011	0%	100%	5	250	0.6	74.00	0.17	0.11	89.24	179.12	0.57	54.00
XS-2	IMP	2.736	0.33	0.028	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
λ3-2	PERV	0.106	0.33	0.028	0%	100%	5	250	0.6	74.00	0.17	0.11	89.24	179.12	0.57	54.00
XS-3	IMP	0.933	0.20	0.013	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
V2-2	PERV	0.146	0.20	0.013	0%	100%	5	250	0.6	74.00	0.17	0.11	89.24	179.12	0.57	54.00
XS-4	IMP	4.181	0.66	0.008	100%	0%	0	250	0.6	98.00	0.28	0.19	5.18	244.42	0.96	93.00
۸۵-4	PERV	0.098	0.66	0.008	0%	100%	5	250	0.6	74.00	0.37	0.25	89.24	179.12	0.57	54.00
XS-5	IMP	2.494	0.26	0.002	100%	0%	0	250	0.6	98.00	0.23	0.15	5.18	244.42	0.96	93.00
V2-2	PERV	0.032	0.26	0.002	0%	100%	5	250	0.6	74.00	0.30	0.20	89.24	179.12	0.57	54.00
XS-6	IMP	4.144	0.50	0.005	100%	0%	0	250	0.6	98.00	0.27	0.18	5.18	244.42	0.96	93.00
A3-0	PERV	0.090	0.50	0.005	0%	100%	5	250	0.6	74.00	0.35	0.23	89.24	179.12	0.57	54.00
XS-7	IMP	1.730	0.12	0.020	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
Λ3-7	PERV	0.028	0.12	0.020	0%	100%	5	250	0.6	74.00	0.17	0.11	89.24	179.12	0.57	54.00
XS-8	IMP	3.756	0.29	0.012	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
V2-0	PERV	0.091	0.29	0.012	0%	100%	5	250	0.6	74.00	0.19	0.12	89.24	179.12	0.57	54.00
XS-9	IMP	0.922	0.13	0.006	100%	0%	0	250	0.6	98.00	0.17	0.11	5.18	244.42	0.96	93.00
Y2-3	PERV	0.029	0.13	0.006	0%	100%	5	250	0.6	74.00	0.17	0.11	89.24	179.12	0.57	54.00

Specific Peak flow q*	Peak flow rate q _p	Runoff volume V _{pervious}	Catchment Discharge
m ³ /s/(km ² mm)	m³/s	m ³	m³/s/km²
0.16	0.535	3183.84	41.09
0.13	0.021	111.77	33.52
0.16	1.12	6686.65	41.09
0.13	0.04	190.41	33.52
0.16	0.38	2281.43	41.09
0.13	0.05	260.80	33.52
0.15	1.52	10220.25	36.35
0.11	0.03	174.82	26.35
0.16	0.97	6096.61	38.84
0.12	0.01	56.78	28.98
0.15	1.54	10128.35	37.20
0.11	0.02	161.57	26.97
0.16	0.71	4227.52	41.09
0.13	0.01	50.87	33.52
0.16	1.54	9180.24	41.09
0.13	0.03	163.18	33.10
0.16	0.38	2254.55	41.09
0.13	0.01	51.23	33.52

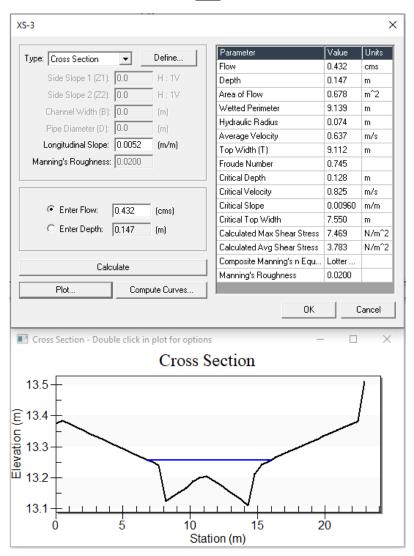
	Peak flow rate	Pipe diameter	Blockage factor	Pipe capacity with allowance for blockage	Final overland flow	Flow depth	Average velocity	Crite	eria	Max flow depth x Average velocity (dxv)
	q _p	DN						Pedestrian	Vehicle	
	m³/s	mm		m³/s	m³/s	m	m/s	safety	safety	m²/s
XS-1	0.556	825	50%	1.075	0.000	0.000	0.000	0.4	0.3	0.000
XS-2	1.160	375	100%	0.000	1.160	0.128	1.863	0.4	0.3	0.238
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XS-8	1.574	750	50%	0.482	1.091	0.232	1.177	0.4	0.3	0.273
XS-9	0.389	750	50%	0.650	0.000	0.000	0.000	0.4	0.3	0.000

Attachment B Cross-sections

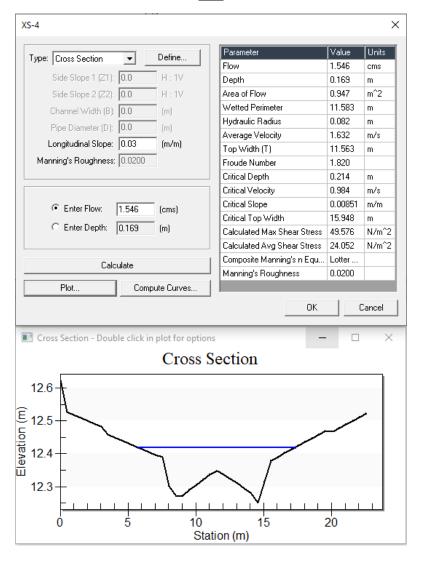
XS 2



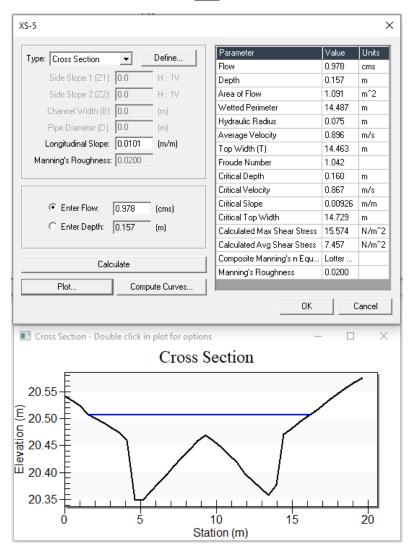
XS 3



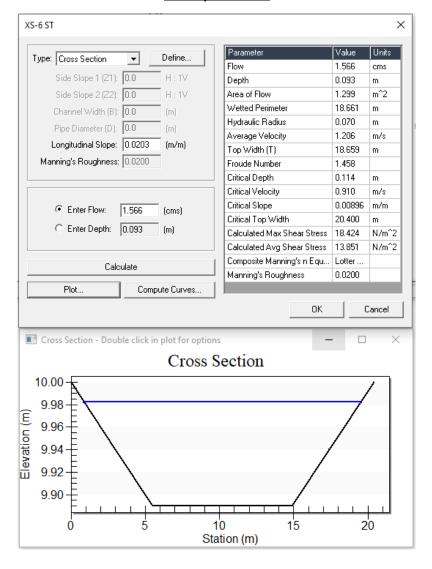
XS 4



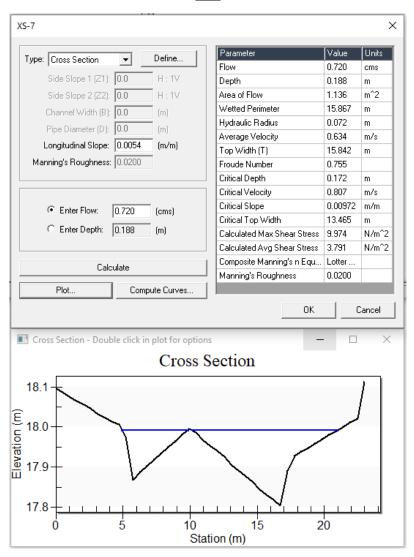
XS 5



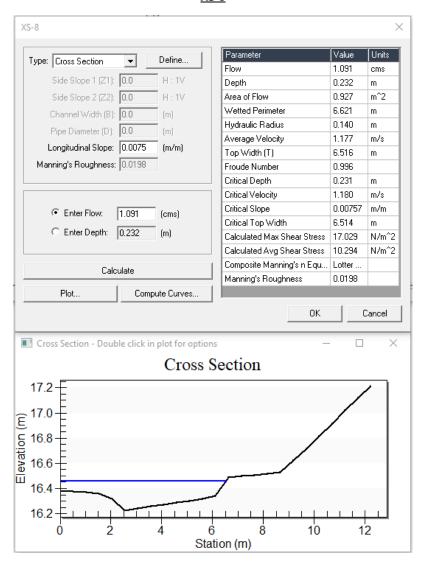
XS 6 (Speed Table)



