

100 Halsey Street

Erosion and Sediment Control Plan

1101937.1000-RPT-CIV-102

Prepared for: NZCRE 100 Halsey Limited

Prepared by: Tonkin & Taylor Ltd

Job Number: 1101937.1000

Date: October 2026

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Document control and review

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1 Overview

1.1 Introduction

NZ CRE 100 Halsey Limited is preparing a substantive application under the Fast-Track Approvals Act (2024) for a proposed development at 100 Halsey Street (the "Project") in Wynyard Quarter, CBD, Auckland.

NZ CRE 100 Halsey Limited has engaged Tonkin & Taylor Ltd (T+T) to prepare an Erosion and Sediment Control Plan (ESCP). This ESCP should be read in conjunction with the T+T Geotechnical Assessment [1] and Contamination Site Management Plan [2].

This ESCP outlines the erosion and sediment control principles, practices and procedures to be implemented to minimise erosion and the effects of sediment generation and discharge to the receiving environment associated with the proposed earthworks activities for the Project.

Erosion and sediment control measures will be put in place to minimise potential adverse effects. These control measures outlined meet industry best practice guidelines referenced in the Auckland Council's Erosion Sediment Control Guide (GD05) document.

It is noted that Contractor once selected will be required (as a condition of consent) to confirm and provide further details to its own construction methodology from which a finalised erosion and sediment control plan will be prepared prior to commencing earthworks construction.

1.2 Background

The project site located at 100 Halsey Street occupies a prominent central block within the Wynyard Quarter precinct, with key frontages to Amey Daldy Park and the Daldy Street Linear Park, as shown in Figure 1.1. Over the past decade, the surrounding area has undergone a significant transformation, with high-quality developments establishing elevated expectations and performance benchmarks across the precinct.

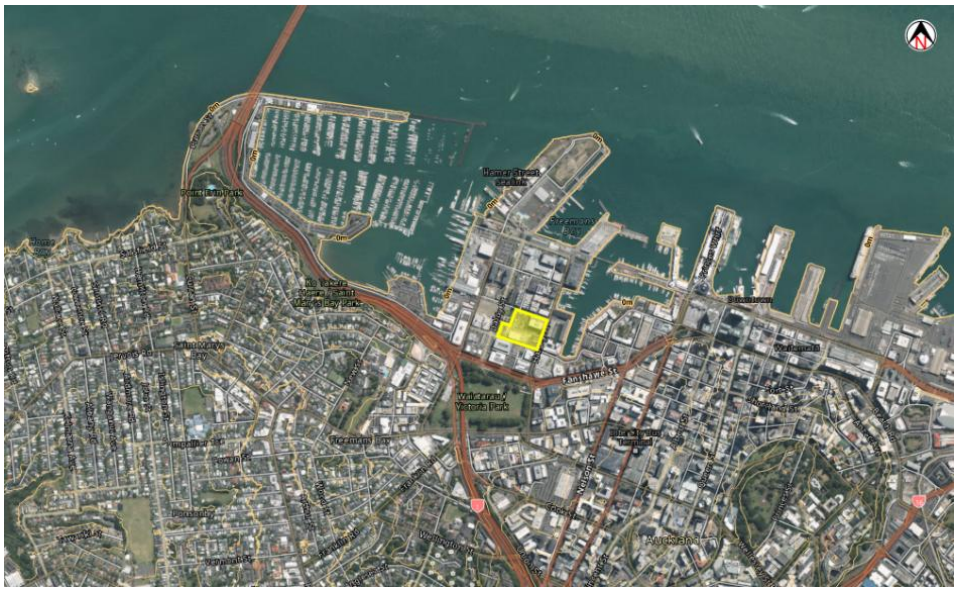


Figure 1.1: Site location (outlined in yellow).

[1] *1101937.1000_RPT-GEO-101_GeotechnicalAssessment.*
[2] *TT1101937_Contamination Site Management Plan_100 Halsey Street.*

The development site shown in Figure 1.2 is located directly west of the Viaduct Basin Marina and north of Fanshawe Street. Amey Daldy Park is adjacent to the site and includes a recently developed wastewater pump station, built in 2020. Structures surrounding the site consist of a mix of mid-rise (four-storey and above) apartments, hotels, and offices.

The total developable area of the underlying 17,142 m^2 parcel is 14,210 m^2 and currently comprises of a mixed industrial, commercial, and hotel use types. Industrial use on this land comprises a bus depot and mechanics workshop operated by NZ Bus. The majority of the site is utilised as a parking lot with the workshop located to the south and a refuelling station and bus wash located to the west.

In the northeast corner of site, an eight-storey structure hosts a supermarket, hotel, and several restaurants. This structure, the Travelodge building, is expected to remain unaltered.

Land Information New Zealand (LINZ) contour data indicates that the site is generally flat and low-lying, gently sloping to the south and east. Ground levels rise more steeply further south, beyond the site.