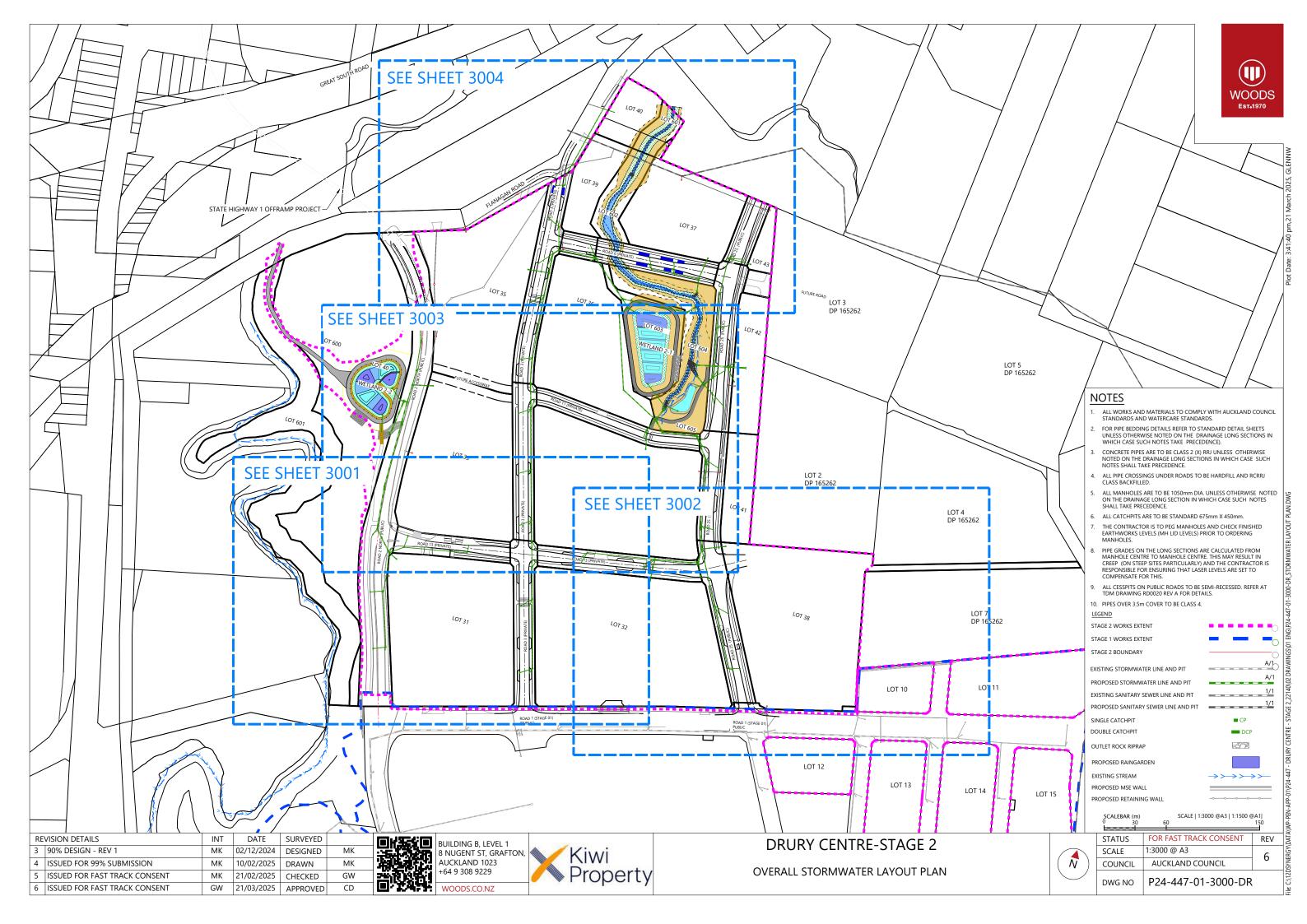
XS 7

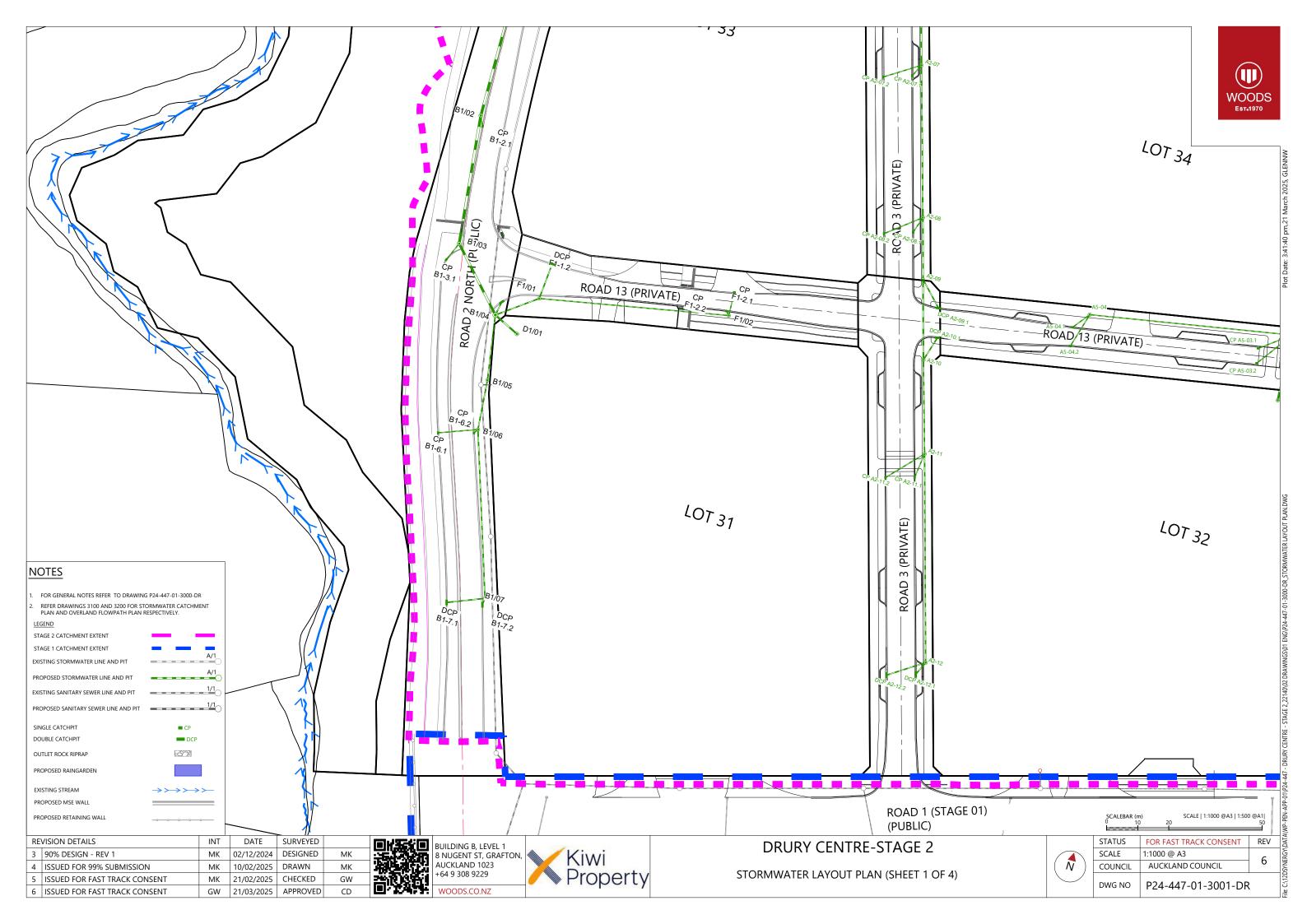


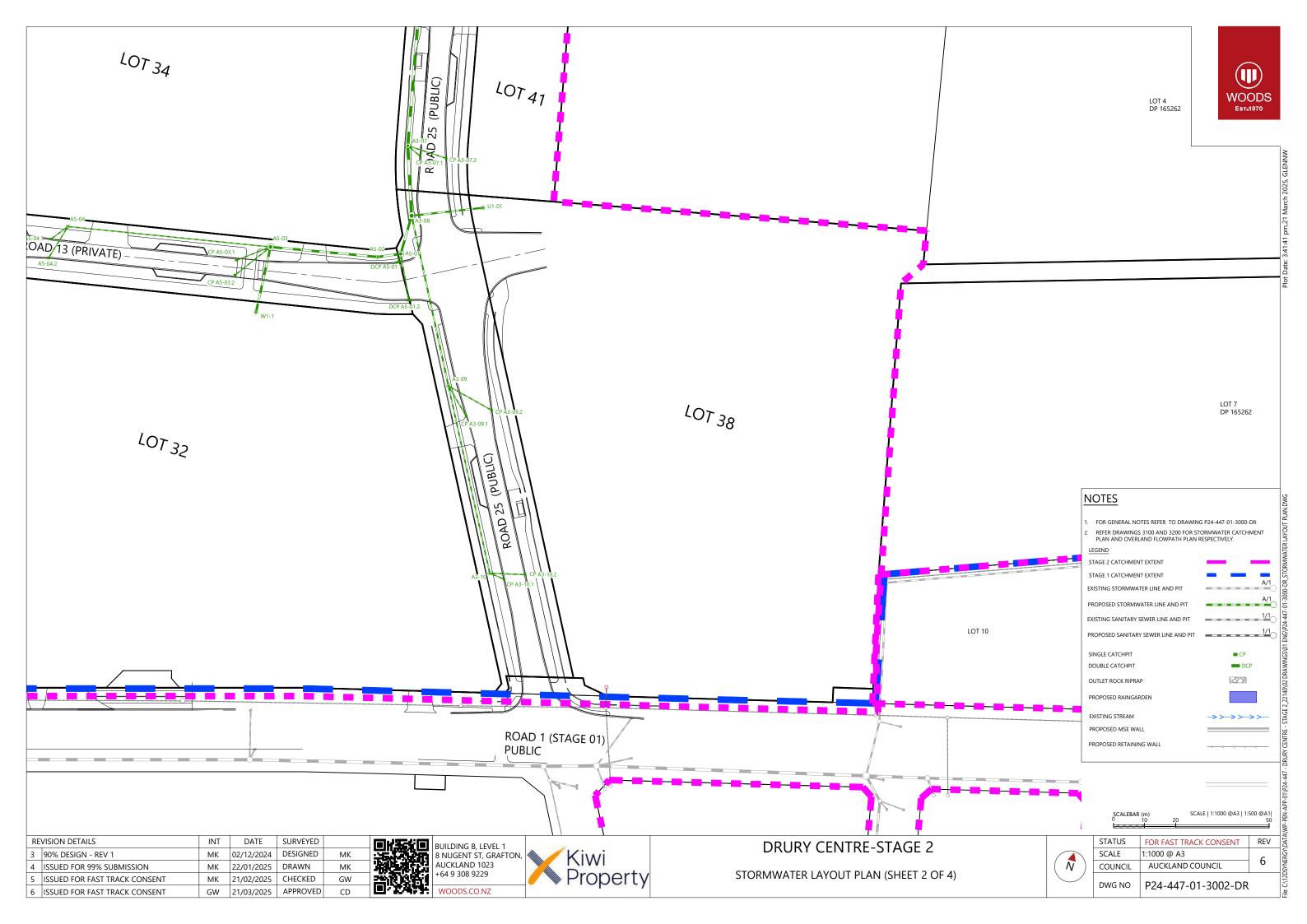
XS 8

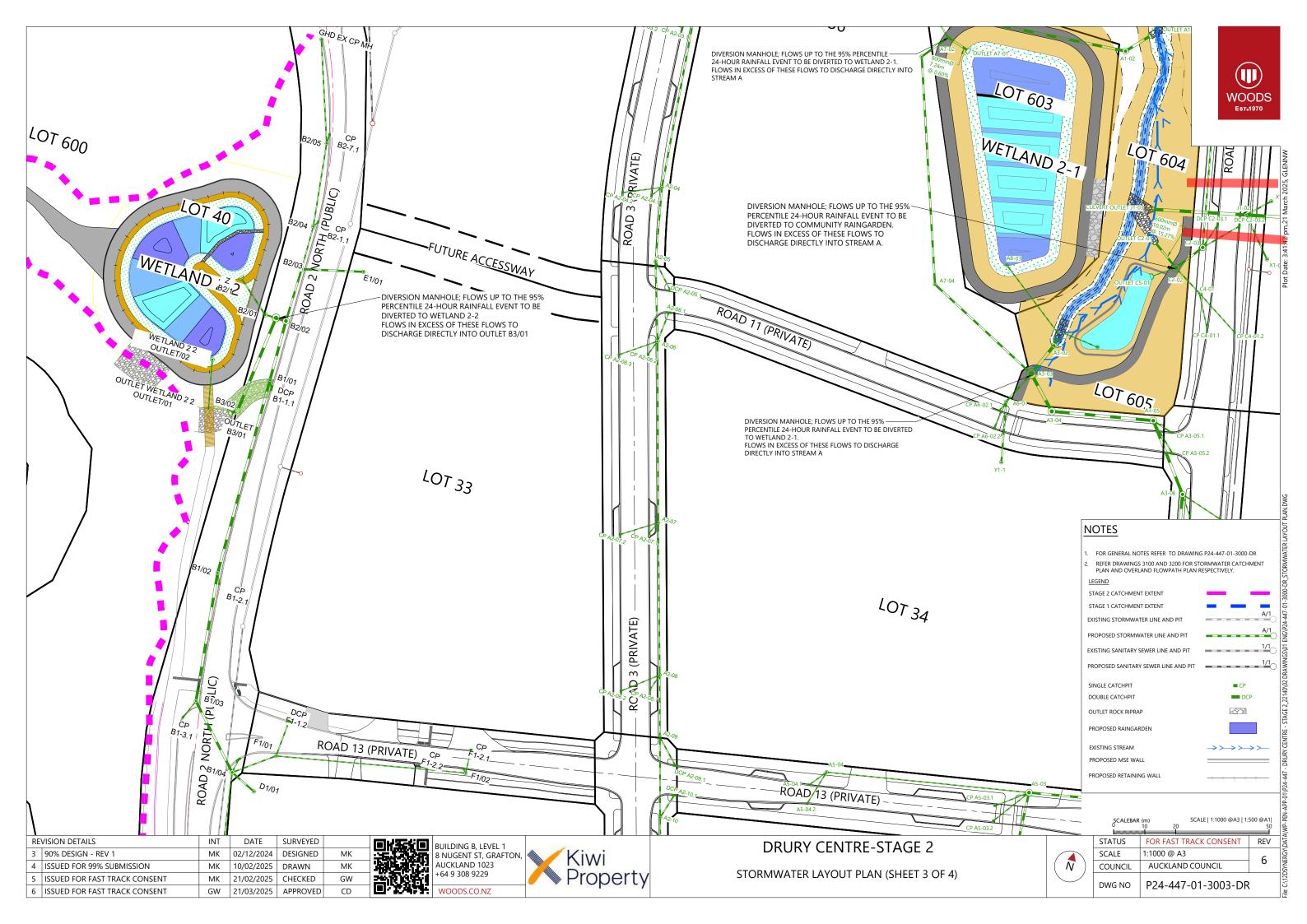


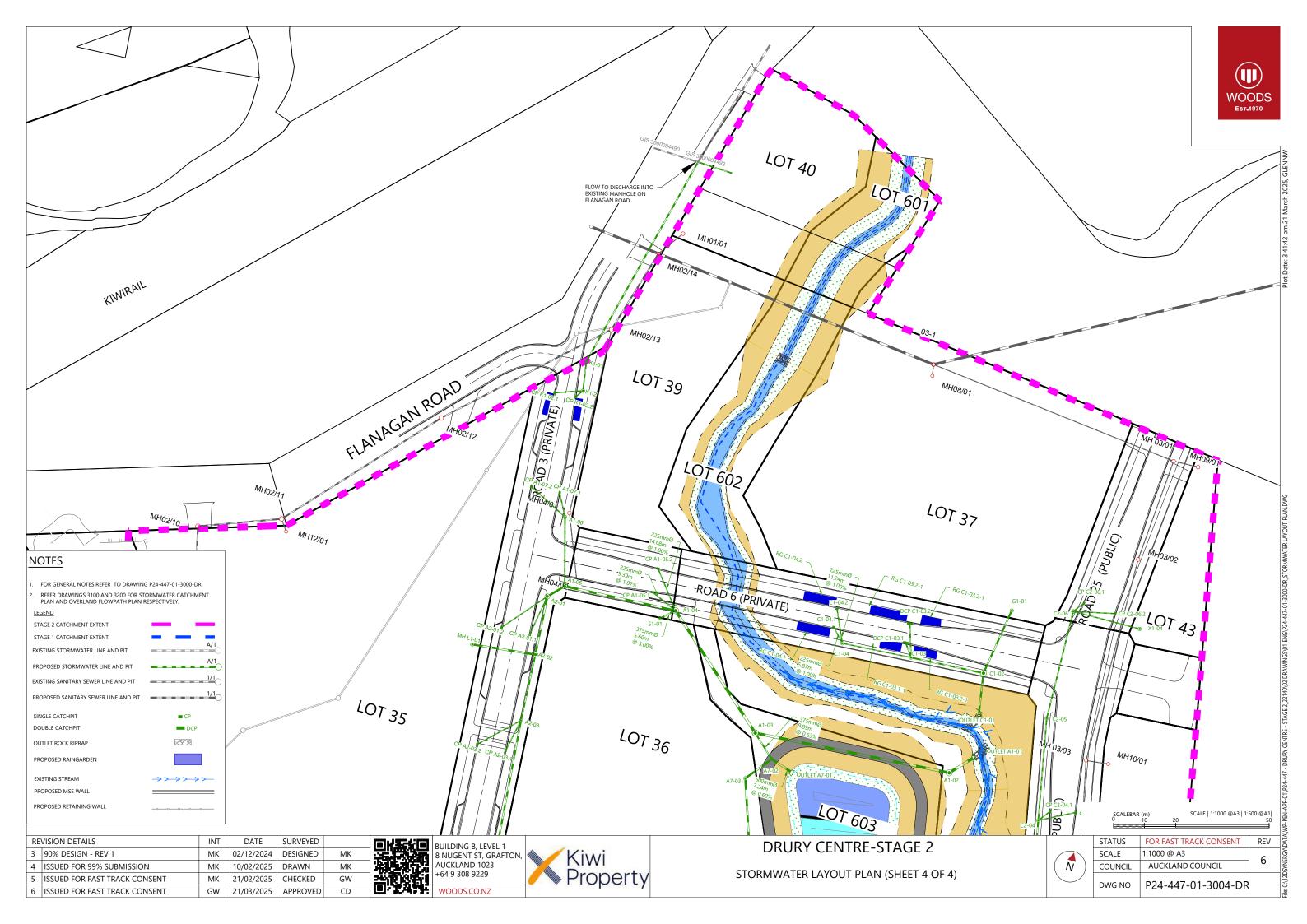
Attachment C Proposed stormwater network











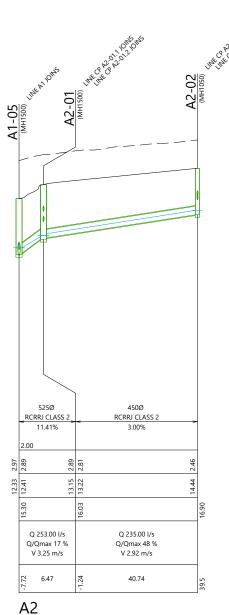
- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE) EXISTING GROUND

375Ø RCRRJ CLASS 4 Q 0.00 I/s V 0.00 m/s

A1-03 TO A7-02



		A1-03	мн 1800)	(MH1800)	(MH1500)
OUTLET A1-01	(WINGWALL GUTLET) A1-02	000H18000			
PIPE DIAMETER PIPE TYPE PIPE GRADE	750Ø RCRRJ CLASS 2 2.00%	750Ø RCRRJ CLASS 4 5.00%	750Ø RCRRJ CLASS 2 1.00%	600Ø RCRRJ CLASS 2 0.75%	300Ø RCRRJ CLASS 4 1.70%
DATUM RL= DEPTH TO INVERT %	-2.00	0 0	ω ω		3
Ö			43 2.28	12.06 2.11	
					15.30 12.63
HYDRAULIC PARAMETERS	Q 506.00 l/s Q/Qmax 31 % V 2.47 m/s	Q 507.00 l/s Q/Qmax 22 % V 3.26 m/s	Q 507.00 l/s Q/Qmax 44 % V 2.20 m/s	Q 275.00 Vs Q/Qmax 52 % V 1.69 m/s	Q 22.00 l/s Q/Qmax 17 % V 1.31 m/s
CHAINAGE/ LENGTH	9.84	63.64	84. 46.93	36.5	6.95

Α1

BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023

DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 1 OF 19)

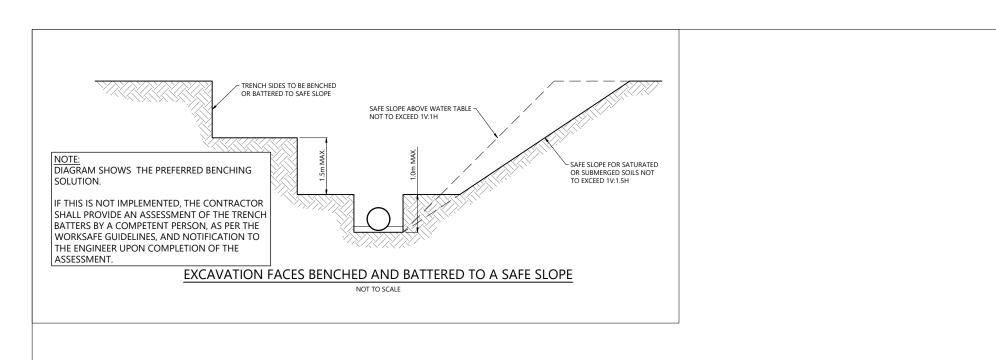
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SCALE	H 1:1000 @ A3 V 1:200 @ A3	3	
COUNCIL	AUCKLAND COUNCIL) 3	
DWG NO	P24-447-01-3500-DR	\ \	

WOODS EsT-1970

e: C:\12DS\DATA\WP-AKL-APP-02\P24-447 - DRURY CENTRE - STAGE 2_22140\02 DRAW

REVISION DETAILS DATE SURVEYED 1 FOR DISCUSSION MK 22/01/2025 DESIGNED MK 2 FOR THE 99% SUBMISSION MK MK 10/02/2025 DRAWN 3 FOR FAST TRACK CONSENT EW 21/02/2025 CHECKED GW APPROVED CD





- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
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- 6. NETWORK MODELLED TO TP108 2.1CC.

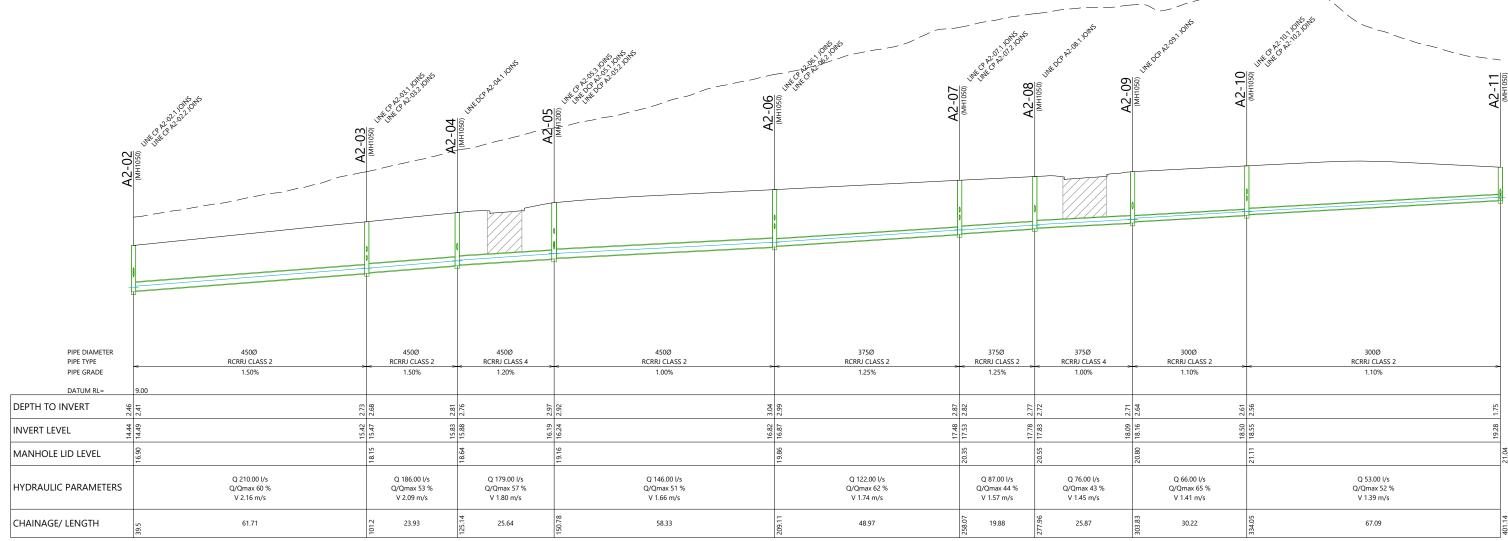
LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND



A2

1					
REVISION DETAILS		INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				APPROVED	CD



BUILDING B, LEVEL 1
8 NUGENT ST, GRAFTON,
AUCKLAND 1023
+64 9 308 9229

WOODS.CO.NZ



DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 2 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV	
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3	
COUNCIL	AUCKLAND COUNCIL) 	
DWG NO	P24-447-01-3501-DR	\ \	

WOODS Est.1970

ite: 11:44:39 am,21 February 2025, ELLAW

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- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
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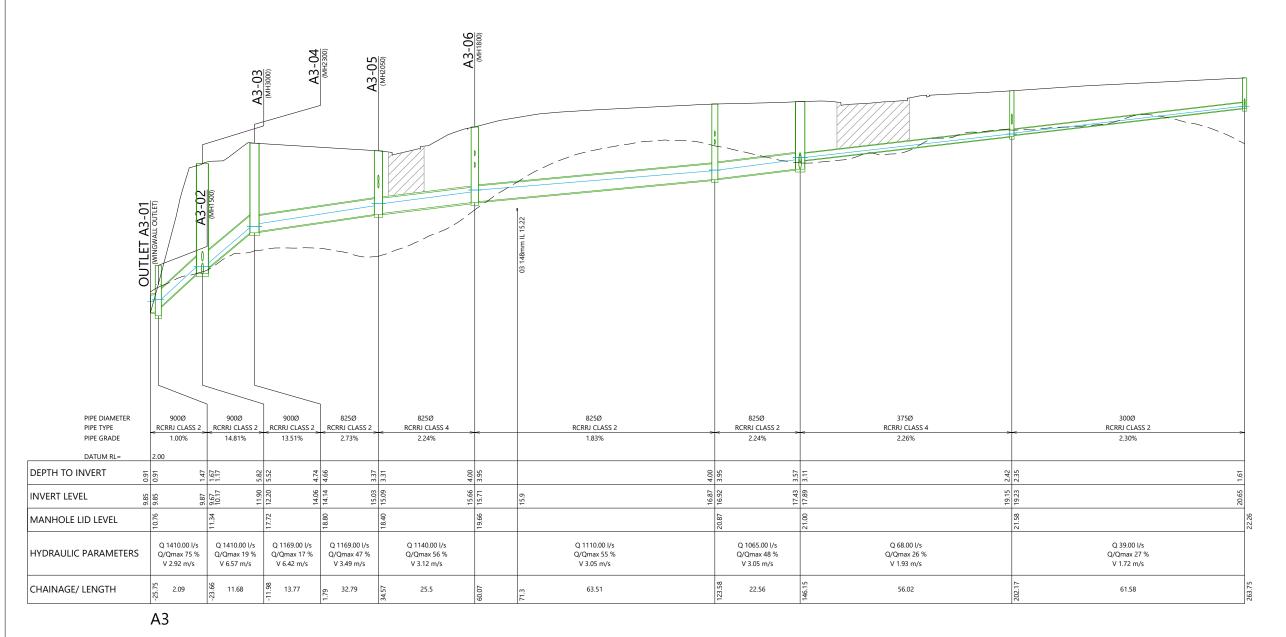
LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND



RCRRJ CLASS 2 RCRRJ CLASS 2 0.50% 0.50% Q 576.00 l/s Q/Qmax 93 % Q/Qmax 88 % V 1.63 m/s 12.75 7.52

A5-01

П	RE	VISION DETAILS	INT	DATE	SURVEYED	
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
					APPROVED	CD



BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, **AUCKLAND 1023** +64 9 308 9229 WOODS.CO.NZ



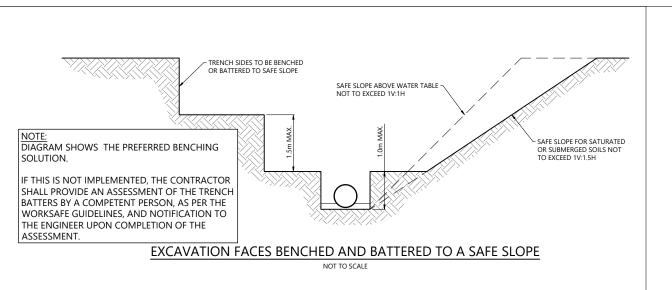
DRURY CENTRE-STAGE 2

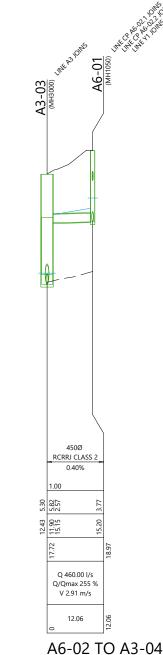
STORMWATER LONGSECTION (SHEET 3 OF 19)

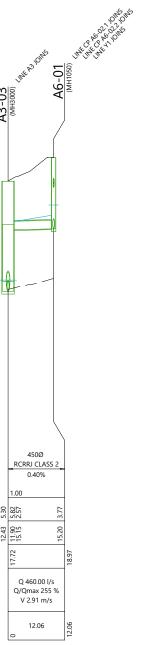
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SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3502-DR	

A5

WOODS EsT-1970







- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND

A3-03 (MH3000) 525Ø RCRRJ CLASS 2 RCRRJ CLASS 4 RCRRJ CLASS RCRRJ CLASS 2 RCRRJ CLASS 2 0.60% 0.60% 0.60% 0.60% 0.60% 11.60 Q 241.00 l/s Q/Qmax 72 % Q 401.00 l/s Q 401.00 l/s Q 242.00 l/s Q/Qmax 84 % Q/Qmax 84 % Q/Qmax 72 % Q/Qmax 73 % V 1.86 m/s V 1.57 m/s V 1.12 m/s V 1.26 m/s V 1.44 m/s 73.3 42.23 5.12 2.12 10.49

RE'	VISION DETAILS	INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				APPROVED	CD

A5

675Ø

RCRRJ CLASS 2

0.50%

Q 548.00 l/s Q/Qmax 88 %

V 1.48 m/s

34.56

17.86

PIPE DIAMETER

PIPE TYPE

DATUM RL

DEPTH TO INVERT

MANHOLE LID LEVEL

CHAINAGE/ LENGTH

HYDRAULIC PARAMETERS

INVERT LEVEL

PIPE GRADE



RCRRJ CLASS 2

1.60%

Q 10.00 l/s Q/Qmax 8 %

V 0.86 m/s

65.42

BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229 WOODS.CO.NZ



DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION	(SHEET 4	OF 19

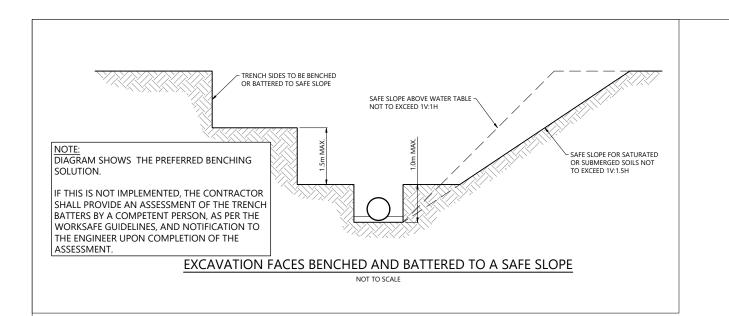
Α7

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL) 3
DWG NO	P24-447-01-3503-DR	2

g: C:\12DS\DATA\WP-AKL-APP-02\P24-447 - DRURY CENTRE - STAGE 2_22140\02 DRA

 (\mathbf{H}) WOODS

EsT-1970



- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.

DEPTH OF FLOW (HYDRAULIC GRADELINE)

6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

W. K

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE

EXISTING GROUND

OUTLET C1-01 RCRRJ CLASS 2 RCRRJ CLASS 2 RCRRJ CLASS 2 RCRRJ CLASS 2 9.34% Q 0.00 l/s Q/Qmax 0 % O 260.00 I/s O 70.00 I/s O 28.00 I/s Q/Qmax 40 % V 1.59 m/s Q/Qmax 30 % Q/Qmax 23 % V 0.00 m/s V 4.20 m/s V 2.15 m/s 27.29 22.58 12.85 26.04 C1

-01 ராள்	C2-02 WEIR	C2-02	(MH2300) May 2 May 1, 10 May 2 May 1, 10 May 2 May 1, 10 May 2 May	THE THE TOTAL THE	C2-04	(MH1050) (MH	(MH1050)	C2-06 (MH1050) (MH1050)
OUTLET C2-01				. — — \				
600 RCRR) 0 16.9	CLASS 2	18000 WEIR 20.62%	600Ø RCRRJ CLASS 2 0.40%	375Ø RCRRJ CLASS 2 0.40%		3000 RCRI CLASS 2 0.40%	300Ø RCRRJ CLASS 4 0.40%	
09:0	1.97	1.83	1.98	2.05	1.75	1.68	1.60	1.57
9.70	11.24	11.39	11.24	11.50	11.70	11.77	11.95	12.09
10.30	12.21	7 2	13.22	13.55			13.56	13.67
Q 135	.00 l/s ax 5 %	Q 135.00 l/s Q/Qmax 0 % V 1.00 m/s	Q 121.00 l/s Q/Qmax 31 % V 1.12 m/s	Q 81.00 l/s Q/Qmax 73 % V 1.10 m/s		Q 53.00 l/s Q/Qmax 87 % V 1.02 m/s	Q 53.00 l/s Q/Qmax 87 % V 0.94 m/s	
-181.46	29	0.73	10.85	65: 47.97		32.91	1.797 35.44	-44.27

MK

MK

GW

CD

PIPE DIAMETER PIPE TYPE

DEPTH TO INVERT INVERT LEVEL

MANHOLE LID LEVEL

CHAINAGE/ LENGTH

REVISION DETAILS

1 FOR DISCUSSION

2 FOR THE 99% SUBMISSION

3 FOR FAST TRACK CONSENT

HYDRAULIC PARAMETERS

Α8

MK

MK

EW

DATE

22/01/2025

10/02/2025

21/02/2025 CHECKED

SURVEYED

DESIGNED

APPROVED

DRAWN

BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, **AUCKLAND 1023** +64 9 308 9229 WOODS.CO.NZ



C2

STORMWATER LONGSECTION (SHEET 5 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	٥
DWG NO	P24-447-01-3504-DR	

WOODS EsT-1970

- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
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- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

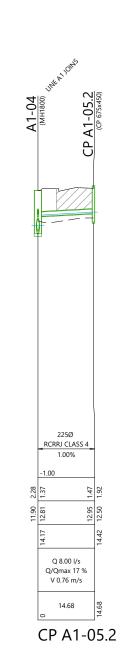
DEPTH OF FLOW (HYDRAULIC GRADELINE)

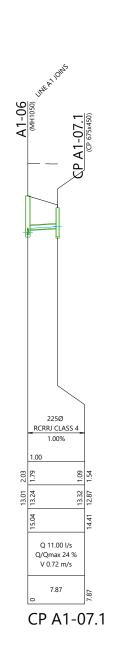
EXISTING GROUND



	PIPE DIAMETER	A1-04	UNITA 1016	d	(059x529 d2)
	PIPE DIAMETER PIPE TYPE PIPE GRADE		225Ø RCRRJ CL/ 2.13%	ASS 4	
DEPTH TO IN	DATUM RL=	80	-1.00	- 2	7
		0 2.28		5 1.12	14.27 12.70 1.57
INVERT LEVE	!L	11.90	14.17 12.95	13.15	12.7
MANHOLE L	ID LEVEL		14.17		14.27
HYDRAULIC	PARAMETERS		Q 7.00 Q/Qmax V 0.98 n	11 %	

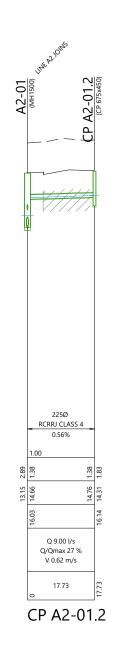
CHAINAGE/ LENGTH











2(Int at John	- -	20)
A2-02	(MH10	₹P A2-02	(CP 675x450)
ſ		9	
-		-	
		\	
	225Ø RCRRJ CLASS 3.00%	54_	
2.46	2.00	1.12	1.57
14.44 2	15.47	15.79	15.34
	16.90		16.91
	Q 13.00 l/s Q/Qmax 17 V 1.24 m/s	%	
	10.81		10.81
	CP A2	-0	2.1

REVISION DETAILS		INT	DATE	SURVEYED	1	
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
					APPROVED	CD

CP A1-05.1



BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229



STORMWATER LONGSECTION	(SHEET 6	5 OF	19

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3505-DR	

VGS\01 ENG\P24-447-01-3500-DR_STORMWAITER LONGSECTION. DWG Plot Date: 11:44

- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.