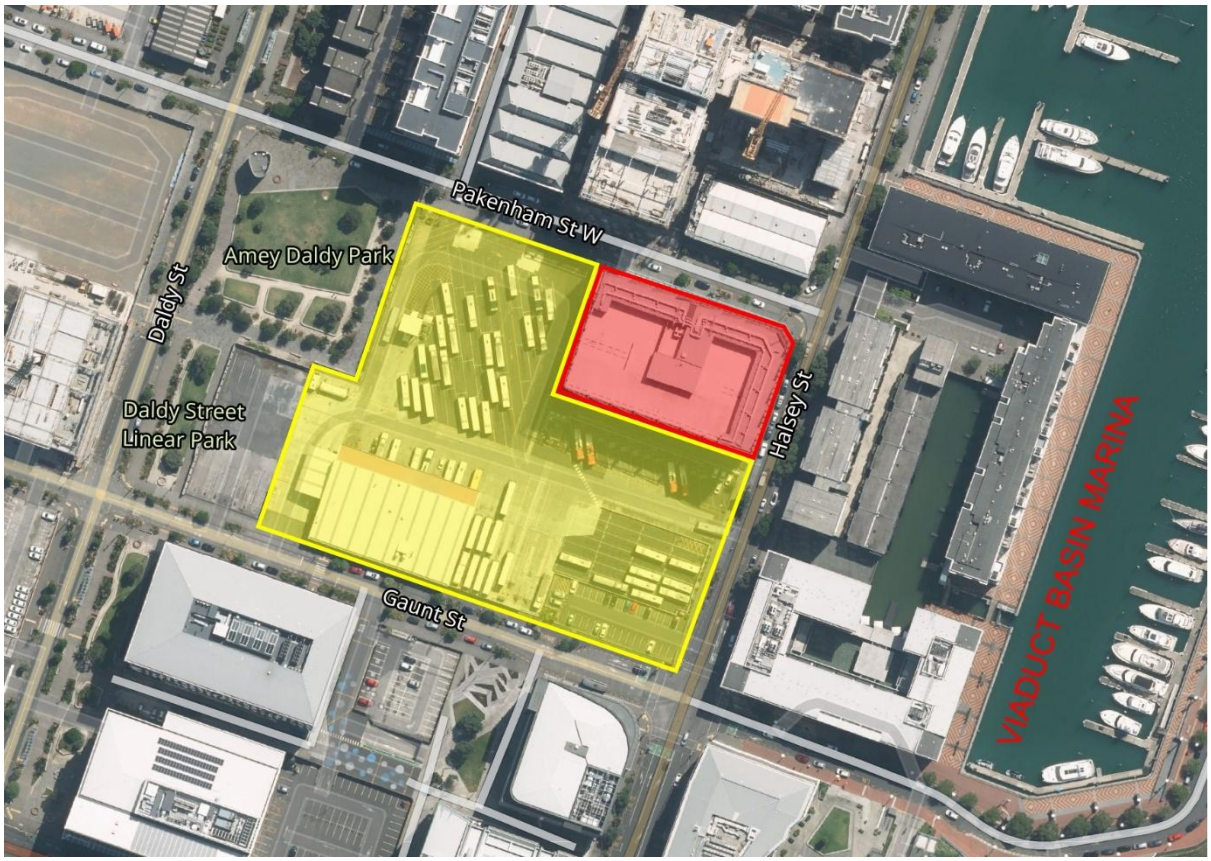


Figure 1.2: Site location. Project area shown in yellow. Existing structure shown in red, which is outside of the scope of this assessment.

1.3 Project Description

The proposed development seeks to deliver a high-quality mixed-use commercial precinct of five buildings comprising premium office accommodation, retail and food and beverage uses, integrated amenities and end-of-trip facilities, together with the supporting infrastructure, building services and car parking required to service the development.

Building 2 is proposed as a purpose-built data centre. The building will accommodate the specialist plant, infrastructure and servicing requirements associated with the data centre use, while also incorporating centralised infrastructure to support the wider development

The proposed development includes five new buildings, in addition to the existing Travelodge building, as shown in Figure 1.3. The buildings are intended to be commercial land use on the ground floor with offices on the upper floors.



Figure 1.3: Proposed development massing (sourced from Peddlethorp Architectural Plans).

The masterplan comprises the five buildings arranged around a central mid-block plaza, bounded by Gaunt Street, Halsey Street, Pakenham Street and Daldy Street / Amey Daldy Park.

- Building 1 - Commercial Office Building corner Halsey Street and Gaunt Street.
- Building 2 - Data Centre, sited centrally between B1 and existing Travelodge.
- Buildings 3A / 3B - Commercial Office buildings fronting Amey Daldy Park.
- Building 4 - Commercial Office on Gaunt Street.
- Building 5 - Commercial Office on Gaunt Street between Building 4 and VHHL site.

The building areas are presented in Table 1.1.

Table 1.1: Proposed development GFA areas

Building	Ground Floor Footprint (m²)	Total GFA (m²) (Excluding EoT/Plant)	Total Area (m²)
Building 1	1,217	9878	11,587
Building 2	1,610	10,832	12,702 (incl basement)
Building 3A/3B	3,430	29448	37,001 (incl basement)
Building 4	1,385	13,123	17,239 (incl basement)
Building 5	1,528	12,296	17,882.5 (incl basement)

Ground floor uses include retail and food and beverage (F&B) tenancies along street frontages and the mid-block plaza, with lobby, end-of-trip, services, and refuse facilities also provided at ground level.

Internal laneways and a central mid-block plaza provide pedestrian connectivity and vehicle access throughout the site.

Construction is proposed across four stages, commencing with Buildings 1 and 2 in late 2027. Buildings 3, 4, and 5 follow sequentially, with possible overlaps between stages. The NZ Bus depot progressively reduces its footprint throughout construction, fully vacating the site by 2035. The Travelodge remains operational throughout all stages via the southern laneway. Full precinct completion is anticipated following the completion of Buildings 4 and 5.

2 Description of construction

2.1 Proposed staging

Construction of Buildings B1 to B5 is to be completed in stages with the intention being to start with either Building B1 or a combination of Building B1 and Building B2 concurrently. As such, Erosion and Sediment Control Plans have been provided for specific construction area earthworks Being:

- Building B1 only.
- Building B2 only.
- Building B1+B2 concurrently.
- Building 3 only.
- Building 4 only.
- Building 5 only.
- Building 4+5 concurrently.

2.2 Extent of earthworks

The total developable area of the underlying 17,142 m^2 parcel is 14,210 m^2. Over the developable area, earthworks required include a site scrape of 500 mm depth, basement excavation, piles, foundations, service trenches and amenity landscaping. Total earthworks volume is approximately to be 59,070 m^3 with the majority of this material cut to waste. Total earthworks area is approximately 14,598 m^2 (388 m^2 of earthworks will be undertaken within the road reserve).

Refer to Table 2.1 for indicative earthworks volumes for each of the proposed stagings listed above.