DEPTH OF FLOW (HYDRAULIC GRADELINE)

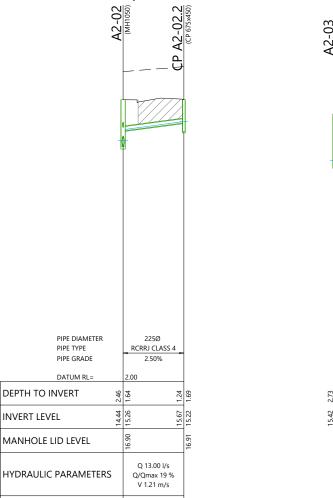
6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

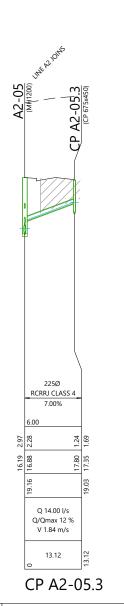
PROPOSED HARDFILL BACKFILL

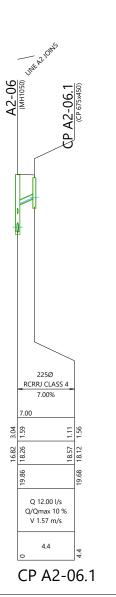
PROPOSED FINISHED SURFACE

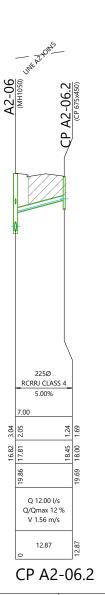
EXISTING GROUND

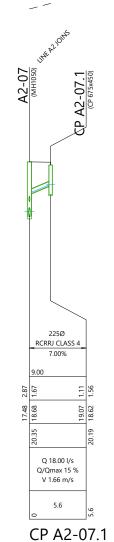


QP A2-03.1	(CP 675x450)	A2-03	The by June -	CP A2-03.2
	18.01 16.44 1.57	15.42 2.73	18.15	16.77 1.39
Q 12.00 I/s /Qmax 10 % V 1.55 m/s 4.18	4.18		Q 13.00 l/s Q/Qmax 13 ⁹ V 1.57 m/s	%









REVISION DETAILS		INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				ADDB∪\/ED	CD

16.08

CP A2-02.2

CHAINAGE/ LENGTH



CP A2-03.1

BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229 WOODS.CO.NZ

CP A2-03.2



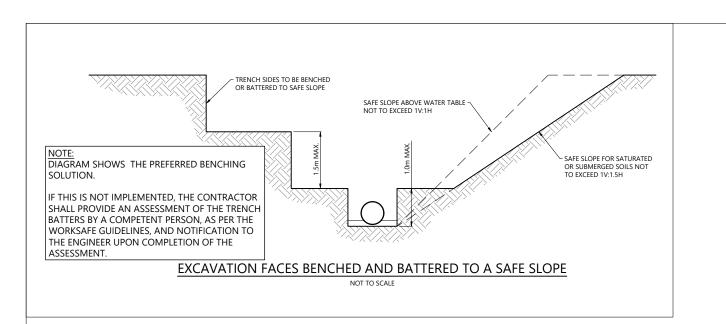
DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 7	OF	19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	3
DWG NO	P24-447-01-3506-DR	

WOODS EsT-1970

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- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
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- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

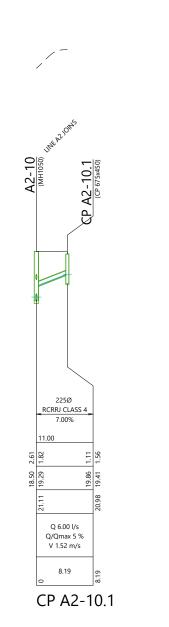
LEGEND

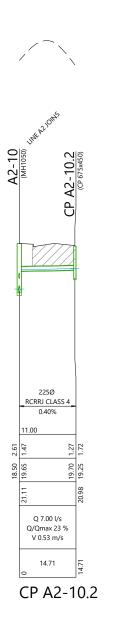
PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND

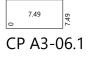
RCRRJ CLASS 4 RCRRJ CLASS 4 RCRRJ CLASS 4 7.00% 5.00% 7.00% Q 16.00 l/s Q 23.00 l/s Q 22.00 l/s Q/Qmax 14 % Q/Qmax 20 % Q/Qmax 22 % V 1.69 m/s 14.51 5.21 13.03 CP A3-06.2 CP A3-07.1 CP A3-07.2

!	A2-07	(MH1050) The Parameter Par	(CP 675x450)
PIPE DIAMETER PIPE TYPE PIPE GRADE		225Ø _RCRRJ CLASS 4 5.00%	
DATUM RL=	37	9.00	69
INVERT LEVEL	17.48 2.87	18.28 2.07	1
MANHOLE LID LEVEL		20.35	20.19 18.50
HYDRAULIC PARAMETERS		Q 18.00 l/s Q/Qmax 18 % V 1.66 m/s	
CHAINAGE/ LENGTH		13.51	13.51
		-	J









DRURY CENTRE-STAGE

STORMW	ATER LONGSECTI	ON (SHEET	8 OF	19)

STATUS	FOR FAST TRACK CONSENT	REV	
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3	
COUNCIL	AUCKLAND COUNCIL	3	
DWG NO	P24-447-01-3507-DR		

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WOODS

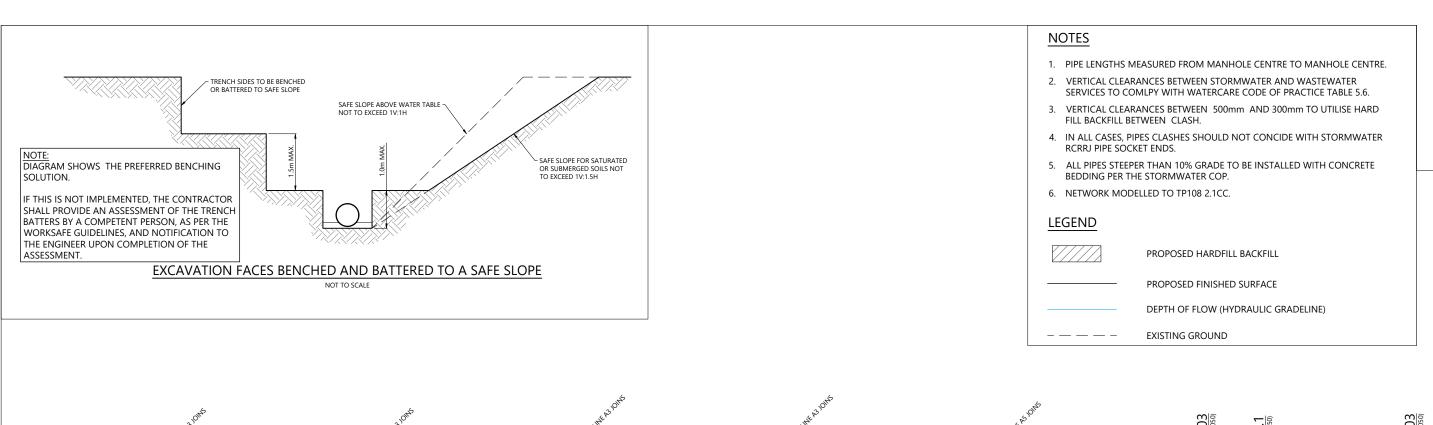
EsT-1970

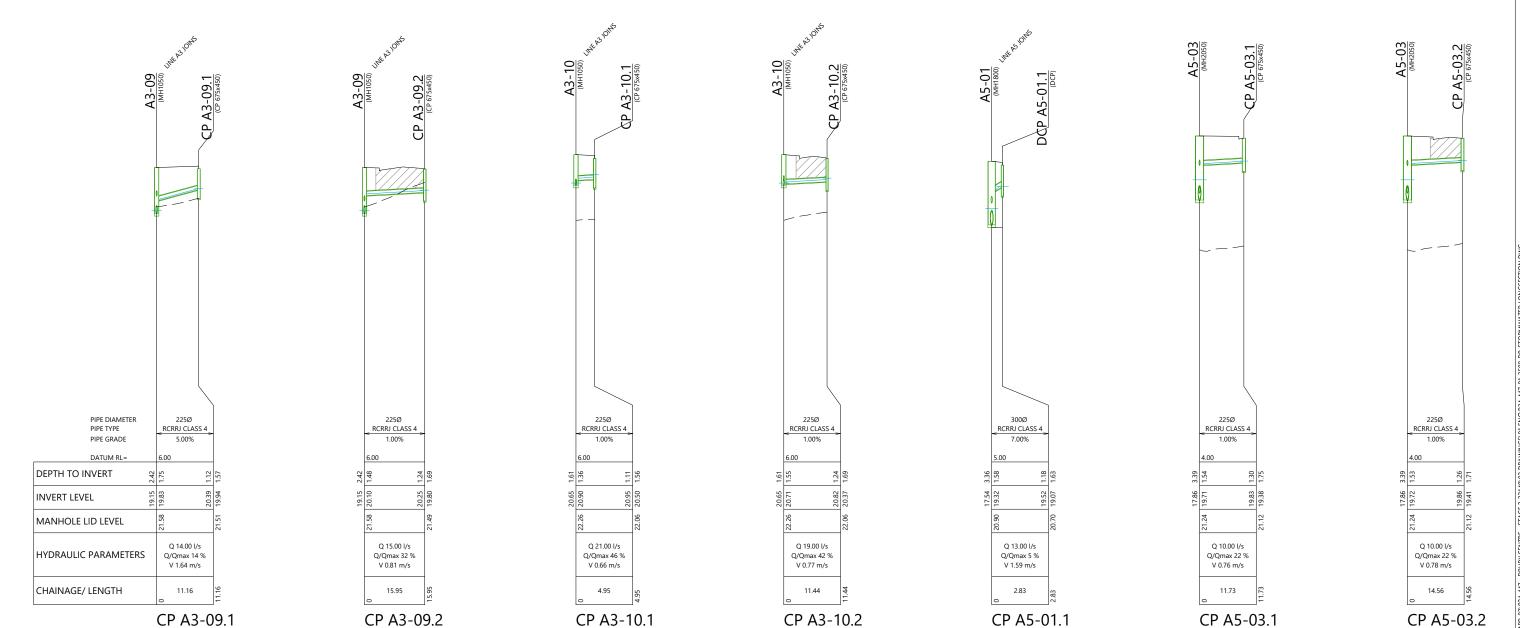
REVISION DETAILS DATE SURVEYED 1 FOR DISCUSSION 22/01/2025 DESIGNED MK MK 2 FOR THE 99% SUBMISSION MK 10/02/2025 DRAWN MK 3 FOR FAST TRACK CONSENT EW 21/02/2025 CHECKED GW APPROVED CD

CP A2-07.2



BUILDING B, LEVEL 1 Kiwi Property 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229 WOODS.CO.NZ





REVISION DETAILS DATE SURVEYED 1 FOR DISCUSSION MK 22/01/2025 DESIGNED MK 2 FOR THE 99% SUBMISSION MK MK 10/02/2025 DRAWN 3 FOR FAST TRACK CONSENT GW EW 21/02/2025 CHECKED APPROVED CD



BUILDING B, LEVEL 1
8 NUGENT ST, GRAFTON,
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DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 9 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	3
DWG NO	P24-447-01-3508-DR	

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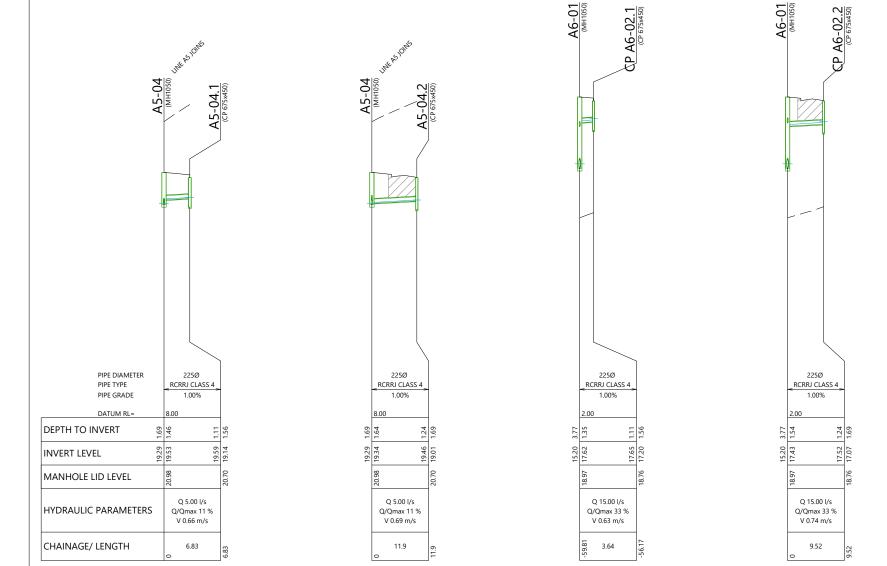
WOODS

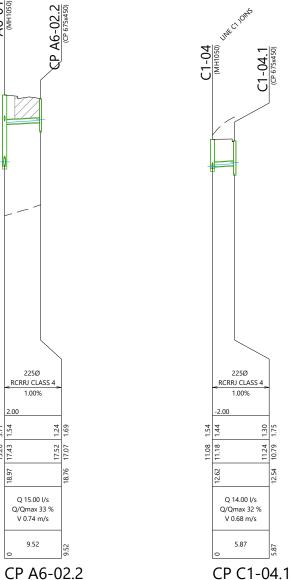
EsT-1970

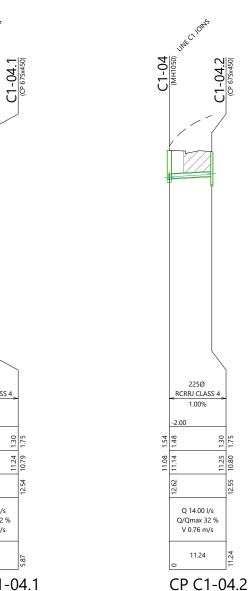
1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.

- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE
- 6. NETWORK MODELLED TO TP108 2.1CC.

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE)







C2-04	(MH1050) (CP C2-04.1	(CP 675x450)
Ā	y y	
	225Ø	
	RCRRJ CLASS 4 7.77%	
	-3.00	
11.70 1.75	1.70	13.23 11.67 1.56
11.70	11.75	11.67
	13.45	13.23
	Q 10.00 l/s Q/Qmax 8 % V 0.77 m/s	
	4.86	4.86
	CP C2-0	•

 (\mathbf{H})

WOODS

EsT-1970

REVISION DETAILS		INT	DATE	SURVEYED		
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
					APPROVED	CD

CP A5-04.1



CP A5-04.2

BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229 WOODS.CO.NZ

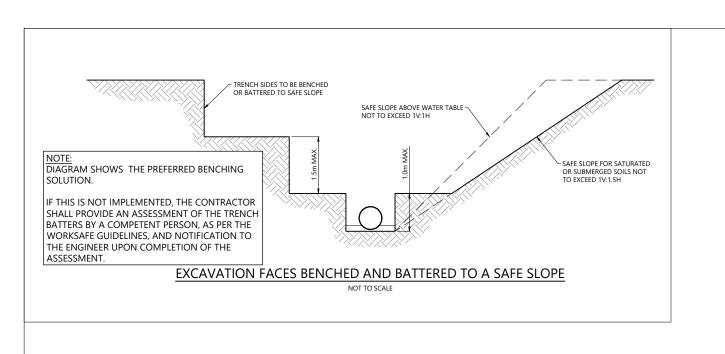
CP A6-02.1



DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 10 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	2
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3509-DR	





- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
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- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND



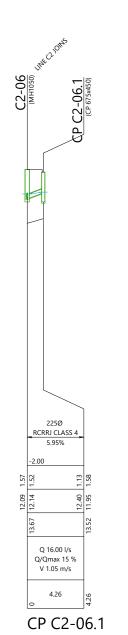
	C2-04	C NO-CO 00	(05kx(2) dD)
PIPE DIAMETER PIPE TYPE PIPE GRADE	-	225Ø RCRRJ CLASS 4 2.01%	-

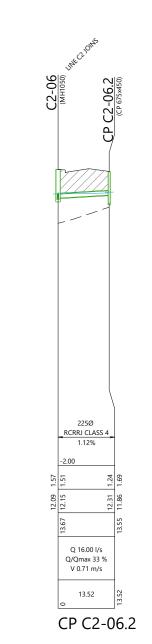
DEPTH TO INVERT INVERT LEVEL

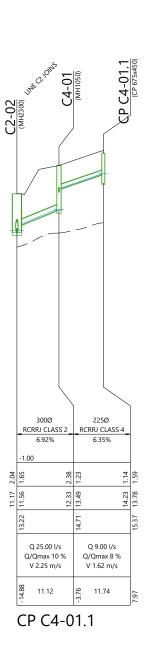
MANHOLE LID LEVEL

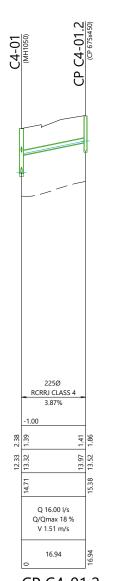
CHAINAGE/ LENGTH

HYDRAULIC PARAMETERS

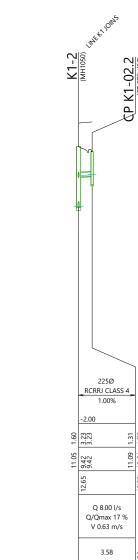












REVISION		VISION DETAILS	INT	DATE	SURVEYED	
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
					APPROVED	CD

O 18.00 l/s

Q/Qmax 29 %

V 0.84 m/s

12.29

CP C2-04.2



BUILDING B, LEVEL 1 8 NUGENT ST, GRAFTON, AUCKLAND 1023 +64 9 308 9229 WOODS.CO.NZ



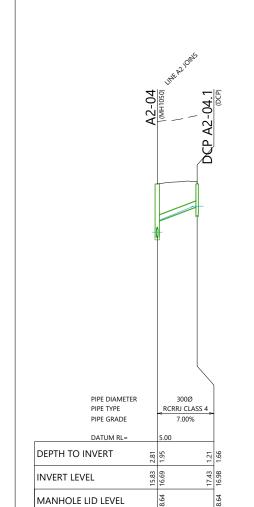
DRURY CENTRE-STAGE 2

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	2
COUNCIL	AUCKLAND COUNCIL	3
DWG NO	P24-447-01-3510-DR	 }

STORMWATER LONGSECTION (SHEET 11 OF 19)

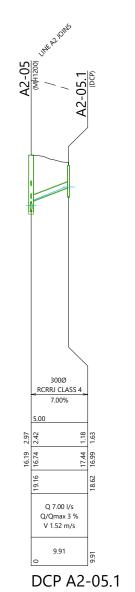
CP C4-01.2 CP K1-02.1

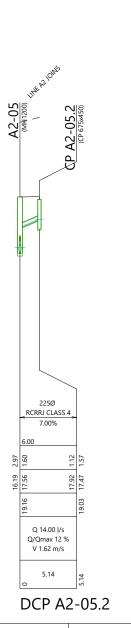
CP K1-02.

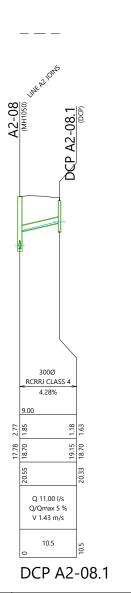


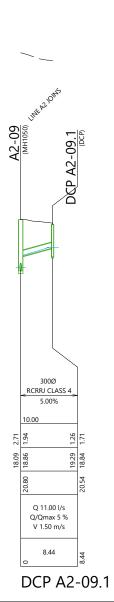
HYDRAULIC PARAMETERS

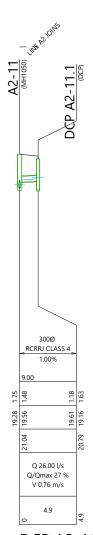
CHAINAGE/ LENGTH

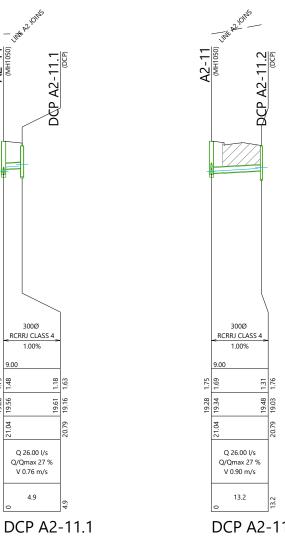












REVISION DETAILS		INT	DATE	SURVEYED		
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
					APPROVED	CD

DCP A2-04.1

Q/Qmax 3 % V 1.55 m/s



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DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 12 OF 19)

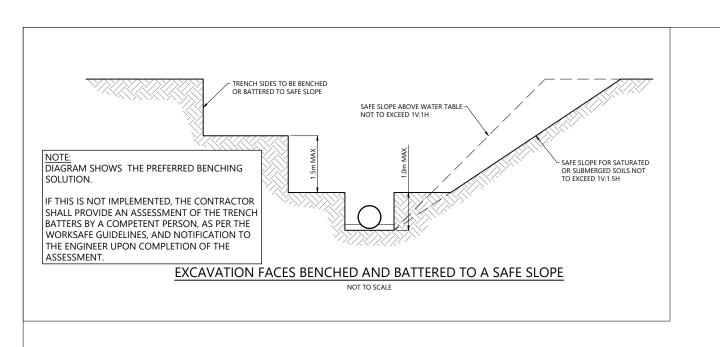
STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	2
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3511-DR	

DCP A2-11.2

 (\mathbf{H})