Any contaminated excavated material will be managed in accordance with the requirements set out in the Contaminated Land Report[3].

The extent of any material reuse and filling is to be confirmed in the next stage of design however it is noted that the site scrape will include the demolition and extraction of slabs and ground structures associated with the previous development and are unlikely to be able to be re-used on site.

Table 2.1: Indicative Earthworks Volumes

Item	Assumptions	Area (m^2)	Depth (m)	Volume[1] (m^3)
Site strip	500 mm site strip	4238	0.5	2120
Piles	Pile design not confirmed. Assume total 160 piles at 900 mm dia to RL-20 m.	0.636*160 = 101.79 m^2	20	2040
Building 1				
Excavation	See Peddlethorp Gross Building Area Schedule – 11.09	1319	0.8	1060
Lift shaft	Assume 13 x 3 m	39	0.8	30

[3] TT1101937_Contamination Site Management Plan_100 Halsey Street

Item	Assumptions	Area (m^2)	Depth (m)	Volume[1] (m^3)
Building 2				
Excavation	See Peddlethorp Gross Building Area Schedule – 11.09	1646	5	8230
Lift shaft	Assume 2 shafts at 7.5 x 4.5 m	67.5	0.5	40
Building 3				
Excavation	See Peddlethorp Gross Building Area Schedule – 11.09	3666	5	18330
Lift shaft	Assume 4 shafts at 8 m x 3 m	96	0.7	70
Building 4+5				
Excavation	See Peddlethorp Gross Building Area Schedule – 11.09	3341	8	26730
Lift shaft	Assume 10.5 m x 9 m + 8 m x 9 m	166.5	0.7	120
Streetscaping				
Raingardens	Indicative sizing from Boffa Miskell	153 m	1.5	230
Vehicle Crossings	Indicative sizing from Boffa Miskell	235 m	0.3	70
Total		14598 m^2		59070 m^3

[1]All areas and volumes rounded up to the nearest 10 m^3.

2.2.1 Earthworks sequencing

Prior to any earthworks commencing for any construction stage, environmental controls will be installed. The earthworks sequence will start with demolition and a site strip before commencing with basement piling and basement excavation, foundations, piling, service utility trenching and amenity landscaping and streetscaping. For each respective staging option, a similar ESCP sequence will be followed. Please refer to the respective ESCP staging drawings for the site general arrangement at each construction stage.

2.2.2 Site Strip

The area within the building footprint will be stripped at each construction stage. Following demolition of above ground structures, this will involve an initial 500 mm excavation to remove the existing surface and any hardfill. Existing surfaces for removal may also include building slabs and foundations. The stripped area will be stabilised with GAP material.

Once the building in a particular zone has been constructed, the area surrounding the building will be excavated for the construction of surrounding pavement and amenity landscaping.

2.2.3 Basement Excavation

Basement excavation includes piling activities required for the basement excavation and the basement excavation itself. Following piling, which may be secant or other method, basements will be excavated as required for each construction stage. Basement excavations make up the majority of the earthworks and cut to waste for the project. Building B1 has no basement, B2 and B3 have a single-story basement while B4 and B5 have a two-story basement.

2.2.4 Piling

A detailed piling schedule has not been prepared at this stage so a general allowance has been made for building piles. Piles will need to extend down below the basement levels into the ECBF (East Coast Bays Formation) rock and be embedded at 3-20 m. Pile diameters will vary from 750 mm-1200 mm.

2.2.5 Utility Trenching

Each building will have a variety of utility connections into the structure. These will include water, wastewater, stormwater, power and communications. Connection details and extents will be confirmed at later design stages.

2.2.6 Streetscaping

Boffa Miskell have provided a markup of proposed changes to streetscaping in the roads adjacent to the development[4]. This markup details the adjustments to vehicle crossings, parking and raingardens on Halsey St, Pakenham St West and Gaunt St.

Any raingarden works are likely to require excavation to subgrade of approximately 1.5 m which will then be backfilled with an impermeable liner and growing media. Vehicle crossings are expected to require excavation of 300 mm and before concrete is poured to tie into the respective laneways and surrounding footpath levels.

2.3 Potential effects associated with earthworks

A list of potential effects that may be associated with activities related to earthworks if appropriate mitigations are not in place are:

- Sediment tracked out onto local roads, which can enter road catch pits and into the stormwater network and be discharged into the receiving environment.
- As the 1% AEP flood plain is located within the streets surrounding the site, during a flood event water from the road could enter the site and inundate the excavation.
- Pumps accidently left on during dewatering of excavations could potentially lead to secondary treatment devices overflowing and draining into the stormwater network and discharged into the receiving environment.
- Dust generation and associated effects on neighbouring properties and/or the public.
- Groundwater in excavation contact with wet concrete, management of acidic water.
- Groundwater is contaminated and harmful if discharged into the receiving environment.
- Back-to-back high intensity rain events could potentially overwhelm the dewatering activities and flood the site with the potential for discharge of sediment loads to the receiving environment.

[4] BM260024_100 Halsey Street FTA_Streetscape_Adjustment_Plans_20261006.pdf.

The specific erosion and sediment control measures to address the above potential effects are addressed in Section 3.

3 Erosion and sediment control measures

The following sections provide guidance on the selection of erosion and sediment control measures that can be implemented over the course of the project. The control measures outlines are based on the controls and principles outlined in GD05.

Refer to drawings 1101937.1000-020 to 1101937.1000-026 for proposed erosion and sediment control measures.

3.1 Construction Access

Construction access will be subject to change throughout the project dependent on what stage of construction is being progressed. Over the course of the project, construction access/egress will occur on Gaunt Street, Halsey St and Pakenham St West. These entrances/exits are to be stabilised and final locations confirmed with the Contractor prior to construction. Dependent on the chosen methodology, wheel wash areas can be installed within the existing hardstand or as part of a new stabilised access. Portable water blasters or proprietary wheel wash systems shall be used to clean vehicle wheels. Any sediment laden run off from these areas shall be collected and pumped to secondary treatment devices.