WOODS

EsT-1970





- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND

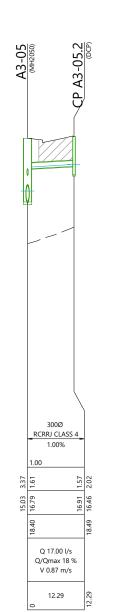
PIPE DIAMETER
PIPE TYPE
PIPE GRADE
DATUM RL=

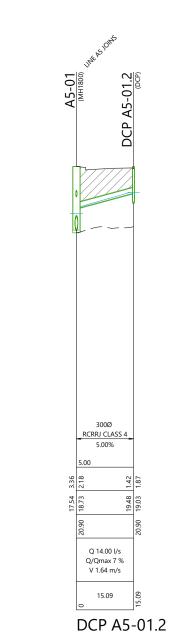
INVERT LEVEL

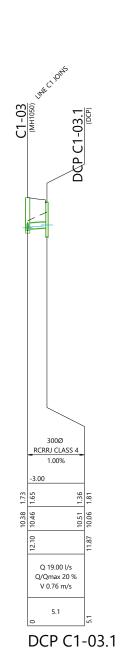
MANHOLE LID LEVEL

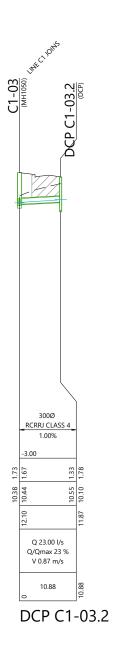
HYDRAULIC PARAMETERS

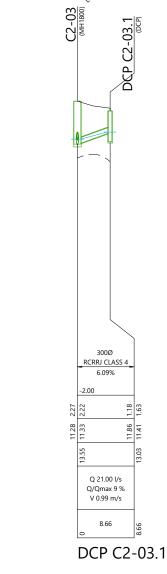
Q 12.00 l/s
Q/Qmax 5 %
V1.73 m/s

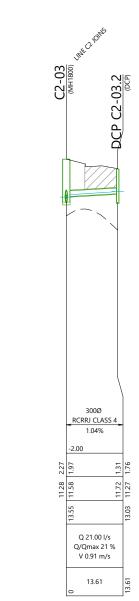












DCP A3-05.1

6.83

CHAINAGE/ LENGTH

REVISION DETAILS		INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				APPROVED	CD



DCP A3-05.2

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DRURY	CENTRE	E-STAGE	2
	CLIVIT		

STORMWATER LONGSECTION	(SHEET	13	OF	19)

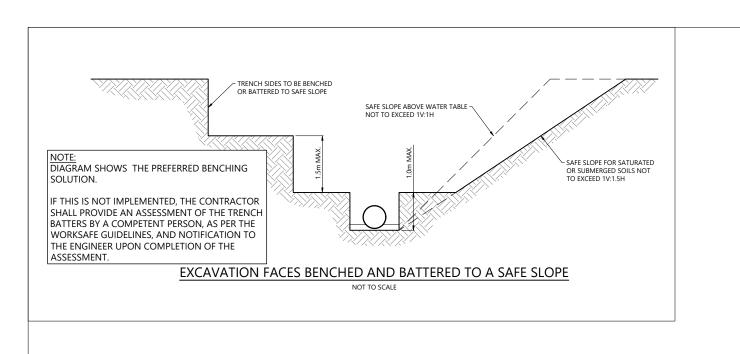
STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	3
DWG NO	P24-447-01-3512-DR	

DCP C2-03.2

WOODS EST-1970

ot Date: 11:44:43 am,21 February 2025, I

NOP24-447-01-3500-DR_STORMWATER LONGSECTION.DWG



GIS 3000084492 RCRRJ CLASS 4 1.50% O/Omax 113 % V 1.92 m/s 11.98

OUTLET J1-03

RCRRJ CLASS 4

3.50%

Q 176.00 l/s

O/Omax 5 %

31.36

J1

NOTES

- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
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- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

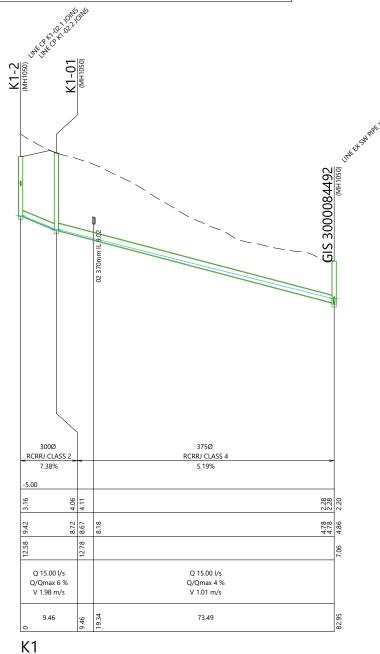
LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

— — EXISTING GROUND



RE		VISION DETAILS	INT	DATE	SURVEYED	
	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW

EX SW PIPE

RCRRJ CLASS 4

1.04%

O/Omax 83 %

15.35

GIS 3000084490

PIPE TYPE

PIPE GRADE

DEPTH TO INVERT

MANHOLE LID LEVEL

CHAINAGE/ LENGTH

HYDRAULIC PARAMETERS



RCRRJ CLASS 4

4.00%

O/Omax 33 %

21.73

G1

CD

APPROVED

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H1



DRURY CENTRE-STAGE 2

RCRRJ CLASS 4

5.39%

O/Omax 0 %

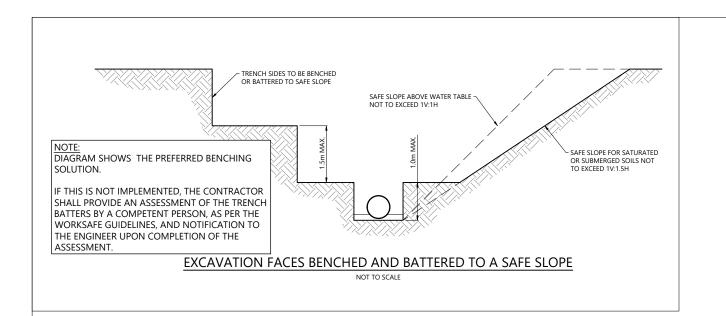
V 0.00 m/s

17.5

STORMWATER LONGSECTION ((SHEET 14 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3513-DR	

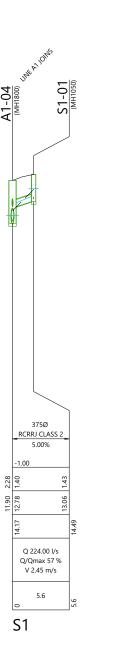
WOODS EST.1970

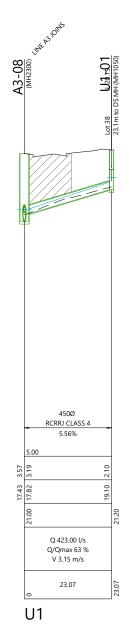


- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
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- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE) EXISTING GROUND





REVISION DETAILS		INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
4	FOR FAST TRACK CONSENT	GW	21/03/2025	APPROVED	CD



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DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 15 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	4
COUNCIL	AUCKLAND COUNCIL	4
DWG NO	P24-447-01-3514-DR	

WOODS EsT-1970

- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
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- 6. NETWORK MODELLED TO TP108 2.1CC.

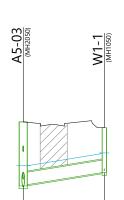
LEGEND

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE) EXISTING GROUND



WOODS

EsT-1970



675Ø

RCRRJ CLASS 4

3.00%

Q/Qmax 38 %

V 1.97 m/s

21.45

W1

PIPE DIAMETER

PIPE TYPE

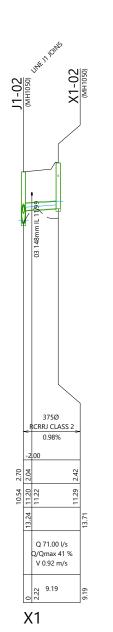
DEPTH TO INVERT INVERT LEVEL

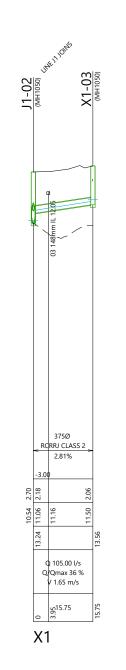
MANHOLE LID LEVEL

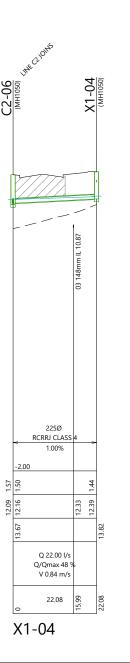
CHAINAGE/ LENGTH

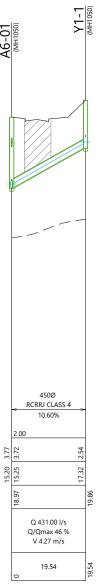
HYDRAULIC PARAMETERS

PIPE GRADE









-9K	.HW)	
4	450Ø RCRRJ CLASS 4 10.60%	
15.20 3.77	15.25 3.72	_
11	18.97	19.86
	Q 431.00 l/s Q/Qmax 46 % V 4.27 m/s	
	19.54	19.54

REVISION DETAILS		INT	DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				APPROVED	CD



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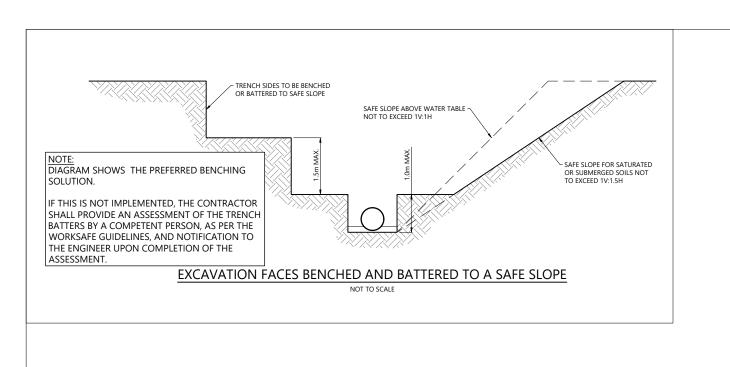


DRURY CENTRE-STAGE 2

Y1

STORMWATER LONGSECTION (SHEET 16 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3515-DR	



- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
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- 6. NETWORK MODELLED TO TP108 2.1CC.