Road sweepers could be used as required to clear any deposited sediment.

3.2 Perimeter Controls

A perimeter bund will be installed (as required) to surround the proposed site boundary. This perimeter bund can be constructed from hot mix (asphalt) or sandbags.

The perimeter bund will isolate the site and keep clean water from entering the proposed earthworks area.

There is one minor overland flow path originating within the site and falling west towards Amey Daldy Park. It is expected that any earthworks will have no impact on this flow path as the construction grading will see the overland flows within the works zones directed to sediment retention devices within the basement excavations.

There are overland flow paths running down Gaunt and Halsey St however these are outside the extent of works and are not expected to be impacted by the project.

3.3 Stormwater inlet protection

Inlet protection will be installed as a precautionary measure on any rain gardens, catchpits etc. adjacent to the specific construction area and any construction access points. The protection method is to be determined by the contractor e.g. witches hats, silt socks.

There are 18 raingardens within the road corridor around the perimeter of the site. The surrounding road network is designed to flow to these raingardens. Protective measures for these raingardens will be provided as required.

During streetscaping works (raingarden or vehicle crossing construction), the contractor will have a small satellite site set up within the public realm. The contractor will keep the site tidy and protect any surrounding pavements. Silt socks are to be placed on the lower end of the site to filter any water exiting the work area during any excavation activities. Dewatering of any raingardens during construction will be as per the measures listed in Section 3.4.

3.4 Dewatering of excavations

There are two main sources of water that will need to be collected and discharged from the proposed site. These are from groundwater inflows, and rainwater surface runoff.

Groundwater inflows are expected to occur within building basement and lift shaft excavations and these locations will need to be dewatered to enable construction to progress.

As the groundwater is expected to be contaminated it should be handled separately and treated to prevent further contamination of water on the site.

All pumping activities would be monitored and recorded (refer to Section 4). Methods of dewatering the excavation site will be based as outlined in Sections 3.4.1.

It is expected that the contractor will shape any excavations to direct flows to specific low points within the building footprint. These could be a drop out pit or a trench which would capture larger volumes of water. The location of these low points is expected to move regularly within the building footprint in conjunction with the contractors detailed construction staging. A pump/pumps will be installed in these excavations to move run-off directly into secondary treatment devices.

3.4.1 Pumping into secondary treatment device.

The purpose of the secondary treatment device is to allow for sediment within the stored water volumes to settle out before being discharged to the stormwater network, or to trade waste (with a permit applied separately with Watercare at the construction stage). It is expected that the secondary treatment devices will sit at existing surface level.

Due to space constraints, options for secondary treatment devices are:

- Portable sediment tank featuring screens/baffle walls (lamellar clarifiers). The approximate size of these units is similar to the dimensions of a 20-foot shipping container (approximately 6.06 m x 2.44 m).
- Proprietary rainwater tanks up to 25,000 L being purposed as settling tanks.

Prior to the discharge into the public stormwater network or to trade waste, the treated water from the secondary treatment devices is to be tested to confirm the quality of the treated water does not contain unacceptable levels of contamination or elevated pH levels. A minimum of 100 mm water clarity is also to be achieved prior to discharge. The construction team are to continually monitor water quality and adjust treatment as required to achieve water quality requirements.

3.5 Dewatering during winter period

If the earthworks are to continue throughout the winter period, it is expected that the groundwater inflow and rainfall run off volumes entering the excavation site will be higher than the summer periods. To address the increased runoff volumes, the following contingency measures can be implemented if required during the winter period to aid in the mitigation of these peak flow rates:

- Additional pumps may be installed as required to improve the dewatering rate and capacity.
- Additional secondary treatment devices and rainwater tanks can be installed to increase the treatment and storage volumes if required.

3.6 Contamination Risk Mitigation

Report 1101937.1000-RPT-CON-102 Contamination Site Management Plan indicates that within the site there may be "*widespread heavy metal and PAH contamination within fill materials, with concentrations exceeding the NESCS and AUP PA criteria, including localised hotspots of arsenic, copper, lead and BaP equivalent."* Likewise with groundwater, historical investigations show concentrations of cyanide.

Refer to Report 1101937.1000-RPT-CON-102 Contamination Site Management Plan for future groundwater sampling and pre-construction testing requirements.

Potential options to manage contaminated water are below:

1 Treatment prior to discharge into the stormwater network by a proprietary treatment device suitable for the specific contaminants identified. The treated groundwater would be tested and confirmed to be within acceptable limits before any controlled discharge is made into the local stormwater network. The treatment process can take time and potentially constrain the rate at which the excavation can be dewatered and so this would need to be managed to avoid adversely impacting the Contractors earthworks construction programme.

2 Subject to Watercare approval, the contaminated groundwater could potentially be discharged into the wastewater network as trade waste. Watercare may impose conditions such as ongoing testing of contamination levels to verify that they remain within any agreed limits.

All procedures for the management of contaminated groundwater, are to follow the requirements and recommendations set out in future Land Contamination Report.

In addition, any runoff which has contacted fresh concrete may have an elevated pH. In this case this runoff will need to be controlled before discharge into the stormwater system. Chemical dosing would ensure the pH of the water is within acceptable limits (typically pH in the range of 6.0 – 10).

A dosage tank (typically a 25,000L rain tank) would be adopted where discharges from this tank will be from a floating inlet to ensure that any sediment settles out and is retained prior to discharge. Clean water discharge from the secondary treatment devices adopted by the Contractor (outlined in Section 3.4 of this report) will have a piped inlet in through the top of the dosing tank.

The pH will be monitored by checking the levels before discharge. If it is found to be outside the required level (typically 6.0 - 10) the water within the discharge tank can be dosed with sodium carbonate or bisulphate or another acceptable dosing agent to adjust this pH before discharge.

During periods of forecast heavy rainfalls and winter periods, additional dosing tanks may be used to increase the dewatering volume capacity.

3.7 Dust Control

Sufficient water should be made available for dust control and used to wet excavation works or work areas if necessary. Where possible this will be recycled from the top water surface (where the water is clearer) of secondary treatment devices.

Excavated material will generally be loaded directly into trucks for disposal offsite to minimise the need for stockpiling.

3.8 Seasonal Considerations

Initial earthworks and any basement excavation is likely to be undertaken within the earthworks season however there is potential that construction may extend into winter.

Therefore, the earthworks stage of building construction is proposed to have no seasonal restrictions. Additional erosion and sediment control measures would be implemented during the winter months enabling earthworks to be completed without the need to temporarily close the site during winter. Given the site's proximity to public amenities, buildings and adjacent roads, it is considered most beneficial to continue the earthworks operation through winter if required to allow the site to be completed and sealed off as soon as practicable.

4 Monitoring, maintenance and reporting

4.1 Extreme Weather Events

The seven-day weather forecast shall be monitored and the erosion and sediment control measures adjusted in preparation for the conditions. Monitoring of all measures shall be undertaken regularly with a checklist recorded and kept on site, this includes monitoring of site conditions and ensuring additional measures are implemented as required. Maintenance of the control devices are to be in accordance with Auckland Council's GD05.

The perimeter bund around the site is to be monitored during heavy rainfall events as the 1% AEP flood plain is located around the site boundaries. Additional measures such as sandbags would be placed (if required) to protect any basement excavation from flood inundation. It is expected that streetscaping works may be more exposed to flood hazard events. However, the extent and duration of the streetscape works is minor in comparison to the main works areas.

It will be the responsibility of the Contractor to prepare an extreme weather site procedure. The procedure may involve shutting down the site and allowing the sediment laden and runoff to collect within the excavation in the meantime. This sediment laden and runoff can then be dewatered and treated accordingly before being discharged.

4.2 Dust

Dust shall be monitored daily with water used to suppress dust as required. Contractor shall keep the site clean and tidy to minimise dust control required.

4.3 Quality of discharged water

When sediment build up in any secondary treatment devices reaches 20% of the storage volume, the secondary treatment device shall be de-silted.

The monitoring of pumping activities will be completed and recorded on monitoring sheets to ensure pumps are not left unattended to mitigate the risk of treatment devices overflowing and discharging into the receiving environment (refer to Appendix C).

4.4 Records

Refer to the maintenance checklist in Appendix B. Records of water discharge frequency and quality control testing are to be maintained by the contractor throughout the course of the project.

5 Unitary Plan Provisions

5.1 E11.4.1(A9) Earthworks Area

Earthworks across the project are greater than 2,500 m^2 therefore the activity is regarded as Restricted Discretionary and a Resource Consent is required.

5.2 E12.4.1(A6) and (A10) Earthworks Area and Volume

Earthworks across the project are greater than 2,500 m^2 and 2,500 m^3 therefore the activity is regarded as Restricted Discretionary and a Resource Consent is required.

5.3 E12.6.2(11) and (12), including PC120

The natural hazard assessments (coastal and flooding) have not identified that the activities will create or increase the natural hazard risk(s) to other properties, infrastructure and the environment.

The detailed flood assessment of the post-development scenario (refer Appendix C of the Natural Hazards Assessment) identified that the flood level in the vicinity of the site is determined by the topographic depression and spillway level to the coast. The proposed development will reduce the flood storage of the floodplain on the site by around 50 m*3* due to the building footprint occupying this area and the raised levels proposed. This small change in flood storage will not increase surrounding flood levels due to the close proximity and overflow to the coast. This also means that there are no adverse effects predicted as a result of filling in the floodplain.

The building design will not adversely affect neighbours' exposure to coastal inundation hazards. This is because coastal inundation from the sea is time and level limited but is not volume limited and is therefore not mass-balanced like catchment flows. For example, neighbouring areas that would be exposed to the same hazard will have the same exposure after the development is complete. Likewise, neighbouring areas that are not exposed will remain unexposed after this development is complete.

6 Applicability

This report has been prepared for the exclusive use of our client NZ CRE 100 Halsey Limited, with respect to the particular brief given to us and it may not be relied upon in other contexts or for any other purpose, or by any person other than our client, without our prior written agreement.

We understand and agree that our client will submit this report as part of an application under the Fast-Track Approvals Act 2024 and that the Expert Panel as the consenting authority will use this report for the purpose of assessing that application.

We understand and agree that this report will be used by Expert Panel in undertaking its regulatory functions.

Tonkin & Taylor Ltd
Environmental and Engineering Consultants

Report prepared by:



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Joseph Sterritt
Civil Engineer

Authorised for Tonkin & Taylor Ltd by:



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Project Director

JSTER

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Appendix A Erosion and Sediment Control Drawings