LEGEND

PROPOSED HARDFILL BACKFILL

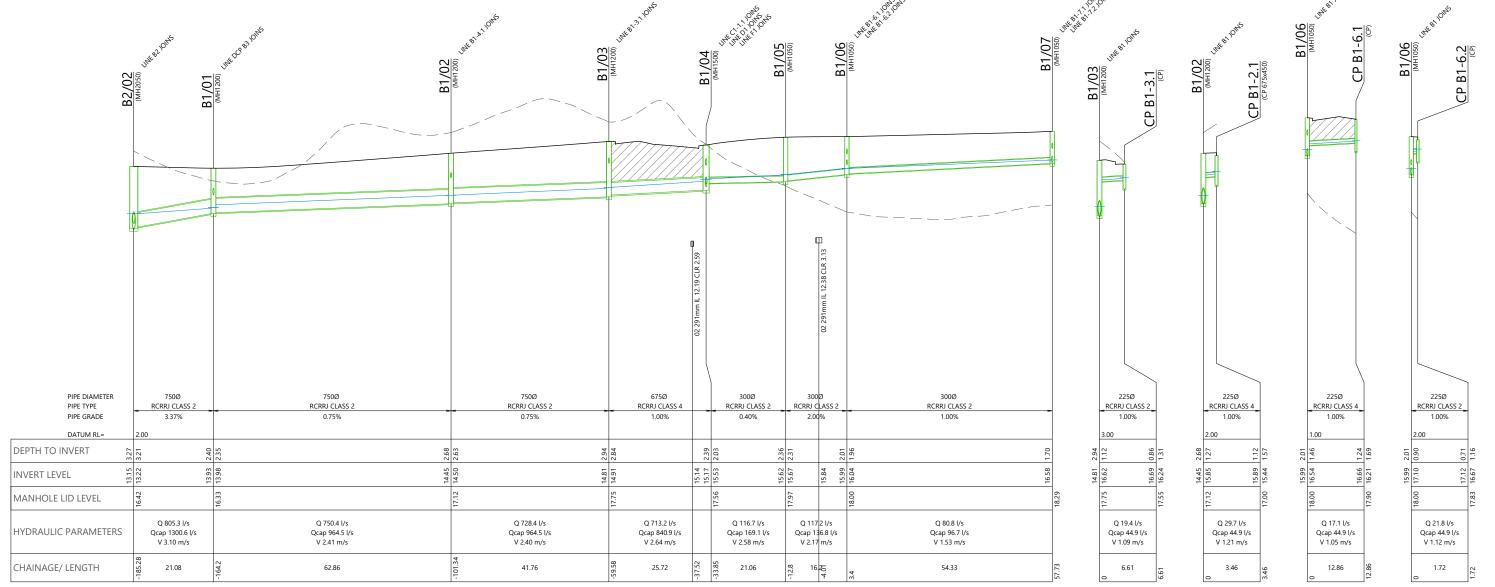
PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND

B1-3.1

B1-4.1



REVISION DETAILS DATE SURVEYED 1 FOR DISCUSSION MK 22/01/2025 DESIGNED MK MK 2 FOR THE 99% SUBMISSION MK 10/02/2025 DRAWN 3 FOR FAST TRACK CONSENT GW EW 21/02/2025 CHECKED

В1



APPROVED

CD

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DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 17 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	2
COUNCIL	AUCKLAND COUNCIL	5
DWG NO	P24-447-01-3516-DR	

B1-6.1

B1-6.2

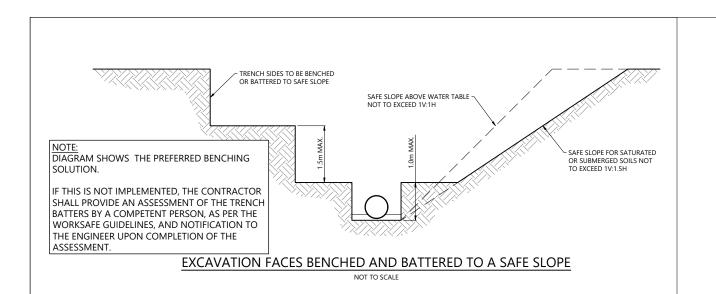
WOODS Est.1970

4 am.21 February 2025, ELLAWA

00-DR_STORMWATER LONGSECTION DWG PIO

APP-02\P24-447 - DRURY CENTRE - STAGE 2_221

B2/03 BYPASS



- 3. VERTICAL CLEARANCES BETWEEN 500mm AND 300mm TO UTILISE HARD FILL BACKFILL BETWEEN CLASH.
- 4. IN ALL CASES, PIPES CLASHES SHOULD NOT CONCIDE WITH STORMWATER RCRRJ PIPE SOCKET ENDS.
- 5. ALL PIPES STEEPER THAN 10% GRADE TO BE INSTALLED WITH CONCRETE BEDDING PER THE STORMWATER COP.
- 6. NETWORK MODELLED TO TP108 2.1CC.

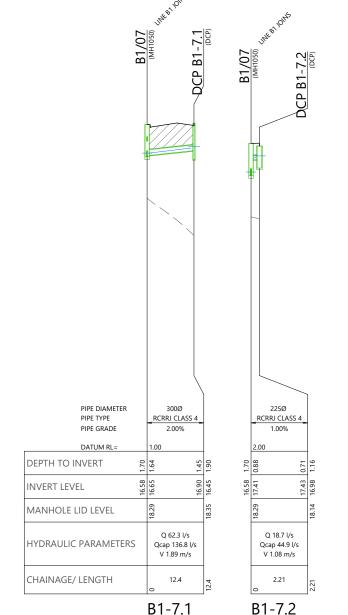
LEGEND

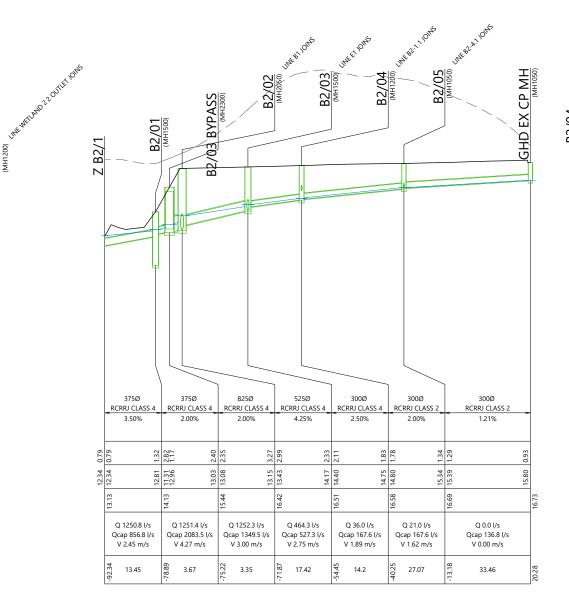
NOTES

PROPOSED HARDFILL BACKFILL PROPOSED FINISHED SURFACE DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND

IR IN BYPASS B2 CP B2-7.1 OUTLET B3/01





B2/05 (MH1050) RCRRJ CLASS 2 Q 15.2 l/s V 1.02 m/s 2.12 2.93 B2-1.1 B2-4.1

825Ø RCRRJ CLASS 4 RCRRJ CLASS 4 RCRRJ CLASS 4 WEIR Q 21.1 l/s V 1.11 m/s

В3

32.59

0.48

	3.00	
2.39	1.05	1.38
15.17	16.51	16.11
	17.56	17.49
	Q 47.9 l/s Qcap 96.7 l/s V 1.37 m/s	
	4.6	

300Ø

	REVISION DETAILS		INT	DATE	SURVEYED	
ſ	1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
	2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
Ī	3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
ſ					APPROVED	CD

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DRURY CENTRE-STAGE 2

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	3
COUNCIL	AUCKLAND COUNCIL	5
DWG NO P24-447-01-3517-DR		

C1-1.1

RCRRJ CLASS 4



- 1. PIPE LENGTHS MEASURED FROM MANHOLE CENTRE TO MANHOLE CENTRE.
- 2. VERTICAL CLEARANCES BETWEEN STORMWATER AND WASTEWATER SERVICES TO COMLPY WITH WATERCARE CODE OF PRACTICE TABLE 5.6.
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- 6. NETWORK MODELLED TO TP108 2.1CC.

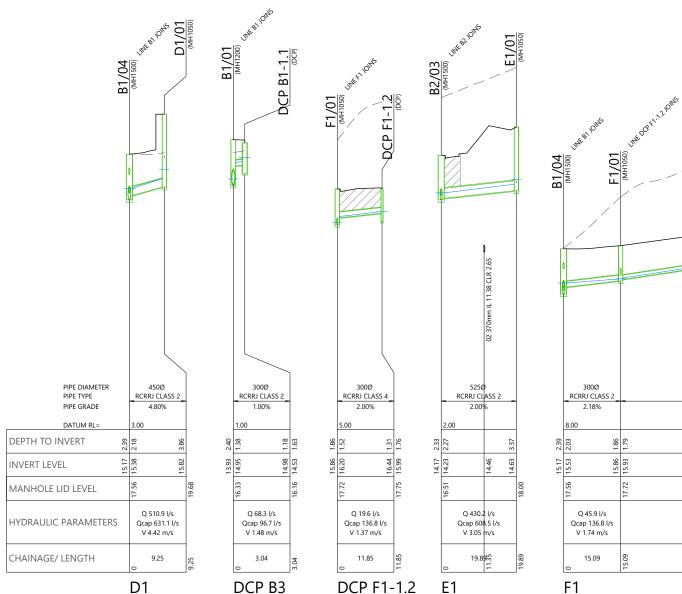
LEGEND

PROPOSED HARDFILL BACKFILL

PROPOSED FINISHED SURFACE

DEPTH OF FLOW (HYDRAULIC GRADELINE)

EXISTING GROUND



SAFE SLOPE FOR SATURATED OR SUBMERGED SOILS NOT TO EXCEED 1V:1.5H

	(MH1500) 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	F1/02	(MH1050) HWH HHY 1475 1702 1702 1703 1703 1703 1703 1703 1703 1703 1703	(MH1050) 1900 1900 1900 1900 1900 1900 1900 19	(CP) (O2	MH10500 WHAT WAS A P P P P P P P P P P P P P P P P P P
70/L9 E1/07		(WH 100	-	(MH1050) (CP F1-2.1	13	(MH1050) CP F1-2.2
	300Ø RCRRI CLASS 2 2.18% 8.00	2250 RCRRJ CLASS 2 3.00%		225Ø RCRII CLASS 4 1.00%		225Ø RCRR/ CLASS 2 1.00%
15.17 2.39		15.93 1.79	77 1.52		17.46 1.69	0.91
	17.56		19.29	19.29	19.15 17	19.29 18 19.10 19.10
	Q 45.9 l/s Qcap 136.8 l/s V 1.74 m/s	Q 27.0 l/s Qcap 89.8 l/s V 1.98 m/s		Q 13.2 l/s Qcap 44.9 l/s V 0.98 m/s		Q 13.9 l/s Qcap 44.9 l/s V 1.00 m/s
	15.09	61.17	76.26	7.66	99'.	1.6
	F1			F1-2.1		F1-2.2

REVISION DETAILS			DATE	SURVEYED	
1	FOR DISCUSSION	MK	22/01/2025	DESIGNED	MK
2	FOR THE 99% SUBMISSION	MK	10/02/2025	DRAWN	MK
3	FOR FAST TRACK CONSENT	EW	21/02/2025	CHECKED	GW
				APPROVED	CD

TRENCH SIDES TO BE BENCHED

NOTE: DIAGRAM SHOWS THE PREFERRED BENCHING

IF THIS IS NOT IMPLEMENTED, THE CONTRACTOR SHALL PROVIDE AN ASSESSMENT OF THE TRENCH BATTERS BY A COMPETENT PERSON, AS PER THE

WORKSAFE GUIDELINES, AND NOTIFICATION TO THE ENGINEER UPON COMPLETION OF THE

SOLUTION.

SAFE SLOPE ABOVE WATER TABLE -NOT TO EXCEED 1V:1H

EXCAVATION FACES BENCHED AND BATTERED TO A SAFE SLOPE



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DRURY CENTRE-STAGE 2

STORMWATER LONGSECTION (SHEET 19 OF 19)

STATUS	FOR FAST TRACK CONSENT	REV
SCALE	H 1:1000 @ A3 V 1:200 @ A3	2
COUNCIL	AUCKLAND COUNCIL)
DWG NO	P24-447-01-3518-DR	

1 FNGP24-447-01-3500-DR STORMWATI

4447 - DRURY CENTRE - STAGE 2 22140/02 DBAWINGSV