A1 **B1 only**

A2 **B2 only**

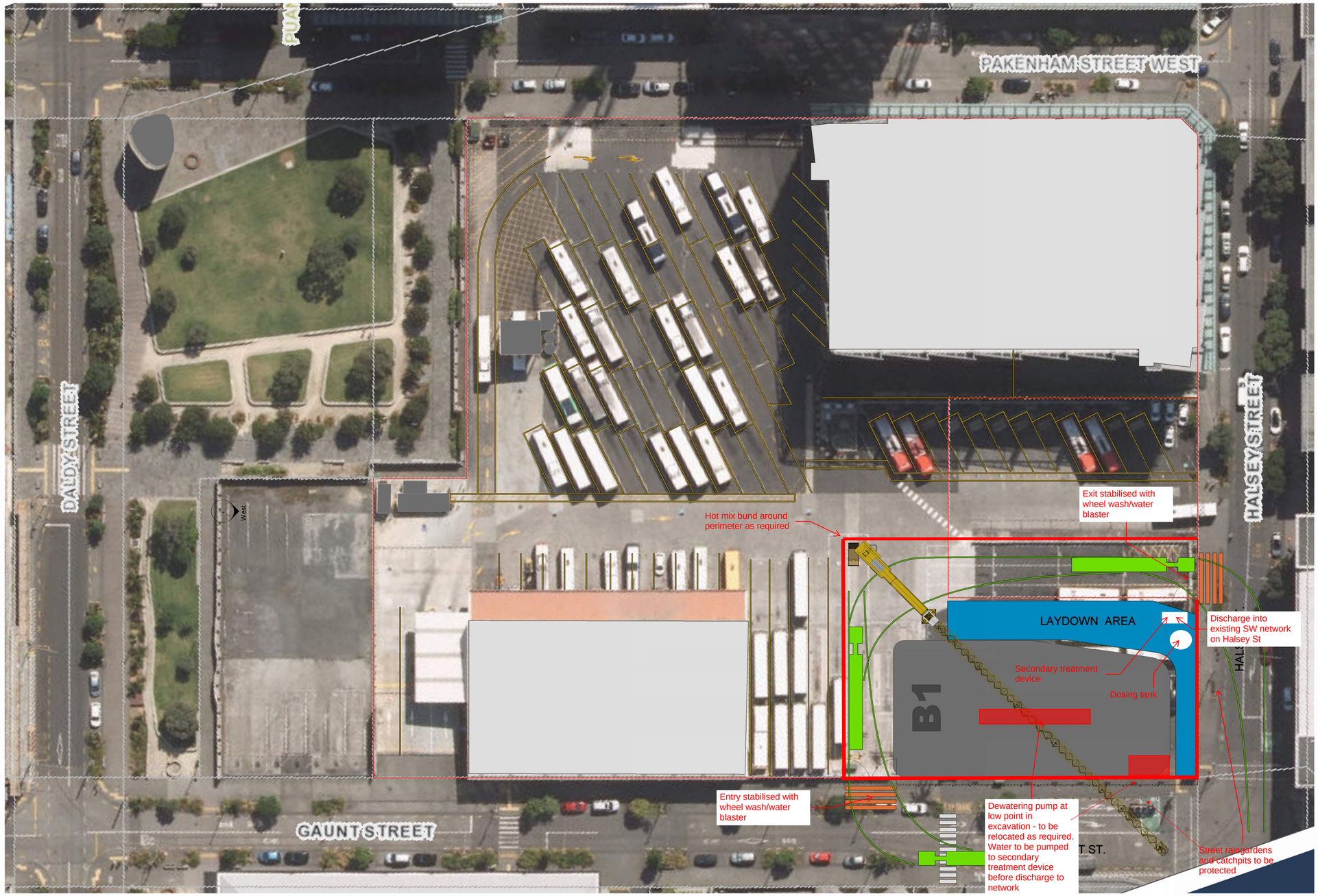
A3 **B1 + B2 concurrently**

A4 **B3 only**

A5 **B4 only**

A6 **B5 only**

A7 **B4+B5 concurrently**



#1 0 GROUND B1 only

1 : 350

McGuinness LIMITED BUILDING CONTRACTORS

Job Code

#1

C1-101

Scale 1 : 350

Revisions

Rev	Description	Date

Designed By	Designer
Drawn by	Johan.B
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Approved by	Approver



#2 0 GROUND B2 only

1 : 350

L.T. McGuinness LIMITED BUILDING CONTRACTORS

Job Code

#2

C1-102

Scale 1 : 350

Revisions

Rev	Description	Date

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Reviewed by	Checker
Approved by	Approver



#3 0_GROUND B1+B2

1 : 350

L.T. McGuinness LIMITED
BUILDING CONTRACTORS

Job Code

#3

C1-103

Scale 1 : 350

Revisions

Rev	Description	Date

Designed By	Designer
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Approved by	Approver

Autodesk Docs://100 Halsey Street - Tender/LTM_100 Halsey St_Tender model.rvt

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#4 0 GROUND B3 alone

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BUILDING CONTRACTORS

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#4

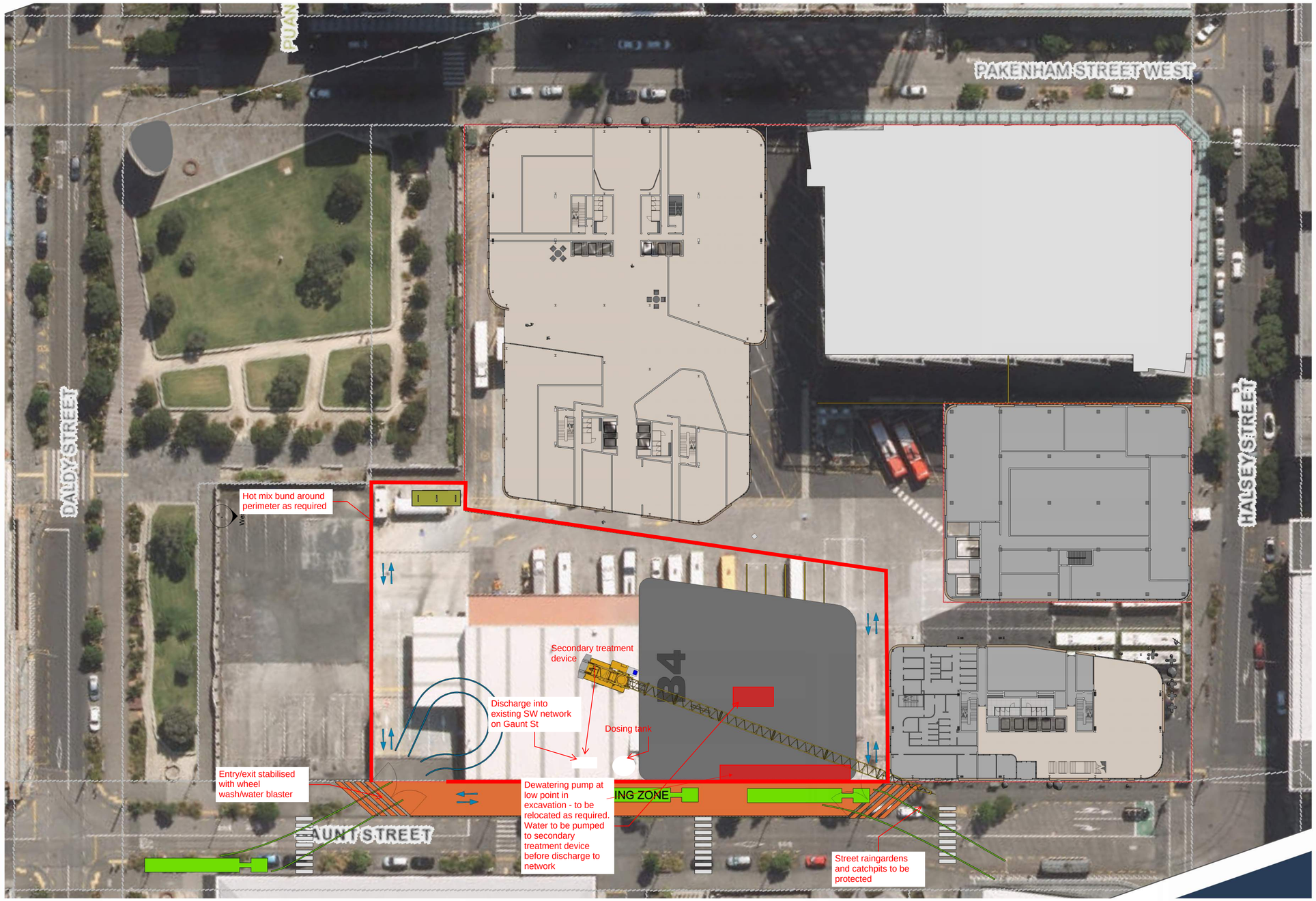
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#5 0_GROUND B4 only

1 : 350

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#5

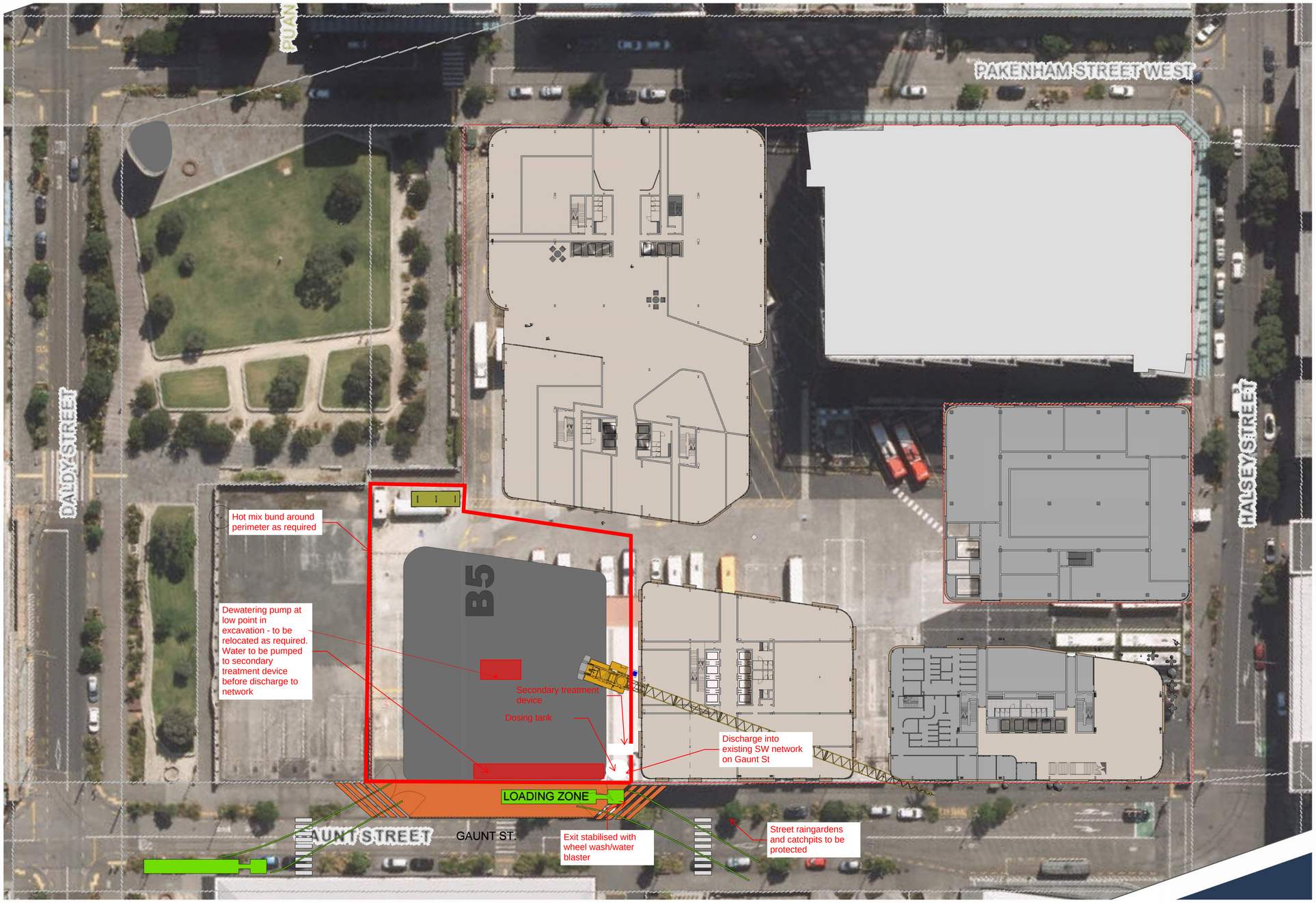
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#6 0 GROUND B5 only

1 : 350

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Job Code

#6

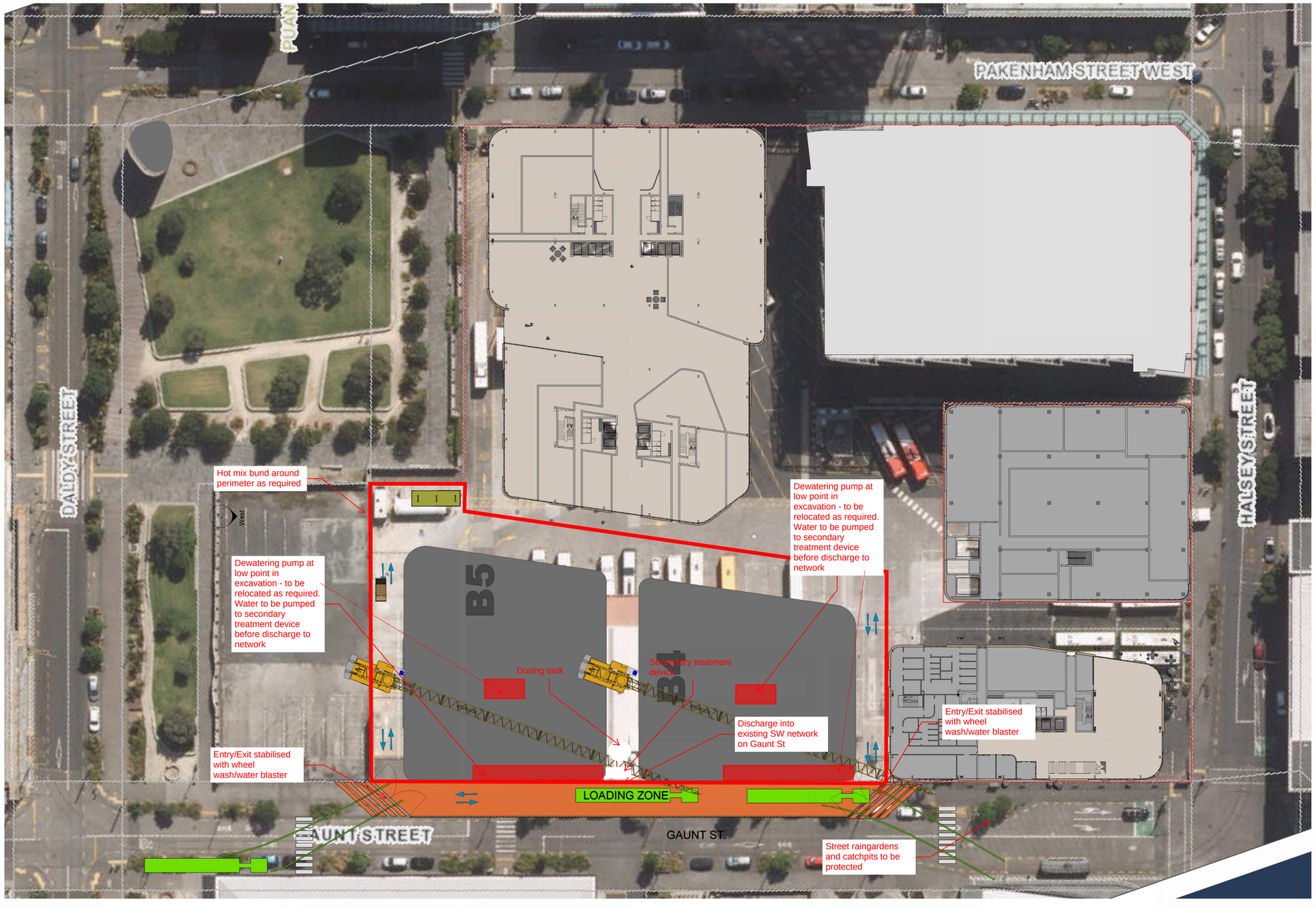
C1-106

Scale 1 : 350

Revisions

Rev	Description	Date

Designed By	Designer
Drawn by	Johan.B
Reviewed by	Checker
Approved by	Approver



#7 0_GROUND B4+B5

1 : 350

L.T. McGuinness LIMITED
BUILDING CONTRACTORS

Job Code

#7

C1-107

Scale 1 : 350

Revisions

Rev	Description	Date

Designed By	Designer
Drawn by	Author
Reviewed by	Checker
Approved by	Approver

Appendix B Monitoring and maintenance checklist

	SITE EROSION & SEDIMENT CONTROL INSPECTION CHECKLIST	

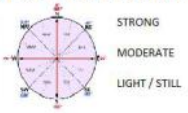
Site: 100 Halsey St **Project No:** 1101937

Inspection by: ..... **Date:** ..... **Time:** .....

Current Weather Condition: (circle)



Wind Direction / Conditions



Area Inspected (tick): ☐ Cleanfill ☐ Clay extraction ☐ Other (please specify):

.....

ITEM / SCOPE OF INSPECTION	(circle)	COMMENTS
EROSION & SEDIMENT CONTROL		
Is the stablised entranceway clean?	Y N N/A	
Clean water is dewatered at the top first, then the residual sediment-laden water is pumped to a tank/truck	Y N N/A	
Stormwater inlet protection measures remain in place.	Y N N/A	
The discharge is monitored to ensure it meets required standards.	Y N N/A	
A minimum of 100 mm water clarity is required to pump directly offsite. If there is not 100 mm water clarity, the water is either: • Treated in situ until the clarity is achieved and then pumped offsite • Pumped to a sediment retention device, tank or skip for settlement or flocculant treatment device before discharge offsite • Pumped to a tanker and removed offsite	Y N N/A	
Check perimeter bunds have not been compromised	Y N N/A	

Appendix C Pumping monitoring sheet

Description of area	
Clarity of water	
Pump inlet is supported above the base of the excavation	

Confirm clarity and pH of water at pump outlet every 60 minutes during pumping				
Time	**Clarity**	**pH**	**signed**	**comments**



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